

# Report No A1077-11

**SANDESEND SLOPE STABILISATION**

**WHITBY, NORTH YORKSHIRE**

**FACTUAL REPORT ON GROUND INVESTIGATION**

Carried out for:  
Balfour Beatty Living Places

Engineer:  
Haskoning Ltd

December 2011

**Soil Mechanics**  
Askern Road, Carcroft,  
Doncaster, South Yorkshire, DN6 8DG, UK  
Tel: +44 (0) 1302 723456 Fax: +44 (0) 1302 725240  
email: sm.doncaster@esg.co.uk

Soil Mechanics part of Environmental Scientifics Group

**SANDESEND SLOPE STABILISATION**

**WHITBY, NORTH YORKSHIRE**

**FACTUAL REPORT ON GROUND INVESTIGATION**

**Report No: A1077-11**

**Date: December 2011**

**Employer:**

**Balfour Beatty Living Places  
Cholmley Way  
Whitby  
North Yorkshire  
YO22 4NQ**

**Engineer:**

**Haskoning Ltd  
Marlborough House  
Marlborough Crescent  
Newcastle Upon Tyne  
NE1 4EE**

<b>Issue No</b>	<b>Date</b>	<b>Details</b>
1	December 2011	Report as submitted

The title to this report is vested in the Employer named but title to copyright is retained. The Contracts (Rights of Third Parties) Act 1999 does not apply to the contract with the Employer and the provisions of the said Act are hereby excluded.

---

**C O N T E N T S**

	Page
<b>1 INTRODUCTION</b>	<b>2</b>
<b>2 THE SITE AND GEOLOGY</b>	<b>2</b>
2.1 The Site	
2.2 Published Geology	
<b>3 FIELDWORK</b>	<b>2</b>
3.1 General	
3.2 Exploratory Holes	
3.3 Instrumentation	
<b>4 LABORATORY TESTING</b>	<b>4</b>
4.1 Geotechnical Testing	
4.2 Geoenvironmental Testing	
<b>REFERENCES</b>	<b>5</b>
<b>ENCLOSURES</b>	
A EXPLORATORY HOLE RECORDS	
B INSTRUMENTATION	
C GEOTECHNICAL LABORATORY TEST RESULTS	
D GEOENVIRONMENTAL LABORATORY TEST RESULTS	
E PHOTOGRAPHS	
F DRAWINGS	

## **1 INTRODUCTION**

In September 2011 Soil Mechanics was commissioned by Balfour Beatty Living Places to carry out a ground investigation at Sandsend, near Whitby in North Yorkshire, under the direction of their technical advisors, Haskoning Ltd. The investigation was required to obtain geotechnical and geoenvironmental information for proposed stabilisation measures in an area of cliff slope instability on the landward side of the A174 Sandsend Road.

The scope of the investigation, which was specified by Haskoning comprised cable percussion and rotary drilled boreholes, dynamic sampling and laboratory testing. The investigation was carried out in accordance with the contract specification, Eurocode 7 and relevant related standards identified below (see also References). The fieldwork was carried out between 3 and 10 October 2011.

This report presents the factual records of the fieldwork and laboratory testing. The data is also presented separately in digital format following AGS (2005).

## **2 THE SITE AND GEOLOGY**

### **2.1 The Site**

The site comprises an approximately 1 km length of coastline between Sandsend and Whitby, see Site Location Plan in Enclosure F. The area of slope instability is about 200 m long and centred at National Grid reference NZ 867 123.

### **2.2 Published Geology**

The published geological map covering the site, combined BGS Sheets 35 and 44 (1998), show Glacial Till overlying the Whitby Mudstone Formation of the Lias Group.

## **3 FIELDWORK**

### **3.1 General**

The fieldwork was carried out in general accordance with BS 5930+A2 (2010), BS EN 1997-2 (2007) and BS EN ISO 22475-1 (2006).

The exploratory hole locations were selected by Haskoning and were set out by Soil Mechanics and Haskoning from local features. The co-ordinates and reduced levels were surveyed by Soil Mechanics to National Grid and Ordnance Datum. The exploratory hole locations are shown on the Site Plan in Enclosure F.

### 3.2 Exploratory Holes

The exploratory holes are listed in the following table.

#### SUMMARY OF EXPLORATORY HOLES

TYPE	QUANTITY	MAXIMUM DEPTH (m)	REMARKS
Cable Percussion Boring extended by Rotary Core Drilling	6	40.7	BH1 to 3 carried out above the slip area, and BH4 to 6 at road level (assumed slip toe).
Dynamic Sampling	5	4.2	WS1 to 5 carried out at beach level spaced along approximately 900 m of coastline.

The exploratory hole records are presented in Enclosure A and should be read in conjunction with the Key which is included in that enclosure. The records provide descriptions of the materials encountered in accordance with BS EN ISO 14688-1 (2002) and 14689-1 (2003), for soils and rocks respectively, as amplified by BS 5930+A2 (2010). The records also give details of the samples taken together with observations made during boring, drilling and dynamic sampling. Standard penetration tests (SPT) were carried out in accordance with BS EN ISO 22476-3 (2005) and the results are included on the borehole records. The SPT hammer energy ratio calibration certificates are also included in Enclosure A.

Photographs of the cores recovered from the rotary drilling are presented in Enclosure E.

On completion of the fieldwork geotechnical samples were transported to the Doncaster laboratory of Soil Mechanics for temporary retention and testing. Geoenvironmental samples were transported from site directly to the ESG Scientifics laboratory.

### 3.3 Instrumentation

The instruments installed in the exploratory holes are shown on the logs and detailed in Enclosure B. Soil Mechanics was not required to carry out any monitoring of these.

## 4 LABORATORY TESTING

### 4.1 Geotechnical Testing

The testing was scheduled by Haskoning and was carried out in accordance with BS 1377 (1990) unless otherwise stated. The testing is summarised below and the results are presented in Enclosure C.

#### SUMMARY OF GEOTECHNICAL LABORATORY TESTING

TYPE	REMARKS
Moisture Content Determination	
Atterberg Limit Determination	
Particle Size Distribution Analysis	
pH and Water Soluble Sulphate Content of Soils	
Unconsolidated Undrained Triaxial Compression	
Consolidated Undrained Triaxial Compression	
Shear Strength by Direct Shear (Shearbox)	
Residual Shear Strength (Ring Shear)	
Laboratory Vane Shear Strength	
One Dimensional Oedometer Consolidation	
Dry Density / Moisture Content Relationship	
Moisture Condition Value (MCV) / Moisture Content Relationship	

### 4.2 Geoenvironmental Testing

The testing was scheduled by Haskoning and was carried out by ESG Scientifics. The results are presented in Enclosure D.

<b>Prepared By</b>	<b>P Hepton BSc PhD</b>
<b>Reviewed By</b>	<b>L K Rodger BSc MSc CGeol CSci FGS</b>
<b>Approved for Issue By</b>	

---

## REFERENCES

- AGS : 2005 : Electronic transfer of geotechnical and geoenvironmental data (Edition 3.1 including addendum May 2005). Association of Geotechnical and Geoenvironmental Specialists.
- BGS England and Wales Sheet 35 and 44 : 1998 : Whitby and Scalby. 1:50 000 geological map (solid and drift). British Geological Survey
- BS 1377 : 1990 : Methods of test for soils for civil engineering purposes. British Standards Institution.
- BS 5930+A2 : 2010 : Code of practice for site investigations (Amendment 2). British Standards Institution.
- BS EN 1997-2 : 2007 : Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing. British Standards Institution.
- BS EN ISO 14688-1 : 2002 : Geotechnical investigation and testing - Identification and classification of soil - Part 1 Identification and description. British Standards Institution.
- BS EN ISO 14689-1 : 2003 : Geotechnical investigation and testing - Identification and classification of rock - Part 1 Identification and description. British Standards Institution.
- BS EN ISO 22475-1 : 2006 : Geotechnical investigation and testing – Sampling methods and groundwater measurements - Part 1 Technical principles for execution. British Standards Institution.
- BS EN ISO 22476-3 : 2005 : Geotechnical investigation and testing - Field testing - Part 3 Standard penetration test. British Standards Institution.

**ENCLOSURE A**  
**EXPLORATORY HOLE RECORDS**

Key to Exploratory Hole Records  
SPT Hammer Energy Reports  
Borehole Logs  
Dynamic Sampler Hole Logs

Key  
Calibration certificates for DC1 and JB15  
BH1 to 6  
WS1 to 5

# Key to Exploratory Hole Records

## SAMPLES

### Undisturbed

U	Driven tube sample	} nominally 100 mm diameter and full recovery unless otherwise stated
UT	Driven thin wall tube sample	
TW	Pushed thin wall tube sample	
P	Pushed piston sample	
L	Liner sample (from Windowless or similar sampler), full recovery unless otherwise stated	
CBR	CBR mould sample	
BLK	Block sample	
CS	Core sample (from rotary core) taken for laboratory testing	
AMAL	Amalgamated sample	

### Disturbed

D	Small sample
B	Bulk sample

### Other

W	Water sample
G	Gas sample

	Environmental chemistry samples (in more than one container where appropriate)
ES	Soil sample
EW	Water sample

### Comments

Sample reference numbers are assigned to every sample taken. A sample reference of 'NR' indicates that attempt was made to take a tube sample, however, there was no recovery.

Monitoring samples taken after completion of hole construction are not shown on the exploratory hole logs.

## TESTS

SPT S or SPT C	Standard Penetration Test, open shoe (S) or solid cone (C)
----------------	--

The Standard Penetration Test is defined in BS EN ISO 22476-3 (2005). The incremental blow counts are given in the Field Records column; each increment is 75 mm unless stated otherwise and any penetration under self weight in mm (SW) is noted. Where the full 300 mm test drive is achieved the total number of blows for the test drive is presented as N = \*\* in the Test column. Where the test drive blows reach 50 the total blow count beyond the seating drive is given (without the N = prefix).

IV	<i>in situ</i> Vane shear strength, peak (p) and remoulded (r)
HV	Hand vane shear strength, peak (p) and remoulded (r)
PP	Pocket penetrometer test, converted to shear strength
KFH, KRH, KPI	Permeability tests (KFH = falling head, KRH = rising head; KPI = packer inflow); results provided in Field Records column (one value per stage for packer tests)

## DRILLING RECORDS

The mechanical indices (TCR/SCR/RQD & If) are defined in BS 5930+A2 (2010)

TCR	Total Core Recovery, %
SCR	Solid Core Recovery, %
RQD	Rock Quality Designation, %
If	Fracture spacing, mm. Minimum, typical and maximum spacings are presented. The term non-intact (NI) is used where the core is fragmented.

Flush returns, estimated percentage with colour where relevant, are given in the Records column

CRF	Core recovered (length in m) in the following run
AZCL	Assessed zone of core loss
NR	Not recovered

## GROUNDWATER

▼	Groundwater strike
▽	Groundwater level after standing period

Notes:  
See report text for full references of standards

Project SANDSEND SLOPE STABILISATION  
Project No. A1077-11  
Carried out for Balfour Beatty Living Places

Key

Sheet 1 of 2

# Key to Exploratory Hole Records

## INSTALLATION

### Standpipe/ piezometer

Details of standpipe/piezometer installations are given on the Record. Legend column shows installed instrument depths including slotted pipe section or tip depth, response zone filter material type and layers of backfill.

SP  
SPIE  
PPIE  
EPIE



The type of instrument installed is indicated by a code in the Legend column at the depth of the response zone:

Standpipe  
Standpipe piezometer  
Pneumatic piezometer  
Electronic piezometer

### Inclinometer or Slip Indicator

The installation of vertical profiling instruments is indicated on the Record. The base of tubing is shown in the Legend column.

ICE  
ICM  
SLIP



The type of instrument installed is indicated by a code in the Legend column at the base of the tubing:

Biaxial inclinometer  
Inclinometer tubing for use with probe  
Slip indicator

### Settlement Points or Pressure Cells

The installation of single point instruments is indicated on the Record. The location of the measuring device is shown in the Legend column.

ESET  
ETM  
EPCE  
PPCE



The type of instrument installed is indicated by a code in the Legend column:

Electronic settlement cell/gauge  
Magnetic extensometer settlement point  
Electronic embedment pressure cell  
Electronic push in pressure cell

## INSTALLATION LEGENDS

A legend describing the installation is shown in the rightmost column. Legends additional to BS5930 are used to describe the backfill materials as indicated below.

Arisings



Concrete



Grout



Bentonite



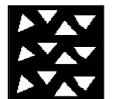
Sand



Gravel



Macadam



## NOTES

- 1 Soils and rocks are described in accordance with BS EN ISO 14688-1 (2002) and 14689-1 (2003) respectively as amplified by BS 5930+A2 (2010).
- 2 For fine soils consistency determined in the field by the logger is reported for those strata where undisturbed samples are available. The consistency is qualified and given (in brackets) when, in the opinion of the logger, the sample is disturbed but the assessed consistency is reasonably representative of the in situ conditions; in these circumstances it will normally underestimate consistency in situ. No consistency is given where the samples available are too disturbed to allow a reasonable assessment.
- 3 Evidence of the occurrence of very coarse particles (cobbles and boulders) is presented on the logs, however, because of their size in relation to the exploratory hole these records may not be fully representative of their size and frequency in the ground mass.
- 4 The declination of bedding and joints is given with respect to the normal to the core axis. Thus in a vertical borehole this will be the dip.
- 5 The assessment of SCR, RQD and Fracture Spacing excludes artificial fractures
- 6 Strata legends are in accordance with BS 5930+A2 (2010).
- 7 Water level observations of discernible entries during the advancing of the exploratory hole are given at the foot of the log and in the Legend column. The term "none observed" is used where no discrete entries are identified although this does not necessarily indicate that the hole has not been advanced below groundwater level. Under certain conditions groundwater cannot be observed, for instance, drilling with water flush or overwater, or boring at a rate much faster than water can make its way into the borehole (ref BS5930+A2:2010, Clause 47.2.7). In addition, where appropriate, water levels in the hole at the time of recovering individual samples or carrying out in situ tests and at shift changes are given in the Records column.
- 8 The borehole logs present the results of Standard Penetration Tests recorded in the field without correction or interpretation. However, in certain ground conditions (eg high hydraulic head or where very coarse particles are present) some judgement may be necessary in considering whether the results are representative of in situ mass conditions.

Updated March 2011

Notes:  
See report text for full references of standards

Project **SANSEND SLOPE STABILISATION**  
Project No. **A1077-11**  
Carried out for **Balfour Beatty Living Places**

**Key**

Sheet 2 of 2

# Hammer Energy Report



Soil Mechanics

**Date of test:** 25/11/2010  
**Instrumented rod:**  
**Type** BW  
**Cross-sectional area (Aa)** 11.30 cm<sup>2</sup>  
**Young's modulus (Ea)** 207000 MPa  
**Length** 0.60 m

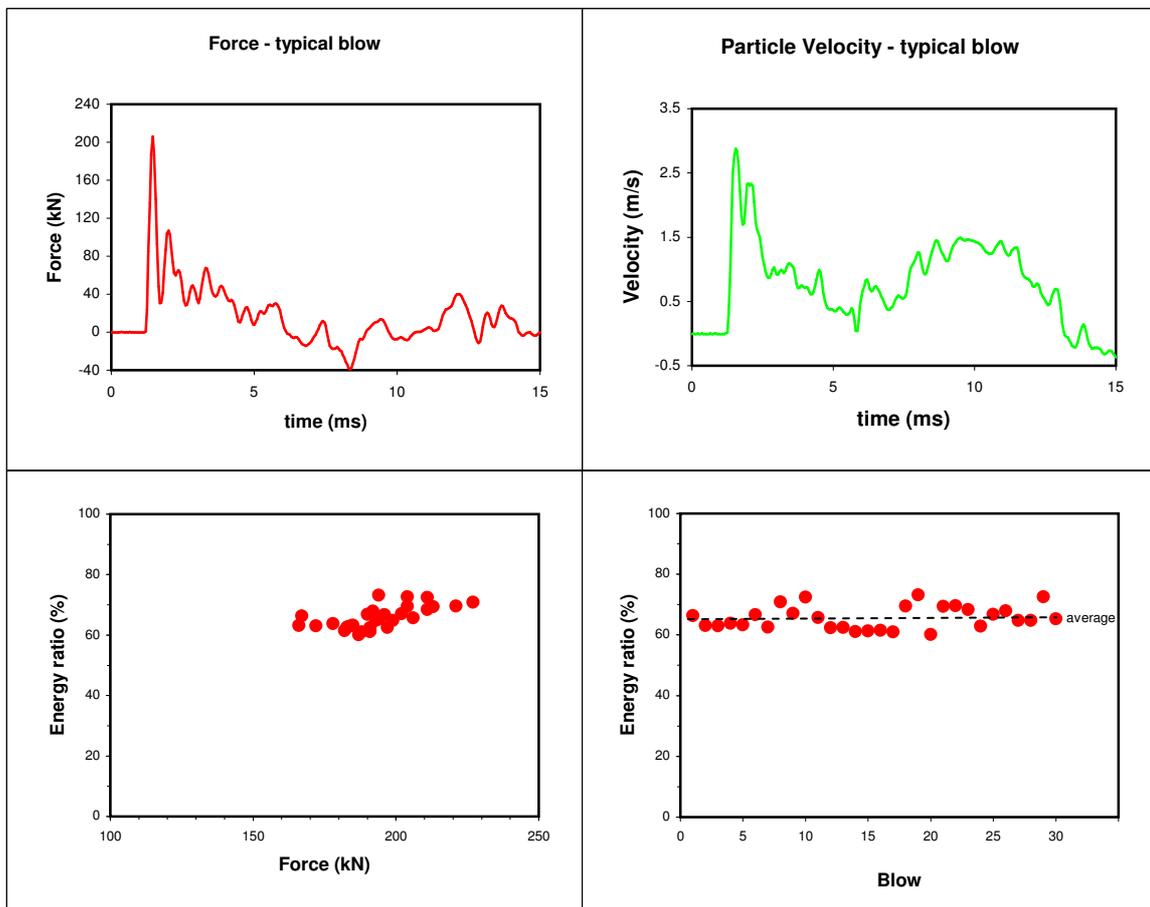
**Hammer ID:** DC1  
**Hammer mass (m)** 63.5 kg  
**Fall height (h)** 0.76 m  
**Test type:** SPT  
**Manufacturer:** Archway  
**Model:** Automatic Trip Hammer

**Test rod type:** BW

**Rig:** Dando 3000  
**Rig ID:** CT3  
**Type:** Cable Percussion  
**Foreman:** D Clay

**Remarks:**

Data obtained from test carried out in BH1, located in SM Doncaster yard. Test carried out at depth of 9.20 mbgl, with a total blow count of 30. Energy determined from every blow.



**Theoretical energy ( $E_{theor}$ )** =  $m \times g \times h$  = **0.473 kN-m (473 J)**

**Measured energy ( $E_{meas}$ )** average of 30 blows = **0.311 kN-m**

**Energy ratio** =  $\frac{E_{meas}}{E_{theor}}$  = **66 %**

Test carried out by: Rob Cooke

Test carried out in accordance with BS EN ISO 22476-3:2005

Signed for issue:

Equipment used: SPT Analyzer Serial No. 4032T



# SPT Hammer Energy Test Report

in accordance with BSEN ISO 22476-3:2005

**J.B. Site Investigations**  
**Windmill Way West**  
**Ramparts Business Park**  
**BERWICK-upon-TWEED**  
**TD15 1TB**

SPT Hammer Ref: JB.15  
Test Date: 24/06/2011  
Report Date:  
File Name: JB15.spt  
Test Operator: AH

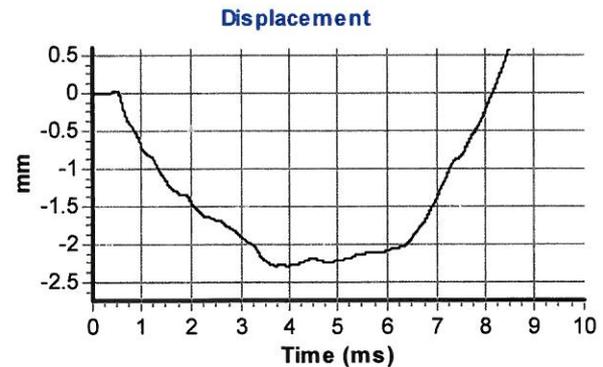
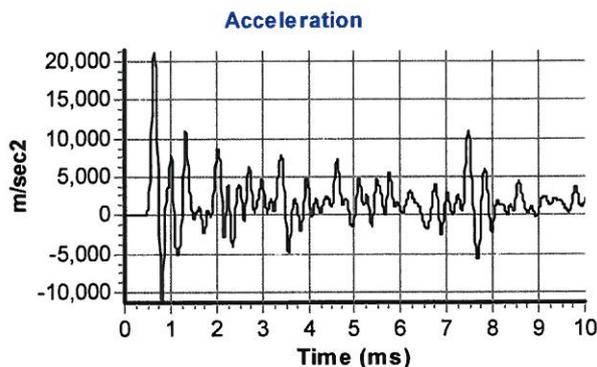
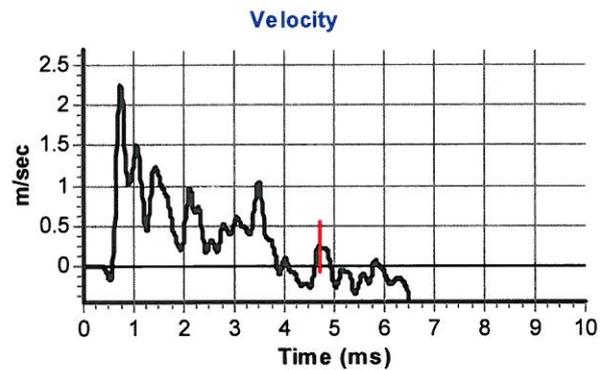
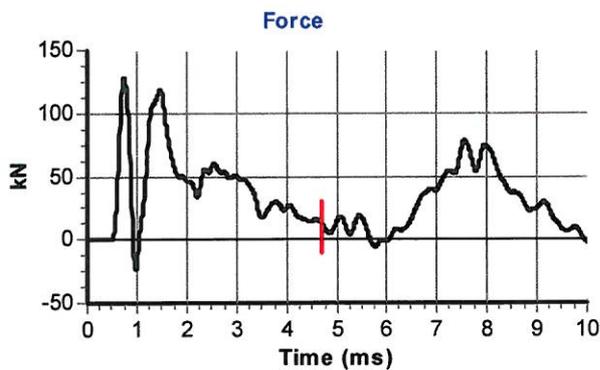
## Instrumented Rod Data

Diameter  $d_r$  (mm): 54  
Wall Thickness  $t_r$  (mm): 6.5  
Assumed Modulus  $E_a$  (GPa): 208  
Accelerometer No.1: 6454  
Accelerometer No.2: 6456

## SPT Hammer Information

Hammer Mass  $m$  (kg): 63.5  
Falling Height  $h$  (mm): 760  
SPT String Length  $L$  (m): 14.0

## Comments / Location



## Calculations

Area of Rod A ( $\text{mm}^2$ ): 970  
Theoretical Energy  $E_{\text{theor}}$  (J): 473  
Measured Energy  $E_{\text{meas}}$  (J): 277

**Energy Ratio  $E_r$  (%):** **59**

  
Signed: Jeff Burnlees  
Title: Proprietor

The recommended calibration interval is 12 months

# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 05/10/2011 End 06/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 24.60m to 24.60m to 34.10m Diameter 200mm Casing Depth 24.60m	Ground Level +44.48 mOD Coordinates E 486765.86 National Grid N 512240.04 Chainage
--	------------------------------------	---	--	---

Samples and Tests				Strata			Depth, Level (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.10 0.10-0.50	D 1 B 2	0.00-1.20 m Hand excavated inspection pit.			Orangish and light brown sandy slightly gravelly CLAY with frequent rootlets. Gravel is subangular fine of coal and mudstone. (TOPSOIL)	(0.50)			
0.50 0.50-1.00	D 3 B 4					0.50 +43.98			
1.00 1.20-1.65	ES 4A U 5	22 blows			Firm orangish yellow and brown slightly gravelly sandy CLAY. Gravel is angular to subangular fine to medium of various lithologies.	(1.10)			
1.65-1.85	D 6					1.60 +42.88			
2.00-2.45 2.00-2.45 2.00	D 7 B 8 ES 8A				Firm, becoming stiff below 6.00m, dark brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, locally coarse, of various lithologies predominantly sandstone and quartzite. (GLACIAL TILL)				
3.00-3.45	U 9	28 blows 400 mm rec							
3.45-3.65	D 10								
4.00-4.45 4.00-4.45	D 11 B 12								
4.45-4.90	SPT S	N=6 (1,1/1,2,2,1)	3.10	dry					
5.00-5.45	U 13	24 blows	3.40	dry					
5.45-5.65	D 14								
5.65-6.10 5.65-6.10 5.65-6.10	SPT S D 15 B 16	N=12 (1,2/2,3,3,4)	3.10	dry		(8.40)			
6.50-6.95	U 17	28 blows	6.10	dry					
6.95-7.15	D 18								
7.15-7.60 7.15-7.60 7.15-7.60	SPT S D 19 B 20	N=12 (5,3/2,3,3,4)	6.10	dry					
8.00-8.45	U 21	27 blows 400 mm rec	7.60	dry					
8.45-8.65	D 22								
8.65-9.10 8.65-9.10 8.65-9.10	SPT S D 23 B 24	N=15 (2,2/3,3,4,5)	7.60	dry					
9.50-9.95	U 25	27 blows	9.10	dry					
9.95-10.15	D 26								

7.00-7.15 m 1  
No. sandstone  
cobble

Groundwater Entries	Depth Related Remarks *	Chiselling
No. Struck (m) Post strike behaviour	From to (m)	Depths (m) Time Tools used

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH1 Sheet 1 of 4
--	--	------------------------------



# Borehole Log



Soil Mechanics

<b>Drilled</b> DC/CL <b>Logged</b> CH/JMH <b>Checked</b> PH	<b>Start</b> 05/10/2011 <b>End</b> 06/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	<b>Depth from</b> 0.00m <b>to</b> 24.60m <b>Diameter</b> 200mm <b>Casing Depth</b> 24.60m	<b>Ground Level</b> +44.48 mOD <b>Coordinates</b> E 486765.86 <b>National Grid</b> N 512240.04 <b>Chainage</b>
---	--	---	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 1)				
10.15-10.60 10.15-10.60 10.15-10.60	SPT S D 27 B 28	N=17 (2,2/3,4,4,6)	9.10	dry	Stiff reddish brown, locally greyish brown, slightly sandy slightly gravelly, locally grading to gravelly, CLAY. Gravel is angular to subrounded fine to medium of various lithologies predominantly mudstone. (GLACIAL TILL)	10.00	+34.48		
11.00-11.45	U 29	33 blows	10.60	dry					
11.45-11.65	D 30								
11.65-12.10 11.65-12.10 11.65-12.10	SPT S D 31 B 32	N=19 (2,3/4,4,5,6)	10.60	dry			(4.25)		
12.50-12.95	U 33	34 blows	12.10	dry					
12.95-13.15	D 34								
13.15-13.60 13.15-13.60 13.15-13.60	SPT S D 35 B 36	N=19 (2,3/3.5,5,6)	12.10	dry					
14.00-14.45	U 37	32 blows	13.60	dry					
14.45-14.65	D 38				Light brown silty fine to medium SAND. (GLACIAL TILL)	14.25	+30.23		
14.65-15.10 14.65-15.10 14.65-15.10	SPT S D 39 B 40	N=24 (2,4/5,6,6,7)	13.60	dry	Stiff dark brown thinly laminated fissured slightly sandy slightly gravelly CLAY. Fissures are very closely spaced, randomly orientated, smooth, polished. Gravel is angular to subrounded fine to medium of various lithologies with occasional 15mm pockets of light brown fine sand. (GLACIAL TILL)	14.45	+30.03		
15.50-15.95	U 41	41 blows	15.10	dry					
15.95-16.15	D 42								
16.15-16.60 16.15-16.60 16.15-16.60	SPT S D 43 B 44	N=24 (4,5/4,6,6,8)	15.10	15.70	Dark brown sandy very clayey angular to subrounded fine to medium GRAVEL of various lithologies predominantly mudstone. (Possibly weathered MUDSTONE)	16.00	+28.48		
17.00-17.45	U 45	45 blows	16.60	dry	Stiff dark brown slightly gravelly sandy CLAY. Gravel is angular to subangular fine to medium, locally coarse, of various lithologies predominantly mudstone. (Possibly weathered MUDSTONE)	16.20	+28.28		
17.45-17.65	D 46						(1.90)		
17.65-18.10 17.65-18.10 17.65-18.10	SPT S D 47 B 48	N=33 (4,5/6,7,10,10)	16.60	dry					
18.10	D 49								
18.50-18.95 18.50-18.95 18.70	SPT C B 50 D 51	N=26 (3,4/5,6,6,9)	18.10	dry	Medium dense dark brown sandy clayey GRAVEL. Gravel is angular to rounded fine to coarse of various lithologies predominantly mudstone. (Possibly weathered MUDSTONE)	18.10	+26.38		
19.50-19.95	U 52	60 blows	19.40	dry	Stiff to very stiff dark brown, becoming greyish brown below 20.50m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to medium of various lithologies predominantly mudstone and sandstone. (GLACIAL TILL)	18.70	+25.78		
19.95-20.15	D 53								
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 23.90 m				

<b>Groundwater Entries</b> No. Struck Post strike behaviour 1 16.00 Rose to 15.70 m after 20 minutes.	Depth sealed (m) -	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
---	-----------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH1 Sheet 2 of 4
--	--	------------------------------



# Borehole Log



Soil Mechanics

<b>Drilled</b> DC/CL <b>Logged</b> CH/JMH <b>Checked</b> PH	<b>Start</b> 05/10/2011 <b>End</b> 06/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	<b>Depth from</b> 0.00m <b>to</b> 24.60m <b>Diameter</b> 200mm <b>Casing Depth</b> 24.60m	<b>Ground Level</b> +44.48 mOD <b>Coordinates</b> E 486765.86 <b>National Grid</b> N 512240.04 <b>Chainage</b>
---	--	---	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 2)				
20.15-20.60 20.15-20.60 20.15-20.60	SPT S D 54 B 55	N=38 (4,6/8,9,10,11)	19.40	dry	Stiff to very stiff dark brown, becoming greyish brown below 20.50m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to medium of various lithologies predominantly mudstone and sandstone. (GLACIAL TILL)				
21.00-21.45	U 56	70 blows	19.40	dry		(5.20)			
21.45-21.65	D 57								
21.65-22.10 21.65-22.10 21.65-22.10	SPT S D 58 B 59	N=29 (11,7/6,7,8,8)	19.40	dry					
22.50-22.95	U 60	60 blows 400 mm rec	19.40	dry					
22.95-23.15	D 61								
23.15-23.60 23.15-23.60 23.15-23.60	SPT S D 62 B 63	N=33 (4,6/7,8,9,9)	19.40	dry					
23.90	D 64						23.90	+20.58	
24.60-24.86 24.60-24.86	SPT S D 65	50 (7,10/18,32 for 35mm)	05/10/2011 19.40 06/10/2011 19.40	dry dry 1030 22.30		Light grey, locally dark grey, thinly laminated silty MUDSTONE. Recovered as angular to subangular fine to coarse gravel.		(0.96)	
24.60-25.60	55 0 0	NA NA NA				Extremely weak thinly laminated grey MUDSTONE. Weathering is complete loss of structure to gravelly clay.	24.60-25.05 m AZCL	24.86	+19.62
25.60-26.60	100 52 43				Weak thinly laminated grey SILTSTONE with rare brown sandstone bands. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 5mm and localised reduction in strength to very weak. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.	25.55-25.89 m NI 26.34-26.50 m 1no. vertical, planar, smooth fracture 26.50-26.64 m NI	25.55	+18.93	
26.60-28.10	100 81 29	NI 90 300			Weak thinly inter-laminated grey SILTSTONE and light brown fine grained SANDSTONE. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 2mm. Fractures are subhorizontal, very closely to closely spaced, planar, smooth.	26.85-27.63 m NI 27.60-27.63 m NI 27.74-27.78 m recovered as sandy clay 27.99-28.05 m NI	26.85	+17.63	
28.10-29.60	100 77 73	NI 120 360			Weak thinly laminated grey SILTSTONE with rare brown sandstone bands. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 5mm and localised reduction in strength to very weak. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.	28.64-28.70 m NI 29.08-29.17 m NI	27.40	+17.08	
					Medium strong thinly laminated brown and grey medium grained SANDSTONE. Weathering is an orangish brown	29.50-29.95 m 1no. subvertical, planar, rough fracture 29.55-29.60 m NI	27.90	+16.58	
					Flush: 24.60-34.10 air mist, 100 %		29.90	+14.58	
Depth	ICR SCR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 30.50 m			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
---	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH1 Sheet 3 of 4
--	--	------------------------------

# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 05/10/2011 End 06/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 24.60m to 24.60m to 34.10m Diameter 200mm Casing Depth 24.60m	Ground Level +44.48 mOD Coordinates E 486765.86 National Grid N 512240.04 Chainage
--	------------------------------------	---	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments	
Depth	TCR SCR ROD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 3)				
29.60-31.10	92 80 70	NI 50 240				27.90m - 29.90m : discolouration on fracture surfaces penetrating up to 30mm. Fractures are: 1) subhorizontal, closely to medium spaced, planar, smooth. 2) 50 degrees, medium spaced, planar, rough.	29.60-29.72 m AZCL 30.32-30.38 m NI 30.38-30.50 m recovered as gravelly clay	(0.60)	XXXXXX XXXXXX XXXXXX XXXXXX	
		NI 70 180						(1.00)	XXXXXX XXXXXX XXXXXX XXXXXX	
31.10-32.60	100 35 20					29.90m - 30.50m : Weak thinly laminated grey SILTSTONE. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 5mm. Fractures are subhorizontal, closely spaced, planar, smooth.	31.10-31.30 m 1no. vertical, planar, rough fracture 31.30-31.38 m NI	31.50 +12.98	XXXXXX XXXXXX XXXXXX XXXXXX	
		NI NI 100				Weak thinly laminated grey fine grained SANDSTONE. Weathering is an orangish brown discolouration on fracture surfaces penetrating up to 10mm. Fractures are subhorizontal, closely spaced, planar, smooth.	32.60-33.00 m AZCL	(2.60)	XXXXXX XXXXXX XXXXXX XXXXXX	
32.60-34.10	73 35 7			06/10/2011 24.60		Extremely weak thinly laminated grey MUDSTONE. Weathering is loss of structure to gravel and localised orangish brown discolouration on fracture surfaces. Fractures are: 1) recovered as non-intact 2) subhorizontal, very closely to closely spaced, planar, rough.	33.12-33.44 m 1no. 75 degree planar, rough fracture		XXXXXX XXXXXX XXXXXX XXXXXX	
EXPLORATORY HOLE ENDS AT 34.10 m								34.10 +10.38		

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
---	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH1 Sheet 4 of 4
--	--	------------------------------



# Borehole Log



Soil Mechanics

<b>Drilled</b> DC/CL <b>Logged</b> CH/JMH <b>Checked</b> PH	<b>Start</b> 03/10/2011 <b>End</b> 05/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	<b>Depth from</b> 0.00m <b>to</b> 17.00m <b>Diameter</b> 200mm <b>Casing Depth</b> 17.00m	<b>Ground Level</b> +45.56 mOD <b>Coordinates</b> E 486709.85 <b>National Grid</b> N 512274.24 <b>Chainage</b>
---	--	---	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.10	D 1	0.00-1.20 m Hand excavated inspection pit.			Orangish brown slightly sandy CLAY. Sand is fine to coarse. (TOPSOIL)	(0.50)			
0.50	D 2				Stiff orangish brown, becoming dark brown below 3.00m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to coarse of various lithologies. (GLACIAL TILL)	0.50	+45.06		
0.50-1.00	B 3								
1.00	ES 3A								
1.20-1.65	U 4	38 blows		dry					
1.65-1.85	D 5								
2.00-2.45	SPT S	N=7 (1,1/1,2,2,2)	1.60	dry					
2.00-2.45	D 6								
2.00-2.45	ES 6A								
2.00-2.45	B 7								
3.00-3.45	U 8	50 blows		dry					
3.45-3.65	D 9								
4.00-4.45	SPT S	N=11 (1,2/3,2,3,3)	3.10	dry					
4.00-4.45	D 10								
4.00-4.45	B 11								
5.00-5.45	U 12	30 blows 400 mm rec	4.60	dry					
5.45-5.65	D 13				5.45-5.60 m pocket of dark brown very sandy slightly gravelly clay. Gravel is subangular to subrounded fine to medium of various lithologies				
6.00-6.45	SPT S	N=11 (1,2/2,2,3,4)	5.80	dry					
6.00-6.45	D 14								
6.00-6.45	B 15								
7.00-7.45	U 16	38 blows	5.80	dry					
7.45-7.65	D 17								
8.00-8.45	U 18	29 blows	7.60	dry		(15.20)			
8.45-8.65	D 19								
9.00-9.45	U 20	35 blows	7.60	dry					
9.45-9.65	D 21								
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 15.70 m				

<b>Groundwater Entries</b> No. Struck (m) Post strike behaviour	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
--	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH2 Sheet 1 of 5
--	--	------------------------------



# Borehole Log



Soil Mechanics

<b>Drilled</b> DC/CL <b>Logged</b> CH/JMH <b>Checked</b> PH	<b>Start</b> 03/10/2011 <b>End</b> 05/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	<b>Depth from</b> 0.00m <b>to</b> 17.00m <b>Diameter</b> 200mm <b>Casing Depth</b> 17.00m 17.00m 26.20m 150mm 26.20m 26.20m 40.20m 121mm 26.20m	<b>Ground Level</b> +45.56 mOD <b>Coordinates</b> E 486709.85 <b>National Grid</b> N 512274.24 <b>Chainage</b>
---	--	---	--	---

Samples and Tests					Strata		Depth, Level (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 1)				
10.00-10.45	U 22	35 blows	7.60	dry	Stiff orangish brown, becoming dark brown below 3.00m, slightly sandy slightly gravelly CLAY. Gravel is angular to rounded fine to coarse of various lithologies. (GLACIAL TILL)				
10.45-10.65	D 23					10.50 m very sandy			
11.00-11.45	U 24	30 blows	7.60	dry					
11.45-11.65	D 25								
12.00-12.45	U 26	36 blows	7.60	dry			12.00 m very sandy		
12.45-12.65	D 27								
13.00-13.65	B 28	55 blows No recovery	7.60	dry					
13.00-13.45	U NR								
14.00-14.45	U 29	35 blows	7.60	dry					
14.45-14.65	D 30								
15.00-15.45	U 31	40 blows 400 mm rec	7.60	dry					
15.45-15.65	D 32								
15.70	D 33								
15.90	W 36	40 blows 400 mm rec	7.60	dry	Dark brown sandy very clayey GRAVEL. Gravel is angular to rounded fine to coarse of various lithologies predominantly mudstone and sandstone. (GLACIAL TILL)	15.70	+29.86		
16.00-16.45	U 34					15.90	+29.66		
16.45-16.65	D 35		03/10/2011 7.60	dry					
17.00-17.45	U 37	30 blows 400 mm rec	04/10/2011 7.60	0800 13.70		(2.00)			
17.45-17.65	D 38		17.00	15.20					
17.90	W 39	N=37 (3,5/9,9,10,9)	18.00	14.50	Orange brown very sandy clayey angular to subrounded fine to medium GRAVEL of various lithologies, predominantly quartzite and flint. (GLACIAL TILL)	17.90	+27.66		
18.00-18.45	SPT S					(0.60)			
18.00-18.45	D 40								
18.50	D 42				Dark brown thinly laminated fissured slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium of various lithologies. Fissures are very closely spaced, randomly orientated, smooth, polished. (GLACIAL TILL)	18.50	+27.06		
19.00-19.45	U 43	55 blows	18.80	dry		(0.50)			
19.45-19.65	D 44				Stiff, locally very stiff, dark brown	19.00	+26.56		
					Stratum continues to 25.40 m				

Groundwater Entries				Depth Related Remarks *		Chiselling		
No.	Struck (m)	Post strike behaviour	Depth sealed (m)	From	to (m)	Depths (m)	Time	Tools used
1	15.90	Rose to 14.90 m after 20 minutes.	-					
2	17.90	Rose to 14.30 m after 20 minutes. Medium inflow.	-					

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	<b>Project</b> SANDSEND SLOPE STABILISATION <b>Project No.</b> A1077-11 <b>Carried out for</b> Balfour Beatty Living Places	<b>Borehole</b> <b>BH2</b> Sheet 2 of 5
--	---	---



# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 03/10/2011 End 05/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 17.00m 17.00m 26.20m	Diameter 200mm 150mm 121mm	Casing Depth 17.00m 26.20m 26.20m	Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage
--	------------------------------------	--	---	----------------------------------	---	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 2)				
20.00-20.45	U 45	43 blows 400 mm rec	19.60	dry	slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, occasionally coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)				
20.45-20.65	D 46								
21.00-21.45	U 47	50 blows 400 mm rec	19.60	dry					
21.45-21.65	D 48								
22.00-22.45	U 49	60 blows 350 mm rec	19.60	dry			(6.40)		
22.45-22.65	D 50								
23.00-23.45	U 51	70 blows 400 mm rec	19.60	dry					
23.45-23.65	D 52								
24.00-24.45	U 53	65 blows	19.60	dry					
24.45-24.65	D 54								
25.00-25.45	U 55	65 blows 400 mm rec	19.60	dry					
25.45-25.65	D 56								
26.10-26.29	SPT S	50 (16,9 for 40mm/50)	04/10/2011 19.60	dry	Very weak light grey and green, locally weathered orange, thinly laminated MUDSTONE. Recovered as angular to subangular fine to coarse gravel. (Weathered MUDSTONE)	25.40 +20.16	(0.90)		
26.10-26.30	D 57		05/10/2011 19.60	0800 dry					
26.20-27.20	66 64 54				Weak locally very weak thinly laminated brown fine grained SANDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, closely to medium spaced, planar, rough to smooth.	26.20-26.54 m AZCL	26.30 +19.26		
27.20-28.50	85 30 23 NI 110 240					26.97-27.20 m 1no 75 degree, planar, rough fracture 27.20-27.60 m AZCL 27.60-27.67 m NI			
28.50-30.20	85 69 59					27.96-28.10 m recovered as gravelly clay 28.30-28.47 m recovered as gravelly clay 28.53-28.70 m 1no subvertical, planar, rough fracture 28.70-28.75 m AZCL 28.75-29.10 m NI 29.20-29.39 m 1no. vertical, planar, smooth fracture 29.69-29.95 m 1no 70 degree	(3.65)		
							29.95 +15.67		
Depth	ICR SCR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 32.00 m			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used 25.90 -26.10 30 mins
---	------------------	--	---

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH2 Sheet 3 of 5
--	--	------------------------------



# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 03/10/2011 End 05/10/2011	Equipment, Methods and Remarks Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 17.00m 17.00m to 26.20m 26.20m to 40.20m	to 17.00m 26.20m 40.20m	Diameter 200mm 150mm 121mm	Casing Depth 17.00m 26.20m 26.20m	Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage
--	------------------------------------	--	--	-------------------------------	----------------------------------	---	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 3)			
30.20-31.70	73 36 36	NI 120 270				Very weak thinly laminated grey fine grained SANDSTONE. Fractures are subhorizontal, closely to medium spaced, planar, smooth.	planar, smooth fracture		
							30.92-31.16 m NI 31.07-31.52 m NI 1no subvertical, planar, rough fracture 31.52-31.64 m NI	(2.05)	
31.70-33.20	100 81 75	100 150 220				Extremely weak thinly laminated grey MUDSTONE. Fractures are subhorizontal, closely to medium spaced, planar, smooth.		32.00 +13.56	
							32.64-32.81 m NI 1no. 65 degree, planar, smooth fracture	(1.10)	
33.20-34.70	67 42 22		Flush: 26.20-40.20 air mist, 100 %			Very weak to extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravel. Fractures are subhorizontal, very closely to closely spaced, planar rough to smooth.		33.10 +12.46	
							33.10-33.20 m NI 33.20-33.70 m AZCL		
34.70-36.20	73 26 0						33.88-34.00 m NI 34.07-34.12 m NI 34.43-34.46 m NI 34.70-34.88 m AZCL 34.88-34.92 m NI 35.14-35.16 m NI 35.38-35.67 m NI		
36.20-37.70	80 21 7	NI 70 150					35.67-35.98 m NI 1no. subvertical, undulating, smooth fracture 35.77-35.80 m NI 35.98-36.03 m NI 36.20-36.50 m AZCL 36.50-36.65 m NI 36.65-37.00 m multiple randomly orientated fractures 37.24-37.44 m NI 37.44-37.58 m recovered as gravelly clay 37.70-37.75 m AZCL	(7.60)	
37.70-39.20	97 67 7						38.28-38.44 m NI 1no 75 degree, planar, smooth fracture 38.44-38.54 m NI 38.91-38.98 m NI 38.98-39.20 m NI 1no. vertical, planar, rough fracture 39.20-39.33 m AZCL 39.46-39.50 m NI 39.59-39.62 m NI		
39.20-40.70	91 45 17								
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 40.70 m			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
---	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH2 Sheet 4 of 5
--	--	------------------------------



# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH/JMH Checked PH	Start 03/10/2011 End 05/10/2011	<b>Equipment, Methods and Remarks</b> Dando 3000 and Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 17.00m 17.00m 26.20m	to 17.00m 26.20m 40.20m	Diameter 200mm 150mm 121mm	Casing Depth 17.00m 26.20m 26.20m	Ground Level +45.56 mOD Coordinates E 486709.85 National Grid N 512274.24 Chainage
--	------------------------------------	---	---	-------------------------------	----------------------------------	---	---

Samples and Tests				Strata				Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 4)				
				05/10/2011 26.20	dry	Very weak to extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravel. Fractures are subhorizontal, very closely to closely spaced, planar rough to smooth.	39.96-40.01 m NI 40.23-40.32 m NI 40.48-40.55 m NI	40.70	+4.86	
						EXPLORATORY HOLE ENDS AT 40.70 m				

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
---	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH2 Sheet 5 of 5
--	--	------------------------------



# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH Checked PH	Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 21.10m Diameter 200mm Casing Depth 21.10m	Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage
--	--	---	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.10 0.10-0.50 0.30	D 1 B 2 ES 2A	0.00-1.20 m Hand excavated inspection pit.			Orangish brown very sandy CLAY. (TOPSOIL)	(0.50)			
0.50-1.00	B 3				Light brownish and orange slightly gravelly sandy CLAY.	0.50 +45.45 (0.90)			
1.20-1.65	U 4	31 blows		dry		1.40 +44.55			
1.65-1.85 1.85-2.30 1.85-2.30 1.85-2.30 2.00	D 5 SPT S D 6 B 7 ES 7A	N=11 (1,1/2,2,3,4)		dry	Stiff, becoming firm below 4.50m, reddish brown, locally dark brown, slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, occasionally coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)				
2.50-2.95	U 8	38 blows	1.60	dry					
2.95-3.15 3.15-3.60 3.15-3.60	D 9 SPT S D 10 B 11	N=18 (2,3/4,4,5,5)	1.60	dry					
4.00-4.45	U 12	28 blows	3.10	dry					
4.45-4.65 4.65-5.10 4.65-5.10 4.65-5.10	D 13 SPT S D 14 B 15	N=10 (1,2/2,2,3,3)	3.10	dry				SPIE	
5.50-5.95	U 16	28 blows	4.50	dry					
5.95-6.15 6.15-6.60 6.15-6.60 6.15-6.60	D 17 SPT S D 18 B 19	N=10 (1,2/2,2,3,3)	4.50	dry		(9.40)			
7.00-7.45	U 20	Blows not recorded							
7.45-7.65 7.65-8.10 7.65-8.10 7.65-8.10	D 21 SPT S D 22 B 23	N=10 (1,2/2,2,3,3)	6.10	dry					
8.50-8.95	U 24	28 blows	7.60	dry					
8.95-9.15 9.15-9.60 9.15-9.60 9.15-9.60	D 25 SPT S D 26 B 27	N=15 (1,2/3,3,4,5)	7.60	dry				9.15 m 1 No. quartzite cobble	
Depth	Type & No	Records	Date Casing	Time Water	Stratum continues to 10.80 m				

Groundwater Entries No. Struck (m) Post strike behaviour	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
--	------------------	--	---

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column. Scale 1:50	(c) ESG www.esg.co.uk 426.4813/12/2011 15:24:40	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH3 Sheet 1 of 5
--	--	--	------------------------------

# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH Checked PH	Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 21.10m to 21.10m Diameter 200mm Casing Depth 21.10m	Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage
--	------------------------------------	--	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 1)				
10.00-10.45	U 28	34 blows	9.10	dry	Stiff, becoming firm below 4.50m, reddish brown, locally dark brown, slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, occasionally coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)	10.60 m 1 No. sandstone cobble	10.80	+35.15	
10.45-10.65	D 29								
10.65-10.93	SPT S	50 (2,18/25,25 for 50mm)	9.10	dry					
10.65-10.90	D 30								
10.65-10.90	B 31								
11.20	D 32				Stiff to very stiff orangish light grey slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of mudstone and siltstone. Below 12.50m, becoming dark grey with bands up to 15mm thick of weathered black vitreous coal.	(3.10)			
11.50-11.95	SPT S	N=26 (3,4/4,6,8,8)	10.60	dry					
11.50-11.95	D 33								
11.50-11.95	B 34								
12.50-12.95	SPT S	N=26 (3,5/5,6,7,8)	10.60	dry					
12.50-12.95	D 35								
12.50-12.95	B 36								
13.50-13.95	SPT S	N=31 (3,4/5,8,8,10)	10.60	dry					
13.50-13.95	D 37								
13.50-13.95	B 38								
14.50-14.95	U 39	45 blows	13.60	dry	Stiff to very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to medium of various lithologies predominantly mudstone. (GLACIAL TILL)		13.90	+32.05	
14.95-15.15	D 40				Dark brownish grey silty slightly gravelly CLAY. Gravel is subangular to subrounded fine of mudstone. (GLACIAL TILL)		14.90	+31.05	
15.15-15.60	SPT S	N=26 (3,5/7,6,6,7)	13.60	14.40					
15.15-15.60	D 41								
15.15-15.60	B 42								
16.00-16.45	U 43	45 blows	15.80	dry	Very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, rarely coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)		15.20	+30.75	
16.45-16.55	D 44								
16.65-17.10	SPT S	N=28 (3,4/6,6,7,9)	15.80	dry					
16.65-17.10	D 45								
16.65-17.10	B 46								
17.50-17.95	U 47	60 blows	16.60	dry					
17.95-18.15	D 48								
18.15-18.60	SPT S	N=35 (4,5/7,9,9,10)	16.60	dry					
18.15-18.60	D 49								
18.15-18.60	B 50								
19.00-19.45	U 51	62 blows	18.10	dry					
19.45-19.65	D 52								
19.65-20.10	SPT S	N=35 (4,5/7,8,9,11)	18.10	dry					
19.65-20.10	D 53								
19.65-20.10	B 54								

Depth Type & No Records Date Casing Time Water	Stratum continues to 20.10 m	Groundwater Entries No. Struck Post strike behaviour Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
		1 14.90 Rose to 14.40 m after 20 minutes. Slow inflow. -		10.90 -11.10 30 mins

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole <b>BH3</b> Sheet 2 of 5
--	--	--



# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH Checked PH	Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 21.10m to 21.10m to 40.10m Diameter 200mm Casing Depth 21.10m	Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage
--	--	--	--	---

Samples and Tests				Strata			Depth, Level (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description (Continued from Sheet 2)				
20.10	D 55		19.60	dry	Very stiff dark brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium, rarely coarse, of various lithologies predominantly mudstone. (GLACIAL TILL)	20.10	+25.85		
20.50-21.00	B 56		07/10/2011 19.60	0800 dry			(1.00)		
21.20-21.62 21.20-21.60		SPT S 50 (8,8/12,13,15,10 for 45mm) D 57	19.60	dry	Very weak greyish yellow thinly laminated MUDSTONE. Generally recovered as angular to subangular fine to medium gravelly clay. (Weathered MUDSTONE)	21.10	+24.85		
21.10-22.10	38 20 0		07/10/2011 19.60	0800 dry		21.10-21.62 m AZCL			
22.10-23.60	95 33 11		19.60	7.90	Medium strong to weak thinly laminated brown fine to medium grained SANDSTONE with occasional grey siltstone bands. Weathering is loss of structure to gravel and orangish brown discolouration on fracture surfaces. Fractures are: 1 No. recovered as NI. 2 No. subhorizontal, very closely to closely spaced, planar, smooth.	22.10-22.17 m AZCL 22.17-22.20 m NI 22.24-22.40 m NI 22.75-22.98 m NI 23.00-23.50 m 1 23.04-23.27 m NI	(2.50)		
23.60-25.10	73 53 39	NI NI 170			Weak to very weak thinly laminated grey SILTSTONE with occasional brown fine grained sandstone. Weathering is localised loss of structure to gravel. Fractures are subhorizontal, closely to medium spaced, planar, smooth.	24.05-24.19 m 1 24.41-24.70 m 24.70-25.10 m AZCL	(2.05)		
25.10-26.60	100 57 57	500 500 500			Very weak thinly laminated grey MUDSTONE.	25.50-25.60 m NI	25.65	+20.30	(0.50)
26.60-28.10	87 23 23	NI NI 150			Weak to very weak thinly laminated brown medium grained SANDSTONE. Weathering is loss of structure to gravel. Fractures are: 1 No. recovered as NI. 2 No. subvertical, undulating, rough.	26.60-26.90 m AZCL	26.15	+19.80	(1.95)
28.10-29.60	100 71 59	NI 100 150			Weak thinly laminated brown fine grained SANDSTONE with occasional siltstone bands. Fractures are: 1 No. subhorizontal, very closely to closely spaced, planar, smooth. 2 No. subvertical, undulating, rough.	28.10-28.14 m NI 28.24-28.27 m NI 28.43-28.48 m NI	28.10	+17.85	(2.00)
						29.53-29.60 m NI 29.60-29.90 m AZCL 29.90-30.00 m NI			
Depth	ICR RSD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 30.10 m			

Groundwater Entries No. Struck (m) Post strike behaviour	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m)	Time	Tools used
--	------------------	--	--------------------------	------	------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANSEND SLOPE STABILISATION	Borehole BH3
Scale 1:50	Project No. A1077-11	Sheet 3 of 5
(c) ESG www.esg.co.uk 426.4813/12/2011 15:24:42	Carried out for Balfour Beatty Living Places	

# Borehole Log



Soil Mechanics

Drilled DC/CL Logged CH Checked PH	Start 06/10/2011 End 10/10/2011	Equipment, Methods and Remarks Dando 3000 & Beretta T51. Cable percussion boring followed by rotary core drilling (PWF size) using air mist flush. SPT: hammer ID DC1, rods BW.	Depth from 0.00m to 21.10m to 21.10m to 40.10m Diameter 200mm Casing Depth 21.10m	Ground Level +45.95 mOD Coordinates E 486620.13 National Grid N 512295.06 Chainage
--	------------------------------------	--	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	TCR SCR ROD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 3)			
29.60-31.10	80 53 35		Flush: 21.10-40.10 air mist, 100 %			Weak thinly laminated brown fine grained SANDSTONE with occasional siltstone bands. Fractures are: 1 No. subhorizontal, very closely to closely spaced, planar, smooth. 2 No. subvertical, undulating, rough.	30.10 +15.85		
31.10-32.60	87 39 39	NI 200 400				Very weak thinly laminated grey MUDSTONE. Weathering is loss of structure to gravel and occasional orange to brown discolouration on fracture surfaces.	(2.55)		
32.60-34.10	100 63 27					Very weak, locally extremely weak, thinly laminated dark grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are: 1 No. subhorizontal, very closely to medium spaced, planar, rough to smooth. 2 No. subvertical, planar, smooth.	32.65 +13.30		
34.10-35.60	77 43 37								
35.60-37.10	93 73 23	NI 80 240					(7.45)		
37.10-38.60	100 43 17								
38.60-40.10	100 64 20								
				10/10/2011					
Depth	TCR SCR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 40.10 m			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
---	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH3 Sheet 4 of 5
--	--	------------------------------





# Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH Checked PH	Start 04/10/2011 End 06/10/2011	Equipment, Methods and Remarks Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.	Depth from 0.00m to 5.00m to 5.00m to 12.00m Diameter 150mm to 146mm Casing Depth 5.00m to 5.30m	Ground Level +14.59 mOD Coordinates E 486819.85 National Grid N 512318.46 Chainage
--	------------------------------------	---	---	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.10	D 1	0.00-1.20 m Hand excavated inspection pit.			MACADAM. (MADE GROUND) (Foreman's description)	0.10 +14.49			
0.10-0.15	B 2					0.15 +14.44			
0.20	D 3					(0.50)			
0.20-0.30	B 4				Dark grey sandy clayey GRAVEL. Gravel is angular to subangular fine to medium of ash, macadam and sandstone. Slight hydrocarbon odour. (MADE GROUND)	0.65 +13.94			
0.35	D 5								
0.35-0.65	B 6								
0.70	D 7								
0.80-1.10	B 8								
1.00	ES 8A								
1.20	D 9								
1.20-1.65	U NR	70 blows No recovery	1.20	dry	Creamish yellow, becoming light grey, very sandy silty angular to subangular fine to medium GRAVEL of limestone. (MADE GROUND)				
1.70	D 11								
2.00-2.45	SPT S	N=4 (1,1/1,1,1,1)	2.00	dry	Firm orangish brown slightly sandy slightly gravelly CLAY. Gravel is angular to subrounded fine to medium of various lithologies including coal, ash and clinker. (MADE GROUND)	(2.65)			
2.00	D 12								
2.00-2.45	D 13								
2.00-2.50	B 14								
2.00	ES 14A								
3.00	D 15								
3.00-3.45	U 16	35 blows							
3.50	D 17				Orangish dark grey sandy slightly gravelly CLAY. Gravel is angular to subangular fine to coarse of mudstone and sandstone. (Weathered MUDSTONE)	3.30 +11.29			
4.00-4.45	SPT S	N=9 (1,2/2,2,3,2)	4.00	dry		(1.20)			
4.00	D 18								
4.00-4.45	D 19								
4.00-4.50	B 20								
4.50	D 21								
4.50-4.70	B 22				Dark grey thinly laminated MUDSTONE. Recovered as angular to subangular fine to coarse gravel.	4.50 +10.09			
4.70-5.00	B 23					(0.50)			
5.00-5.40	SPT S	50 (7,10/10,13,16,11 for 29mm)	04/10/2011	dry		5.00 +9.59			
5.00-5.40	D 24		4.50	dry					
	NA		05/10/2011	0800	Extremely weak thinly laminated grey MUDSTONE. Recovered as gravelly clay.	(0.75)			
	NA		4.50	dry					
	NA								
5.00-8.07	93 44 44	Flush: 5.00-8.07 mud, 95 %			Extremely weak to very weak thinly laminated grey MUDSTONE. Weathering is an orangish brown discolouration on fracture surfaces and localised loss of structure to gravelly clay. Fractures are very closely to medium spaced, planar, smooth.	5.75 +8.84			
					7.17-7.41 m 1no. 75 degree, undulating, smooth fracture				
					7.22-7.30 m NI				
					7.55-8.15 m recovered as gravelly clay				
					8.44-8.60 m 1no. 65 degree, planar, smooth fracture	(6.25)			
					8.95-9.25 m 1no. vertical, planar, smooth fracture				
8.07-10.69	100 62 42	Flush: 8.07-10.69 mud, 90 %							
Depth	ICR	If	Records/Samples	Date Casing	Time Water	Stratum continues to 12.00 m			

Groundwater Entries No. Struck Post strike behaviour None observed (see Key Sheet)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) 4.70-5.00 Time 60 mins Tools used
--	------------------	--	---

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH4 Sheet 1 of 2
--	--	---------------------------------

Scale 1:50

(c) ESG www.esg.co.uk  
426.4813/12/2011 15:24:50



# Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH Checked PH	Start 04/10/2011 End 06/10/2011	<b>Equipment, Methods and Remarks</b> Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.	Depth from 0.00m to 5.00m to 5.00m to 12.00m Diameter 150mm to 146mm Casing Depth 5.00m to 5.30m	Ground Level +14.59 mOD Coordinates E 486819.85 National Grid N 512318.46 Chainage
--	------------------------------------	--	---	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 1)			
10.69-12.00	100 36 0		Flush: 10.69-12.00 mud, 85 %	05/10/2011 5.30	dry	Extremely weak to very weak thinly laminated grey MUDSTONE. Weathering is an orangish brown discolouration on fracture surfaces and localised loss of structure to gravelly clay. Fractures are very closely to medium spaced, planar, smooth.  10.00-10.17 m 1no. 65 degree, planar, smooth fracture 10.30-10.52 m 1no. vertical, planar, rough fracture 10.42-10.52 m NI 10.64-10.80 m recovered as gravelly clay 11.04-11.06 m NI 11.27-11.38 m recovered as gravelly clay 11.38-11.57 m 1no. 75 degree, planar, smooth fracture 11.87-12.00 m 1no. 75 degree, planar, smooth fracture	12.00	+2.59	
				06/10/2011 5.30	0800 0.00				
				06/10/2011 5.30	dry				
						EXPLORATORY HOLE ENDS AT 12.00 m			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m) None observed (see Key Sheet)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
--	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH4 Sheet 2 of 2
--	--	------------------------------



# Borehole Log



Soil Mechanics

<b>Drilled</b> JB/PS <b>Logged</b> CH <b>Checked</b> PH	<b>Start</b> 05/10/2011 <b>End</b> 07/10/2011	<b>Equipment, Methods and Remarks</b> Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.	<b>Depth from</b> 0.00m <b>to</b> 4.60m <b>Diameter</b> 150mm <b>Casing Depth</b> 4.60m	<b>Ground Level</b> +12.66 mOD <b>Coordinates</b> E 486705.88 <b>National Grid</b> N 512374.44 <b>Chainage</b>
---	--	--	--	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.10	D 1	0.00-1.20 m Hand excavated inspection pit.			MACADAM (Foreman's description) (MADE GROUND)	0.10	+12.56		
0.10-0.20	B 2					0.20	+12.46		
0.30	D 3								
0.40-0.80	B 4								
0.90	D 5	N=2 (1,1/0,1,1,0)		dry	Black very sandy silty angular to subangular fine to medium GRAVEL of ash and macadam. (MADE GROUND)	0.80	+11.86		
0.90-1.10	B 6								
1.20-1.65	SPT S								
1.20	D 7								
1.20-1.65	D 8								
1.20-1.70	B 9								
2.00-2.45	SPT S	N=5 (1,1/1,1,2,1)	2.00	dry	Firm greyish brown slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of various lithologies including ash, coal and sandstone. (MADE GROUND)	(2.30)			
2.00	D 10								
2.00-2.45	D 11								
2.00-2.50	B 12								
3.00	D 13	75 blows 350 mm rec	3.00	dry	Very stiff orangish grey slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of mudstone. (Weathered MUDSTONE)	3.10	+9.56		
3.00-3.45	U 14								
3.50	D 15	50 (8,11/12,17,21,- for 68mm)	3.00	dry		(1.50)			
3.50-3.90	B 16								
4.00-4.44	SPT S								
4.00	D 17								
4.00-4.35	D 18								
4.00-4.60	B 19								
4.60-4.98	SPT S	50 (8,13/15,16,18,1 for 3mm)	05/10/2011	3.00	dry	4.60	+8.06		
4.60-4.98	D 20		07/10/2011	0800	dry				
4.60-7.64	98 8 3	Flush: 4.60-7.80 mud flush, 100 %			Extremely weak dark grey thinly laminated MUDSTONE. Fractures are very closely spaced, randomly orientated, locally stained orange, planar, smooth.	4.60-4.66 m			
						4.66-4.83 m			
						4.83-7.64 m			
						generally NI reduced to angular fine to coarse gravel due to natural fractures			
7.64-8.10	100 0 0	Flush: 7.80-8.10 mud flush, 0 %				7.64-8.10 m			
						NI reduced to a gravelly clay			
8.10-9.38	100 15 10					8.10-8.29 m			
		intact core							
						8.29-9.38 m			
						NI reduced to angular fine to coarse gravel due to natural fractures			
						9.92-9.94 m			
Depth	ICR SCR ROD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 12.00 m			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m) None observed (see Key Sheet)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used 4.00-4.60 60 mins
--	------------------	--	--

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole BH5 Sheet 1 of 2
--	--	------------------------------





# Borehole Log



Soil Mechanics

Drilled		Start		Equipment, Methods and Remarks			Depth from		Diameter		Casing Depth		Ground Level		Chainage	
JB/PS	03/10/2011	Dando 2000 and Geotech 6.			0.00m to 5.00m		150mm		5.00m		+13.02 mOD		E 486739.83		N 512358.09	
Logged	CH/JMH	Cable percussion boring followed by rotary core drilling (PWF size) using mud flush.			5.00m to 12.14m		146mm		5.00m							
Checked	PH	SPT: hammer ID JB15, rods NWY.														
Samples and Tests				Strata												
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level (Thickness)	Legend	Backfill/ Instruments								
0.10	D 1	0.00-1.20 m Hand excavated inspection pit.			MACADAM (Foreman's description) (MADE GROUND)	0.20 +12.82										
0.10-0.25	B 2					0.25 +12.77										
0.30	D 3					(0.45)										
0.40-0.70	B 4				Black very sandy angular to subrounded fine to medium GRAVEL of ash, macadam and sandstone. (MADE GROUND)	0.70 +12.32										
0.75	D 5															
0.80-1.10	B 6															
1.00	ES 6A		03/10/2011	dry	Creamish grey very sandy silty angular to subangular fine to medium GRAVEL of limestone. (MADE GROUND)											
1.20-1.58	SPT S	N=3 (1,0/1,1,0,1)														
1.20	D 7		04/10/2011	0800												
1.20-1.65	D 8			dry	Soft to firm brownish orangish slightly sandy slightly gravelly CLAY. Gravel is angular to subangular fine to medium of coal and burnt shale. (MADE GROUND)											
1.20-1.70	B 9															
2.00-2.45	SPT S	N=4 (1,1/1,1,1,1)	2.00	dry		(2.70)										
2.00	D 10															
2.00	ES 10A															
2.00-2.45	D 11															
2.00-2.50	B 12															
3.00	D 13															
3.00-3.45	U 14	80 blows														
3.50	D 15					3.40 +9.62										
3.50-3.90	B 16				Firm greyish brown slightly sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium of various lithologies. (GLACIAL TILL)	3.50 +9.52										
4.00-4.38	SPT S	50 (7,11/14,17,19)	4.00	dry		4.00 +9.02										
4.00	D 17															
4.00-4.39	D 18															
4.00-4.50	B 19				Dark grey very gravelly CLAY. Gravel is angular to subangular fine to coarse of mudstone. (Weathered MUDSTONE)											
5.00-5.36	SPT S	50 (8,14/17,17,16 for 55mm)	04/10/2011	4.50	Very weak grey thinly laminated MUDSTONE. Recovered as angular to subangular fine to medium gravel.	(1.36)										
5.00-5.36	D 20		06/10/2011	0800												
5.00-6.17	74 0 0	Flush: 5.00-6.17 mud, 100 %		dry	Extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, extremely to very closely spaced, planar, rough.	5.36 +7.66										
	NI 10 30					(2.19)										
6.17-9.14	100 53 29	Flush: 6.17-12.14 mud, 95 %			Very weak locally extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.	7.55 +5.47										
	NI 70 450					(4.59)										
Depth	ICR RCD RSD	If	Records/Samples	Date Casing	Time Water	Stratum continues to 12.14 m										
Groundwater Entries					Depth Related Remarks *			Chiselling								
No.	Struck (m)	Post strike behaviour	Depth sealed (m)	From	to (m)	4.00-5.00		Time	Tools used							
None observed (see Key Sheet)								60 mins								
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project			Borehole								
					SANSEND SLOPE STABILISATION			BH6								
					Project No. A1077-11											
					Carried out for			Sheet 1 of 2								
					Balfour Beatty Living Places											



# Borehole Log



Soil Mechanics

Drilled JB/PS Logged CH/JMH Checked PH	Start 03/10/2011 End 06/10/2011	<b>Equipment, Methods and Remarks</b> Dando 2000 and Geotech 6. Cable percussion boring followed by rotary core drilling (PWF size) using mud flush. SPT: hammer ID JB15, rods NWY.	Depth from 0.00m to 5.00m to 5.00m to 12.14m Diameter 150mm to 146mm Casing Depth 5.00m to 5.00m	Ground Level +13.02 mOD Coordinates E 486739.83 National Grid N 512358.09 Chainage
--	------------------------------------	--	---	---

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	TCR SCR RGD	If	Records/Samples	Date Casing	Time Water	Description (Continued from Sheet 1)			
9.14-12.14	100 40 24			06/10/2011 5.00	5.02	Very weak locally extremely weak thinly laminated grey MUDSTONE. Weathering is localised loss of structure to gravelly clay. Fractures are subhorizontal, very closely to medium spaced, planar, smooth.  9.76m - gravelly clay 10.11-10.19 m recovered as gravelly clay 10.47-10.67 m weakly laminated grey SILTSTONE band  11.84-11.96 m 1no. vertical, planar, smooth fracture 12.08-12.14 m NI			
EXPLORATORY HOLE ENDS AT 12.14 m							12.14	+0.88	

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m) None observed (see Key Sheet)	Depth sealed (m)	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
--	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole <b>BH6</b> Sheet 2 of 2
--	--	--



# Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 11/10/2011 End 11/10/2011	<b>Equipment, Methods and Remarks</b> Dando Terrier 2002. Windowless sampling.	Depth from 0.00m to 1.30m Diameter 80mm Casing Depth	Ground Level +4.43 mOD Coordinates E 487176.62 National Grid N 512177.53 Chainage				
Samples and Tests			Strata					
Depth	Type & No	Records	Date Casing	Time Water	Description	Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
0.30 0.30-1.00	D 1 B 2	0.00-1.20 m Hand excavated inspection pit.			Light brown medium to coarse SAND. (BEACH DEPOSITS)	(1.30)		
1.30	D 3		11/10/2011	dry	EXPLORATORY HOLE ENDS AT 1.30 m	1.30 +3.13		
					1.30 m 1 No. rounded sandstone cobble			
Depth	Type & No	Records	Date Casing	Time Water	Groundwater Entries		Depth Related Remarks *	
					No. Struck (m)	Post strike behaviour	Depth sealed (m)	From to (m)
					None observed (see Key Sheet)			1.30
								Hole terminated due to refusal.
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole <h2 style="text-align: center;">WS1</h2> Sheet 1 of 1		
Scale 1:50 (c) ESG www.esg.co.uk 426.4813/12/2011 15:25:02								

# Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH		Start 11/10/2011 End 11/10/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.		Depth from 0.00m 1.30m 2.30m 4.30m	to 1.30m 2.30m 4.30m 5.00m	Diameter 80mm 70mm 40mm 20mm	Casing Depth	Ground Level Coordinates National Grid Chainage	+3.86 mOD E 487089.81 N 512215.23		
Samples and Tests					Strata				Depth, Level/ (Thickness)	Legend	Backfill/ Instruments	
Depth	Type & No	Records	Date Casing	Time Water	Description							
0.30 0.30-1.00 0.60-2.10	D 1 B 2 B 5				Light brown slightly gravelly medium SAND. Gravel is fine to medium, occasionally coarse, of sandstone. (BEACH DEPOSITS)				(1.60)			
1.20	D 3				1.00-1.60 m increase in gravel content							
1.50	D 4								1.60	+2.26		
2.20	D 6				Stiff dark grey and brown slightly sandy slightly gravelly CLAY. Gravel is subangular to rounded fine to medium of various lithologies. (GLACIAL TILL)				(1.70)			
2.80	D 7				1.95-2.10 m band of firm dark brown sandy silt							
3.30	D 8				2.80-3.10 m band of firm dark brown sandy silt							
					NO RECOVERY				3.30	+0.56		
			11/10/2011						(1.70)			
					EXPLORATORY HOLE ENDS AT 5.00 m				5.00	-1.14		
Depth	Type & No	Records	Date Casing	Time Water	Groundwater Entries				Depth Related Remarks *			
					No.	Struck (m)	Post strike behaviour	Depth sealed (m)	From	to (m)	Hole collapsing.	
					1	2.00	-	-	3.00			
Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.					Project	SANSEND SLOPE STABILISATION				Borehole		
Scale 1:50					Project No.	A1077-11				WS2		
(c) ESG www.esg.co.uk 426.4813/12/2011 15:25:04					Carried out for	Balfour Beatty Living Places				Sheet 1 of 1		

# Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 10/11/2011 End 10/11/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.	Depth from 0.00m 1.30m 2.00m	to 1.30m 2.00m 2.50m	Diameter 80mm 60mm -	Casing Depth	Ground Level +3.98 mOD Coordinates E 486809.97 National Grid N 512352.00 Chainage
---------------------------------------	------------------------------------	---	------------------------------------	----------------------------	----------------------------	--------------	--

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.30 0.30-1.00	D 1 B 2				Light brown gravelly medium to coarse SAND with occasional sand size shell fragments. (BEACH DEPOSITS)	(1.80)			
1.30-1.50	D 3								
1.80-2.00	D 4				Multicoloured sandy subrounded to rounded fine to coarse GRAVEL of sandstone. (BEACH DEPOSITS)	1.80 +2.18 (0.40)			
			10/11/2011			2.20 +1.78 (0.30)			
2.50	D 5				Dark grey and black slightly sandy gravelly CLAY. Gravel is angular fine to medium of mudstone. (Weathered MUDSTONE)  EXPLORATORY HOLE ENDS AT 2.50 m	2.50 +1.48			

<b>Groundwater Entries</b> No. Struck Post strike behaviour (m) None observed (see Key Sheet)	Depth sealed (m)	Depth Related Remarks * From to (m) 2.50 Hole terminated due to refusal.	Chiselling Depths (m) Time Tools used
--	------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole <b>WS3</b> Sheet 1 of 1
--	--	--



# Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 10/11/2011 End 10/11/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.	Depth from 0.00m to 1.00m 1.00m to 1.90m	Diameter 80mm Casing Depth -	Ground Level +2.69 mOD Coordinates E 486649.56 National Grid N 512432.75 Chainage
---------------------------------------	------------------------------------	---	---	---------------------------------	--

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.40 0.40-0.90	D 1 B 2	0.00-1.20 m Hand excavated inspection pit.			Light brown gravelly fine to coarse SAND. Gravel is subrounded to rounded fine to medium of various lithologies including brick. (BEACH DEPOSITS)	(1.00)			
0.90 1.20-1.33	D 3 SPT S	50 (15/50 for 50mm)			Extremely weak dark grey, becoming light brown and orange, thinly laminated MUDSTONE.	1.00 +1.69 (0.90)			
1.50	D 4		10/11/2011			1.90 +0.79			
EXPLORATORY HOLE ENDS AT 1.90 m									

<b>Groundwater Entries</b> No. Struck Post strike behaviour 1 1.00 -	Depth sealed (m) -	Depth Related Remarks * From to (m)	Chiselling Depths (m) Time Tools used
--	-----------------------	--	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole <b>WS4</b> Sheet 1 of 1
--	--	--



# Dynamic Sampler Hole Log



Soil Mechanics

Drilled CS Logged CH Checked PH	Start 10/11/2011 End 10/11/2011	Equipment, Methods and Remarks Dando Terrier 2002. Windowless sampling.	Depth from 0.00m 1.30m 2.20m	to 1.30m 2.20m 4.00m	Diameter 80mm 60mm 40mm	Casing Depth	Ground Level +3.72 mOD Coordinates E 486380.62 National Grid N 512549.27 Chainage
---------------------------------------	------------------------------------	---	------------------------------------	----------------------------	-------------------------------	--------------	--

Samples and Tests				Strata			Depth, Level/ (Thickness)	Legend	Backfill/ Instruments
Depth	Type & No	Records	Date Casing	Time Water	Description				
0.40	D 1				Brown slightly sandy angular to rounded fine GRAVEL of various lithologies. (BEACH DEPOSITS)	(0.60)			
0.60-1.30	B 3					0.60 +3.12			
0.80	D 2					(0.95)			
1.70	D 4				Stiff, locally firm, orangish brown slightly sandy slightly gravelly CLAY. (GLACIAL TILL)	1.55 +2.17			
2.20	D 5								
3.00-3.40	D 6				1.80-1.90 m band of firm sandy silt	(2.65)			
			10/11/2011						
					EXPLORATORY HOLE ENDS AT 4.20 m	4.20 -0.48			
					4.20 m fragments of medium mudstone gravel				

<b>Groundwater Entries</b> No. Struck Post strike behaviour 1 1.30 -	Depth sealed (m) -	Depth Related Remarks * From to (m) 2.80 Hole collapsing.	Chiselling Depths (m) Time Tools used
--	-----------------------	---	---------------------------------------

Notes: For explanation of symbols and abbreviations see key sheet. All depths and reduced levels in metres. Stratum thickness given in brackets in depth column.	Project SANDSEND SLOPE STABILISATION Project No. A1077-11 Carried out for Balfour Beatty Living Places	Borehole WS5 Sheet 1 of 1
--	--	------------------------------



**ENCLOSURE B  
INSTRUMENTATION**

Installation Details

B1

# Groundwater Installation Details

Hole No	Instrument ID	Installation Type	Date of Installation	Reference depth (mBGL)	Piezometer Diameter (mm)	Top of response zone (mBGL)	Base of response zone (mBGL)	Tubing Completion Details	Headworks	Remarks
BH2		SPIE	6 Oct 2011	21.00	19	20.50	21.50	None	Stop cock cover	
BH3	A	SPIE	11 Oct 2011	4.70	19	4.00	5.00	None	Stop cock cover	
BH3	B	SPIE	11 Oct 2011	15.70	19	15.00	16.00	None	Stop cock cover	
BH4		SPIE	6 Oct 2011	10.70	19	10.00	11.00	None	Stop cock cover	
BH5	A	SPIE	7 Oct 2011	4.20	19	3.50	4.50	None	Stop cock cover	
BH5	B	SPIE	7 Oct 2011	10.70	19	10.00	11.00	None	Stop cock cover	

Notes: Type: SP - Standpipe, SPIE - Standpipe Piezometer, HPIE - Hydraulic Piezometer, PPIE - Pneumatic Piezometer, EPIE - Electronic Piezometer Prepared: 01/12/2011 17:30



Project SANDSEND SLOPE STABILISATION  
 Project No. A1077-11  
 Carried out for Balfour Beatty Living Places

Table

**B1**

**ENCLOSURE C**  
**GEOTECHNICAL LABORATORY TEST RESULTS**

Index Properties – Summary of Results	INDX 1 and 2
Particle Size Distribution Analyses	PSD 1 to 50
Unconsolidated Undrained Triaxial Compression Tests – Summary of Results	UUSUM 1
Consolidated Undrained Triaxial Compression	CUM 1 to 10
Shear Strength by Direct Shear (Shearbox)	SSB 1 to 4
Residual Shear Strength (Ring Shear)	RS 1 and 2
Shear Strength by Laboratory Vane	LVANE1
One Dimensional Consolidation Test	OED 1 to 7
Dry Density / Moisture Content Relationship	COMPH 1 to 13
Moisture Condition Value (MCV) / Moisture Content Relationship	MCVREL 1 to 13
Chemical Tests – Summary of Results	CHEM 1 and 2

# INDEX PROPERTIES - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANSEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample			Soil Description	$\rho$	$\rho_d$	$W$	< 425 $\mu$ m sieve	$W_L$	$W_P$	$I_p$	$\rho_s$	Remarks
	No.	Depth (m)			type	Mg/m <sup>3</sup>		%	%	%	%	Mg/m <sup>3</sup>	
		from	to										
BH1	5	1.20	1.65	U	Firm brown slightly sandy slightly gravelly CLAY.	2.11	1.78	19	76 s	40 a	18	22	
BH1	9	3.00	3.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.18	1.89	15	80 s	31 a	15	16	
BH1	13	5.00	5.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.26	1.98	14	81 s	31 a	14	17	
BH1	17	6.50	6.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.14	1.86	15	74 s	33 a	15	18	
BH1	21	8.00	8.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.			15	84 s	35 a	16	19	
BH1	25	9.50	9.95	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.	2.24	1.95	15	83 s	32 a	15	17	
BH1	29	11.00	11.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.			15	84 s	34 a	15	19	
BH1	33	12.50	12.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.19	1.91	15	79 s	32 a	15	17	
BH1	37	14.00	14.45	U	Firm greyish brown slightly sandy slightly gravelly CLAY.			23	97 s	37 a	17	20	
BH1	41	15.50	15.95	U	Stiff brown thinly laminated CLAY with thin laminations of sand becoming slightly sandy	2.11	1.77	19	91 s	36 a	17	19	
BH1	45	17.00	17.45	U	Stiff brown slightly sandy slightly gravelly CLAY.			17	94 s	37 a	17	20	
BH1	52	19.50	19.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.24	2.00	12	87 s	31 a	15	16	
BH1	56	21.00	21.45	U	Stiff to very stiff brown slightly sandy slightly gravelly CLAY.			13	88 s	31 a	15	16	
BH1	60	22.50	23.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.19	1.92	14	88 s	32 a	15	17	
BH2	4	1.20	1.65	U	Firm to stiff greyish brown and brownish grey slightly sandy slightly gravelly CLAY.	2.07	1.71	21	91 s	53 a	21	32	
BH2	8	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.16	1.89	14	82 s	34 a	16	18	
BH2	12	5.00	5.45	U	Brown slightly sandy slightly gravelly CLAY.			14	87 s	31 a	15	16	
BH2	16	7.00	7.45	U	Stiff brown slightly sandy slightly gravelly CLAY.			15	88 s	30 a	15	15	
BH2	20	9.00	9.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.			13	80 s	28 a	14	14	
BH2	24	11.00	11.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.	2.22	1.93	15	86 s	33 a	15	18	
BH2	31	15.00	15.45	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.			19	99 n	49 a	23	26	
BH2	37	17.00	17.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.14	1.83	17	86 s	35 a	16	19	
BH2	43	19.00	19.45	U	Firm brown slightly sandy slightly gravelly CLAY.			15	79 s	31 a	16	15	
BH2	47	21.00	21.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			14	81 s	31 a	15	16	
BH2	51	23.00	23.45	U	Firm brown slightly sandy slightly gravelly CLAY.	2.20	1.93	14	85 s	32 a	15	17	
BH2	55	25.00	25.45	U	Firm to stiff greyish brown and grey slightly sandy slightly gravelly CLAY.			16	88 s	35 a	16	19	
BH3	4	1.20	1.65	U	Firm to stiff greyish brown and grey slightly sandy slightly gravelly CLAY.	2.04	1.68	21	99 n	53 a	22	31	
BH3	8	2.50	2.95	U	Firm to very stiff brown slightly sandy slightly gravelly CLAY.	2.15	1.86	16	87 s	37 a	18	19	
BH3	16	5.50	5.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.17	1.85	17	70 s	32 a	15	17	
BH3	24	8.50	8.95	U	Firm greyish brown slightly sandy slightly gravelly CLAY.			17	86 s	34 a	15	19	
BH3	28	10.00	10.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.			14	69 s	33 a	16	17	
BH3	33	11.50	11.95	D	Grey slightly sandy CLAY.				100 n	31 a	17	14	

General notes: All above tests carried out to BS1377 : 1990 definitive method in all cases unless annotated otherwise. See individual test reports for further details.

Key :  $\rho$  bulk density, linear       $W_L$  Liquid limit       $W_P$  Plastic limit      <425um preparation       $\rho_s$  particle density  
 $\rho_d$  dry density      a 4 point cone test      NP non - plastic      n from natural soil      -g = gas jar  
w moisture content      b 1 point cone test       $I_p$  Plasticity Index      s sieved specimen      -p = small pycnometer

# INDEX PROPERTIES - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANSEND BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample			Soil Description	$\rho$	$\rho_d$	$W$	< 425 $\mu$ m sieve	$W_L$	$W_P$	$I_P$	$\rho_s$	Remarks	
	No.	Depth (m)			type				%	%	%	%		
		from	to			Mg/m <sup>3</sup>	%	%	%	Mg/m <sup>3</sup>				
BH3	37	13.50	13.95	D	Brownish grey slightly sandy slightly gravelly CLAY.				91 s	35 a	16	19		
BH3	39	14.50	14.95	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.21	1.94	14	87 s	29 a	14	15		
BH3	47	17.50	17.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			12	85 s	32 a	13	19		
BH4	13	2.00	2.45	D	Brown slightly sandy slightly gravelly CLAY.			19	71 s	36 b	17	19		
BH4	16	3.00	3.45	U	Firm greyish brown slightly sandy slightly gravelly CLAY.	1.96	1.59	23	79 s	46 a	21	25		
BH4	19	4.00	4.45	D	Greyish brown slightly sandy slightly gravelly CLAY.			17	72 s	37 b	18	19		
BH4	21	4.50		D	Dark grey slightly gravelly CLAY. Gravel is mainly weak mudstone.			15	98 n	47 a	22	25		
BH4	24	5.00	5.40	D	Grey slightly sandy slightly gravelly silty CLAY.			14	98 s	44 a	25	19		
BH5	7	1.20		D	Dark brownish grey slightly gravelly sandy CLAY.				42 s	28 a	18	10		
BH5	8	1.20	1.65	D	Grey very gravelly slightly clayey SAND.			15						
BH5	11	2.00	2.45	D	Grey slightly sandy slightly gravelly CLAY.			17	74 s	35 a	19	16		
BH5	14	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	1.87	1.53	22	82 s	44 a	22	22		
BH5	18	4.00	4.35	D	Brownish grey slightly gravelly CLAY.			10	78 s	36 a	22	14		
BH6	8	1.20	1.65	D	Greyish brown slightly gravelly sandy CLAY.			17	60 s	26 b	16	10		
BH6	10	2.00		D	Greyish brown slightly sandy gravelly CLAY.				38 s	28 a	18	10		
BH6	11	2.00	2.45	D	Brown slightly sandy slightly gravelly CLAY.			21						
BH6	14	3.00	3.45	U	Firm brownish grey slightly sandy slightly gravelly CLAY.	2.08	1.72	21	90 s	49 a	26	23		
BH6	18	4.00	4.39	D	Grey slightly gravelly silty CLAY.			10	80 s	37 a	22	15		
WS1	3	1.30		D	Brown very sandy COBBLE.			2.7	29 s	25 b	NP			
WS2	2	0.30	1.00	B	Brownish grey gravelly SAND.			4.4	64 n	23 b	NP			
WS2	7	2.80		D	Grey sandy clayey SILT.			18	100 n	26 a	17	9		
WS3	2	0.30	1.00	B	Brown gravelly SAND.			11	62 s	24 b	NP			
WS3	4	1.80	2.00	D	Brownish grey sandy GRAVEL.			3.8						
WS3	5	2.50	2.50	D	Brownish grey slightly sandy gravelly CLAY.			13	30 s	39 b	22	17		
WS4	2	0.40	0.90	B	Grey and brown very gravelly SAND.			13	32 s	22 b	NP			
WS4	4	1.50		D	Dark grey slightly sandy silty CLAY.			16	100 n	39 a	25	14		
WS5	2	0.80		D	Brown gravelly SAND.			14	27 s	24 b	NP			
WS5	6	3.00	3.40	D	Brown slightly sandy slightly gravelly CLAY.			18	95 s	34 a	16	18		

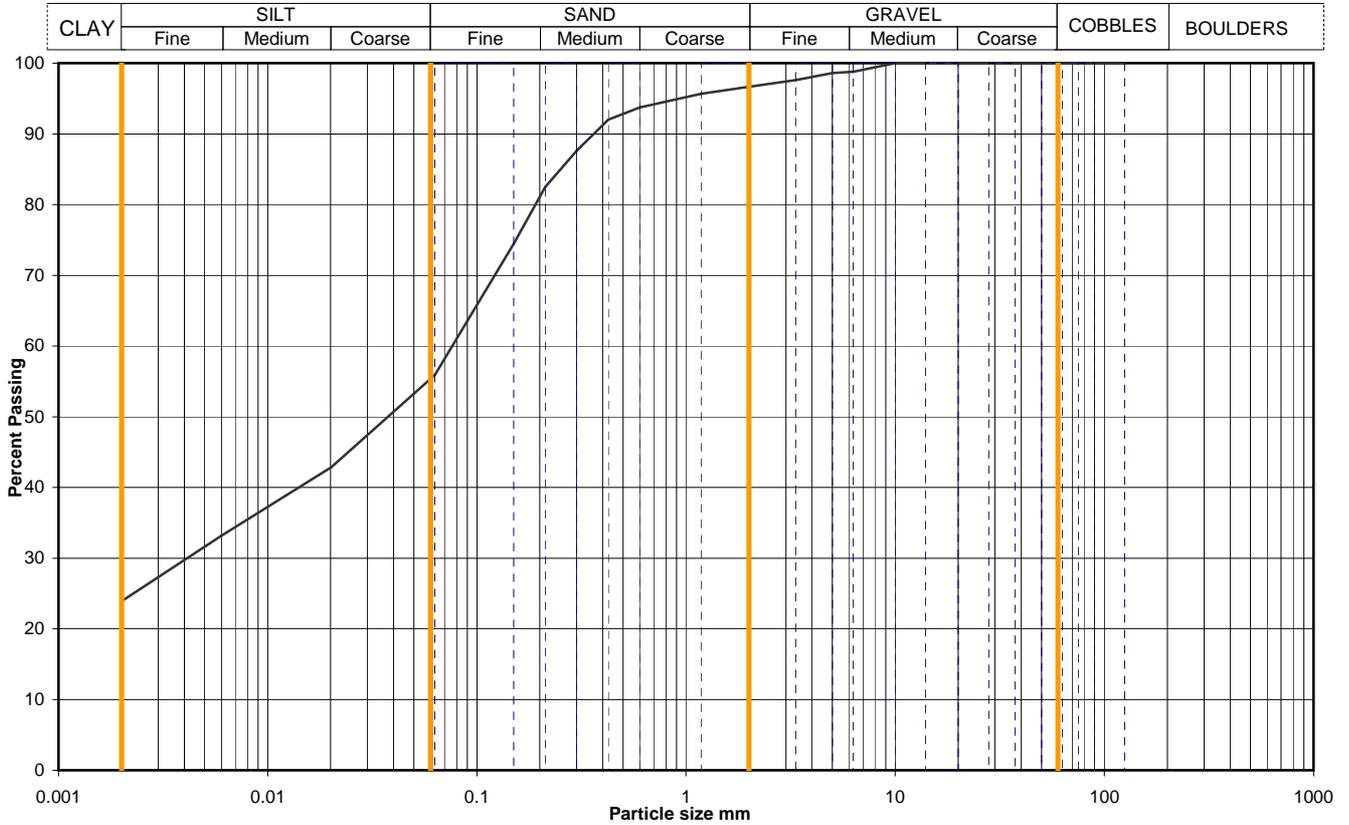
General notes:	All above tests carried out to BS1377 : 1990 definitive method in all cases unless annotated otherwise. See individual test reports for further details.
----------------	--

Key :	$\rho$ bulk density, linear	$W_L$ Liquid limit	$W_P$ Plastic limit	<425um preparation	$\rho_s$ particle density
	$\rho_d$ dry density	a 4 point cone test	NP non - plastic	n from natural soil	-g = gas jar
	w moisture content	b 1 point cone test	$I_P$ Plasticity Index	s sieved specimen	-p = small pycnometer

<b>QA Ref</b> SLR 1 Rev 88 Aug 11		Printed:01/12/2011 09:37	<b>Table</b>  <b>INDX 2</b>
--	---	--------------------------	-----------------------------------

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	4	Type	B
			ID	ESGA1077-11201110100000000004		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	43
90	100	0.0060	33
75	100	0.0020	24
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	99		
5.0	99		
3.35	98		
2.00	97		
1.18	96		
0.600	94		
0.425	92		
0.300	88		
0.212	82		
0.150	74		
0.063	56		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 13.5
---	--------------------------------

Soil description	Dark brown slightly gravelly sandy CLAY with occasional rootlets.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	* <60mm
		0	0
		3	3
		41	41
		32	32
24	24		

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

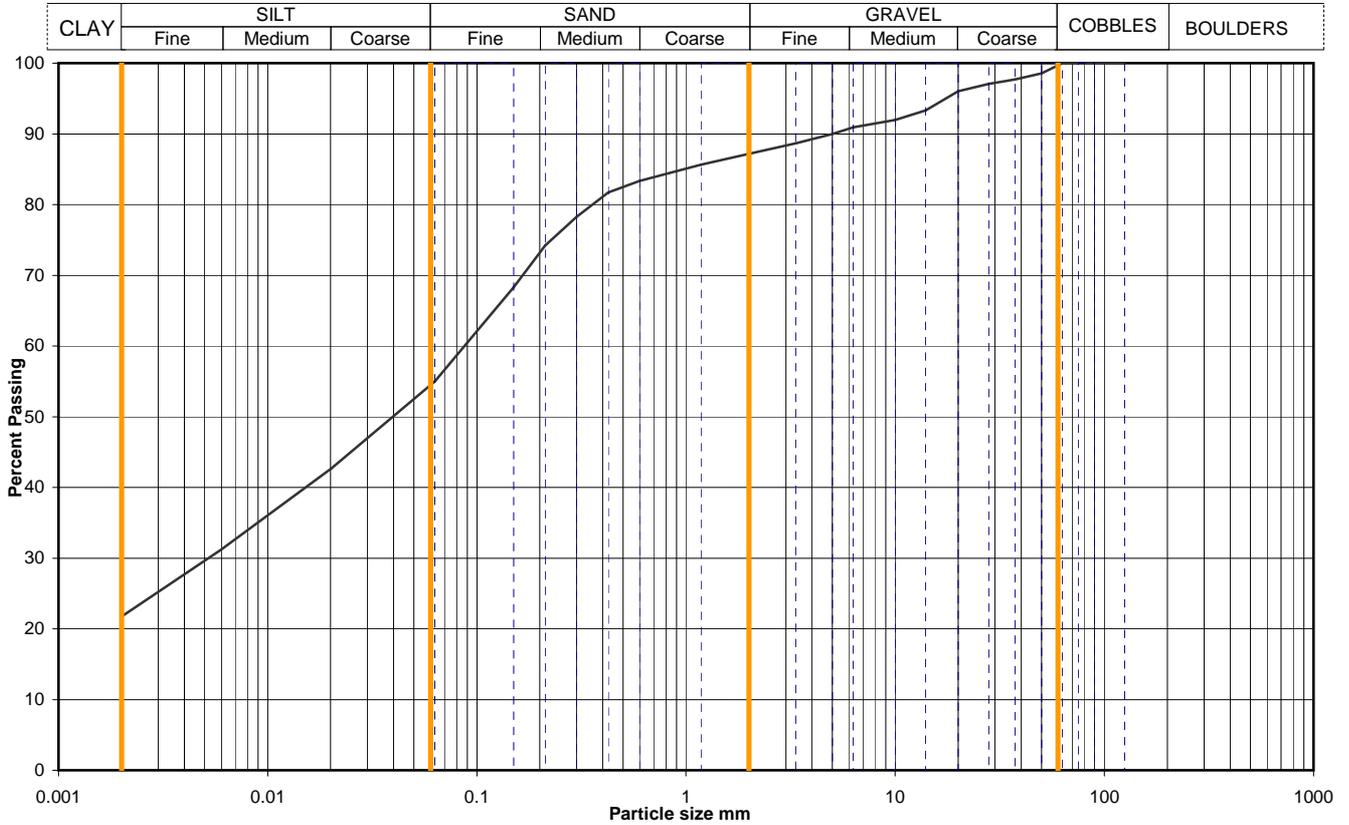


Printed: 18/11/2011 17:29

Figure  
**PSD 1**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANSENDBOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	8	Type	B
			ID	ESGA1077-11201110100000000009		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	43
90	100	0.0060	31
75	100	0.0020	22
63	100		
50	99		
37.5	98		
28	97		
20	96		
14	93		
10	92		
6.3	91		
5.0	90		
3.35	89		
2.00	87		
1.18	86		
0.600	83		
0.425	82		
0.300	78		
0.212	74		
0.150	68		
0.063	55		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 12.5
---	--------------------------------

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		13	13
		33	33
		22	22

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

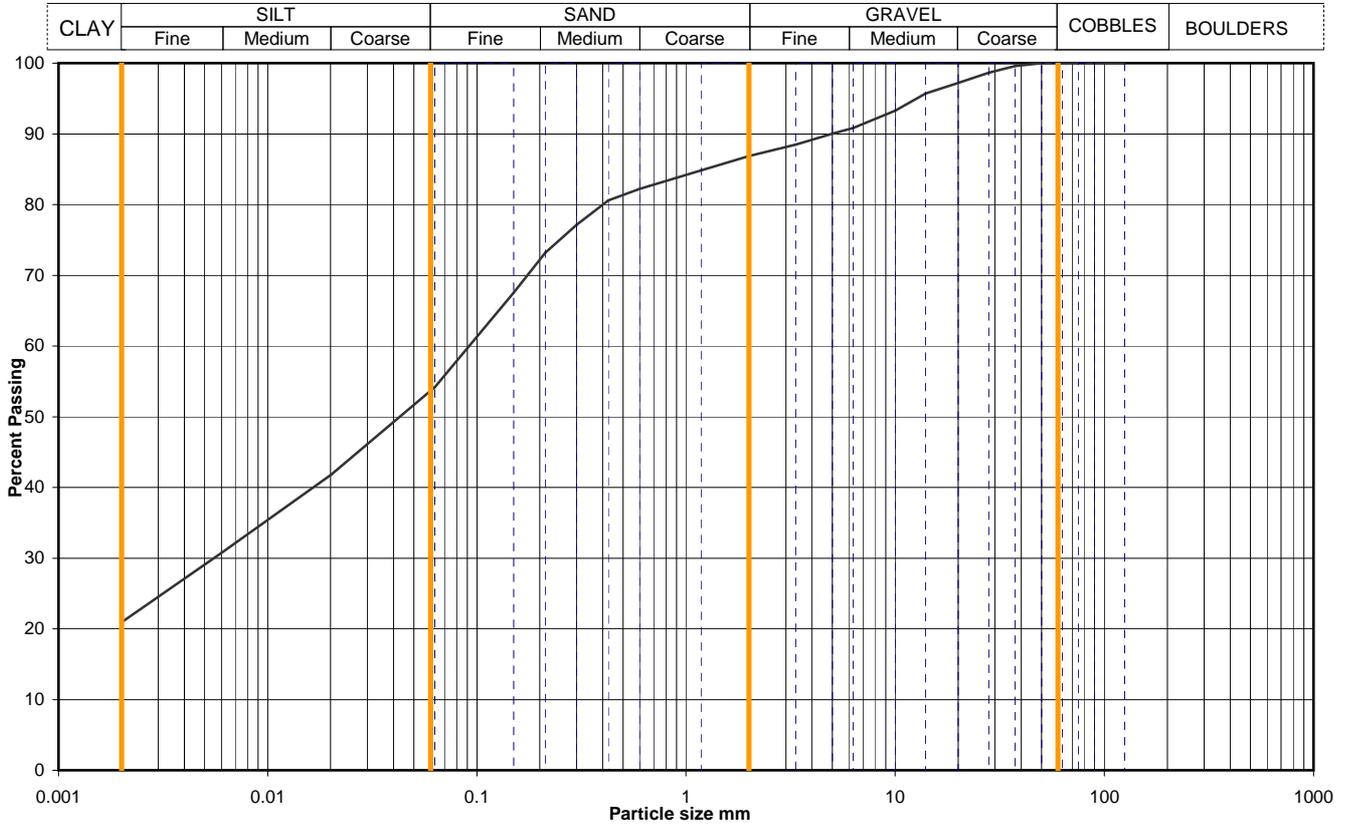


Printed:18/11/2011 17:30

Figure  
**PSD 2**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	12	Type	B
			ID	ESGA1077-11201110100000000013		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	42
90	100	0.0060	31
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	99		
20	97		
14	96		
10	93		
6.3	91		
5.0	90		
3.35	88		
2.00	87		
1.18	85		
0.600	82	Particle density, Mg/m <sup>3</sup>	
0.425	81	2.65 assumed	
0.300	77	Dry mass of sample, kg	
0.212	73	16.9	
0.150	68		
0.063	54		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		13	13
		33	33
		21	21
*<60mm values to aid description only			

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

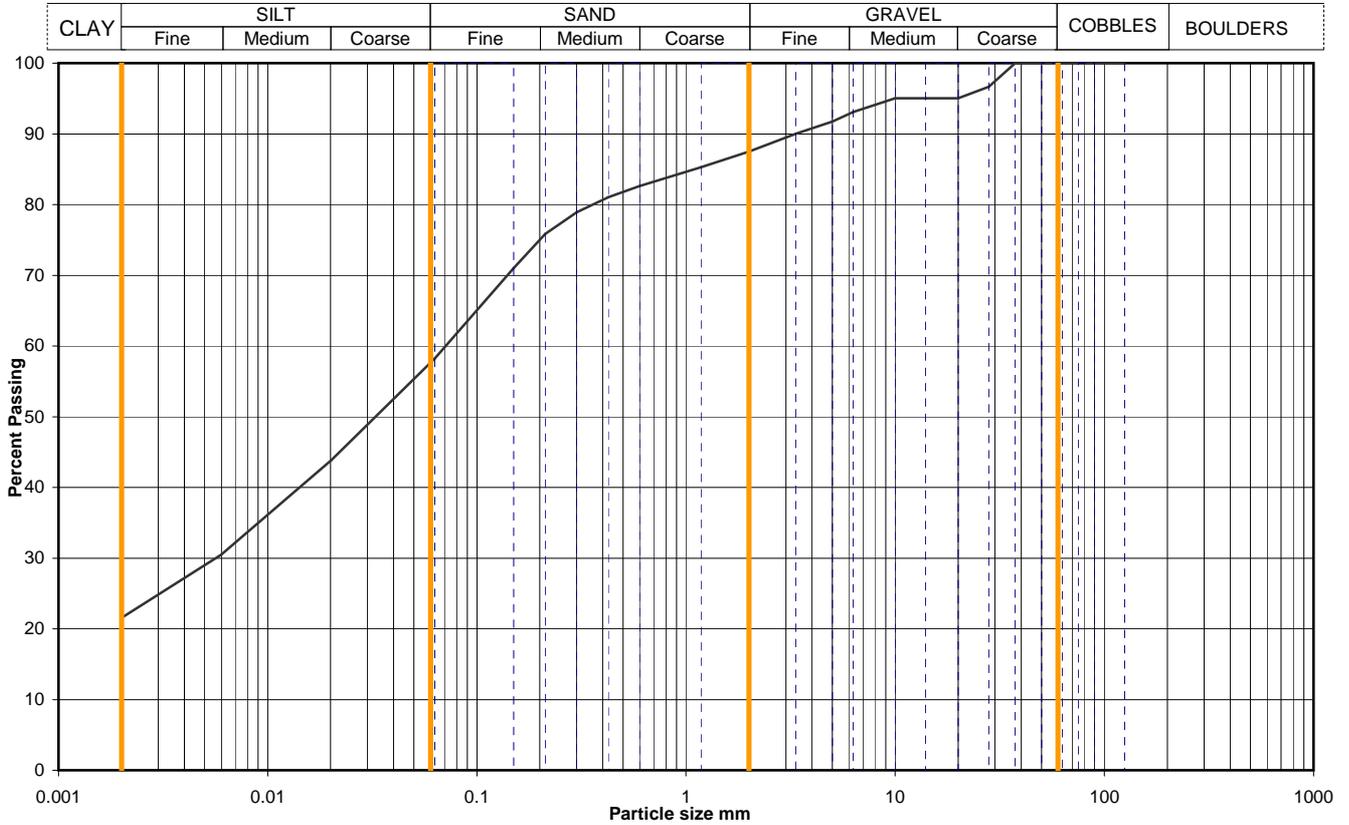


Printed: 18/11/2011 17:30

Figure  
**PSD 3**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANSENDBOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	9.95		
			Samp No	26	Type	D
			ID	ESGA1077-11201110100000000027		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	30
75	100	0.0020	22
63	100		
50	100		
37.5	100		
28	97		
20	95		
14	95		
10	95		
6.3	93		
5.0	92		
3.35	90		
2.00	87		
1.18	85		
0.600	83	Particle density, Mg/m <sup>3</sup>	
0.425	81	2.65 assumed	
0.300	79	Dry mass of sample, kg	
0.212	76	1.7	
0.150	71		
0.063	58		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		13	13
		30	30
		36	36
*<60mm values to aid description only		21	21

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

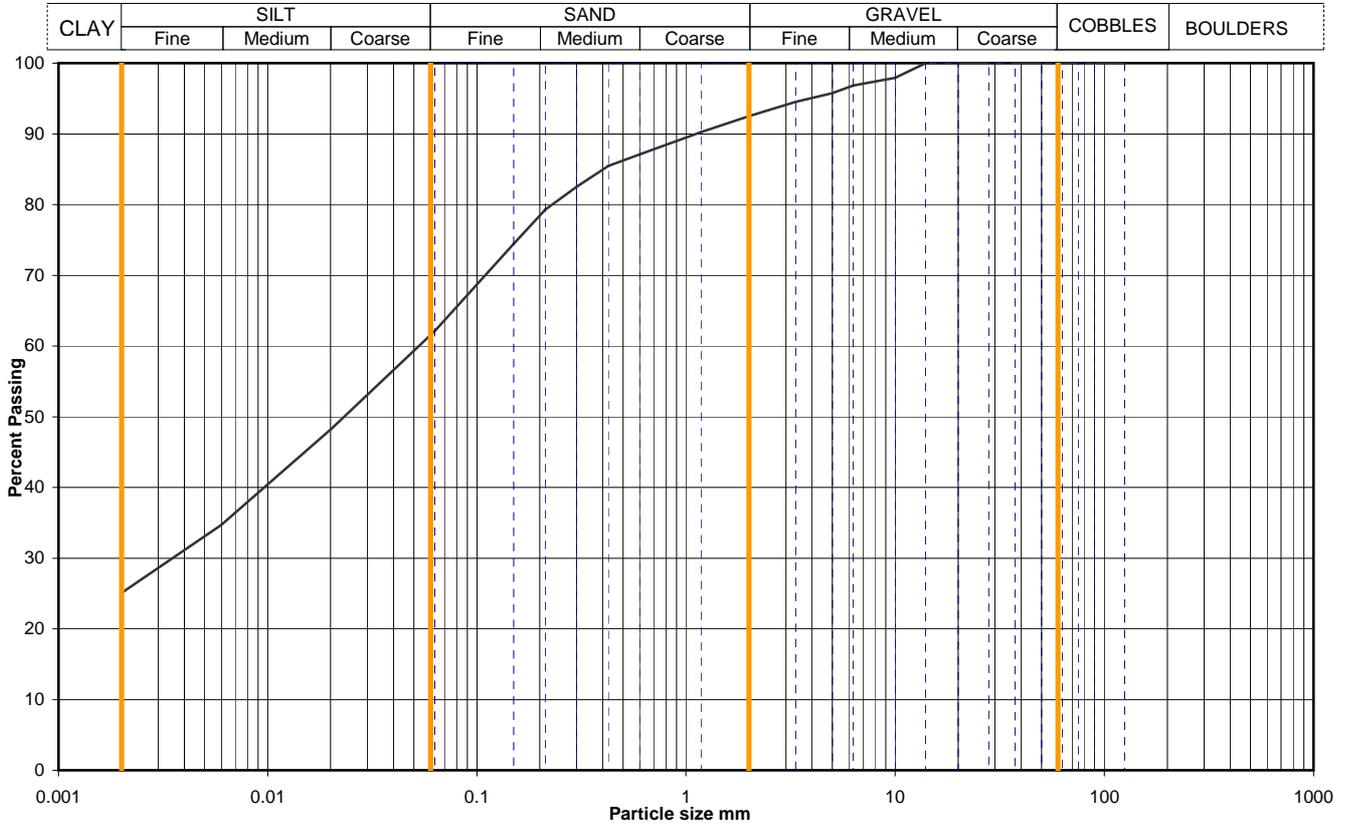


Printed:18/11/2011 17:30

Figure  
**PSD 4**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	13.15	
		Samp No	35	Type	D
		ID	ESGA1077-11201110100000000036		
		Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	48
90	100	0.0060	35
75	100	0.0020	25
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	98		
6.3	97		
5.0	96		
3.35	95		
2.00	93		
1.18	90		
0.600	87		
0.425	85		
0.300	83		
0.212	79		
0.150	74		
0.063	62		
		Particle density, Mg/m <sup>3</sup>	
		2.65 assumed	
		Dry mass of sample, kg	
		0.4	

Soil description	Greyish brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		7	7
		31	31
		37	37
*<60mm values to aid description only		25	25

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

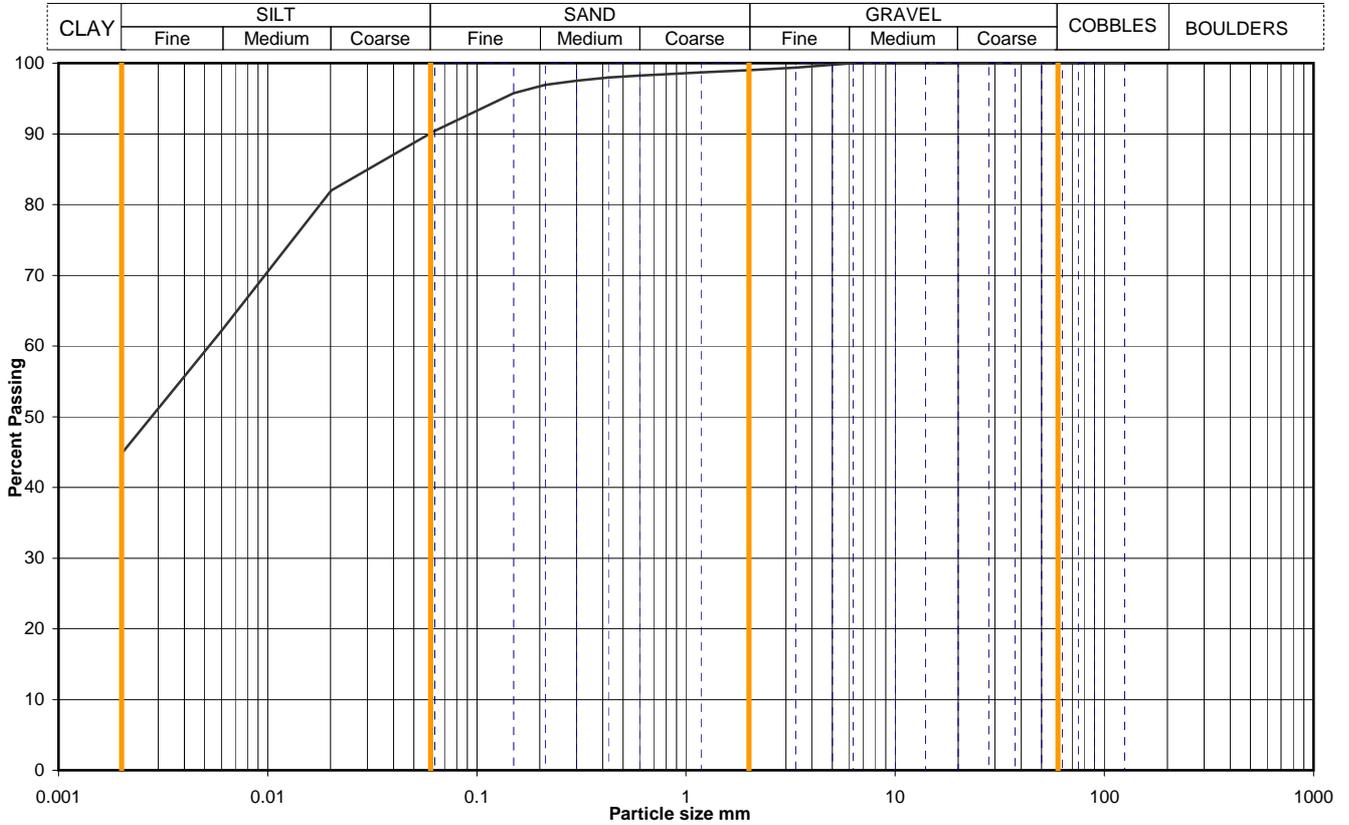


Printed: 18/11/2011 17:30

Figure  
**PSD 5**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	14.45
			Samp No	38
			Type	D
			ID	ESGA1077-11201110100000000039
		Spec Ref		



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	82
90	100	0.0060	62
75	100	0.0020	45
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	99		
2.00	99		
1.18	99		
0.600	98	Particle density, Mg/m <sup>3</sup>	
0.425	98	2.65 assumed	
0.300	98	Dry mass of sample, kg	
0.212	97	1.5	
0.150	96		
0.063	90		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		1	1
		9	9
		45	45
<small>*&lt;60mm values to aid description only</small>			

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

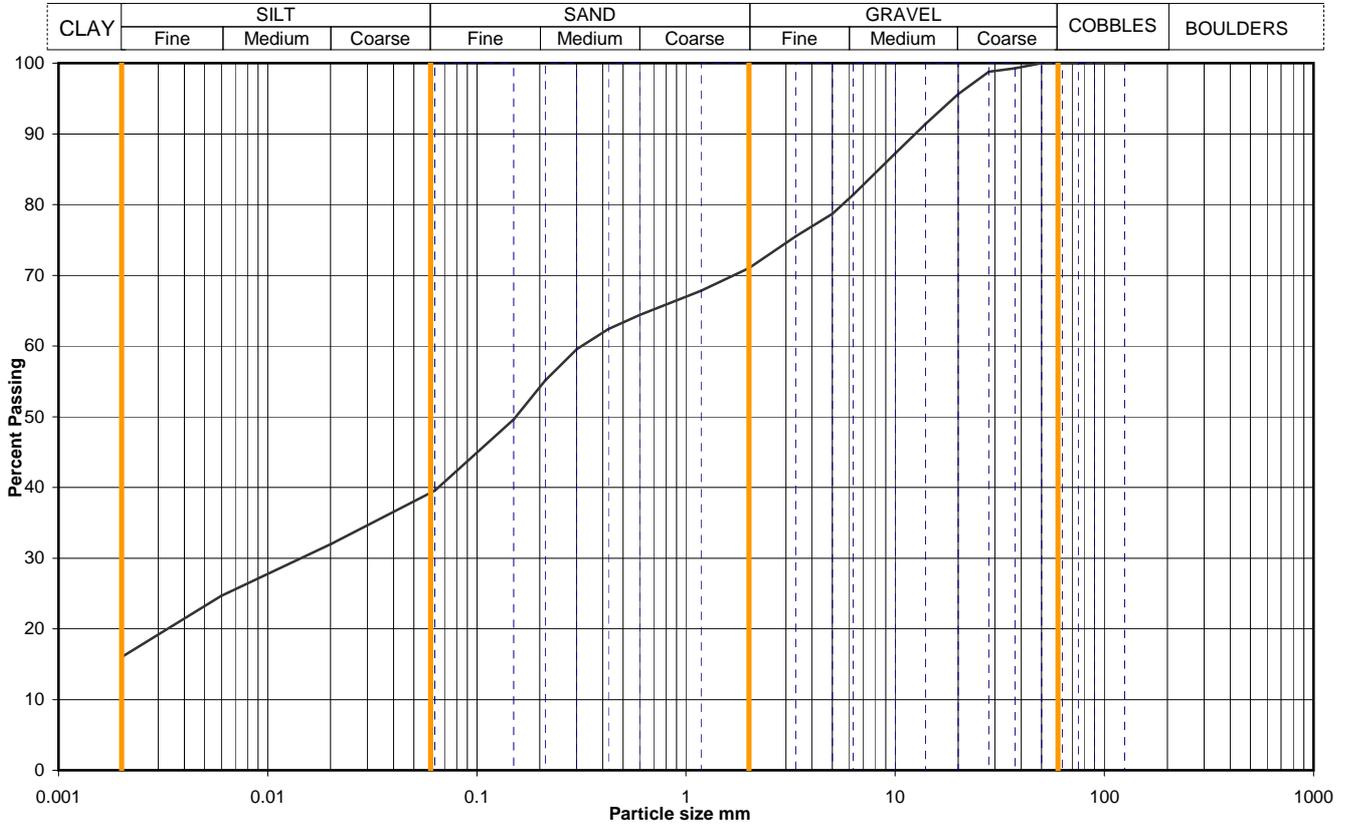


Printed: 18/11/2011 17:30

Figure  
**PSD 6**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	16.15		
			Samp No	44	Type	B
			ID	ESGA1077-11201110100000000045		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	32
90	100	0.0060	25
75	100	0.0020	16
63	100		
50	100		
37.5	99		
28	99		
20	96		
14	91		
10	87		
6.3	81		
5.0	79		
3.35	76		
2.00	71		
1.18	68		
0.600	64	Particle density, Mg/m <sup>3</sup>	
0.425	62	2.65 assumed	
0.300	60	Dry mass of sample, kg	
0.212	55	12.8	
0.150	50		
0.063	40		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		29	29
		32	32
		23	23
*<60mm values to aid description only		16	16

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

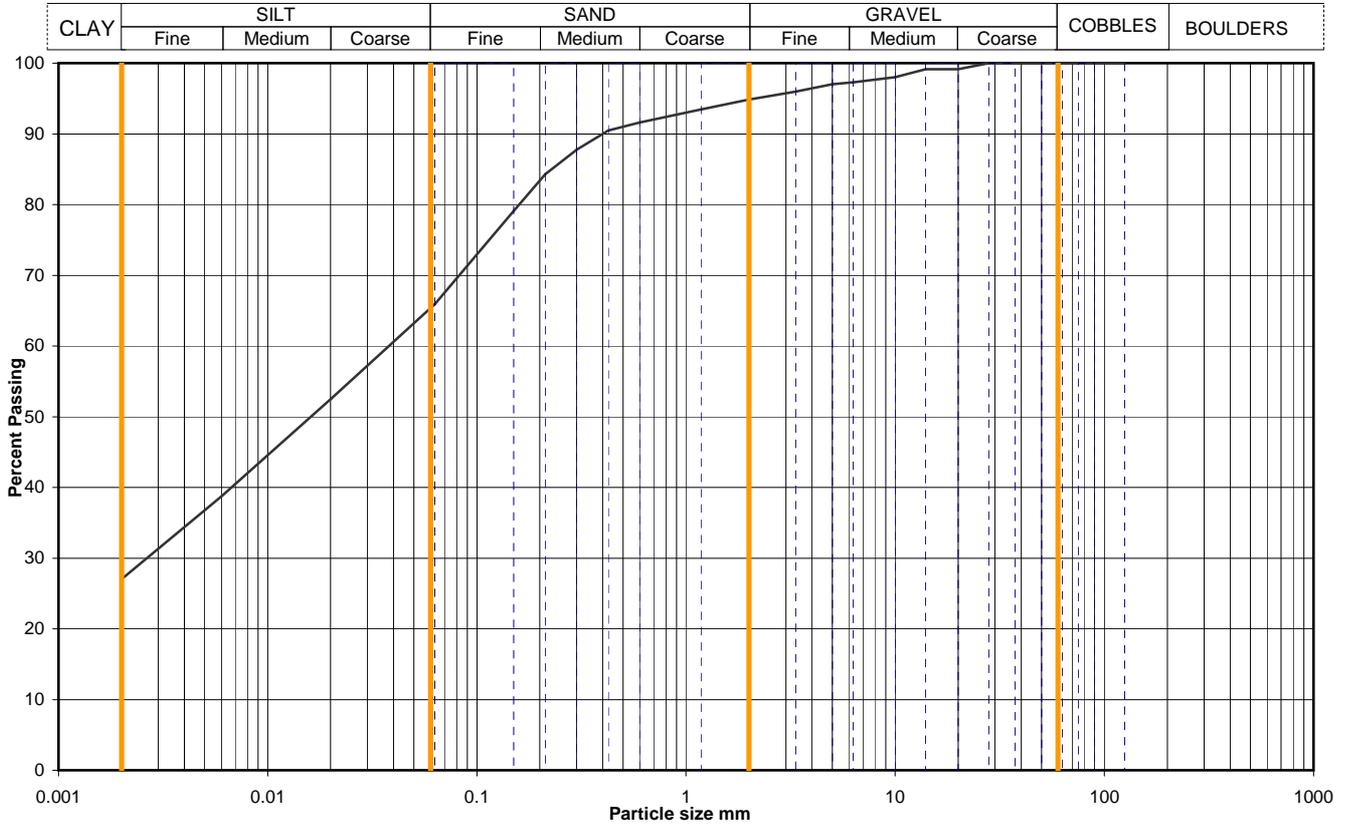


Printed:18/11/2011 17:30

Figure  
**PSD 7**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	20.15		
			Samp No	55	Type	B
			ID	ESGA1077-11201110100000000056		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	53
90	100	0.0060	39
75	100	0.0020	27
63	100		
50	100		
37.5	100		
28	100		
20	99		
14	99		
10	98		
6.3	97		
5.0	97		
3.35	96		
2.00	95		
1.18	93		
0.600	92		
0.425	90		
0.300	88		
0.212	84		
0.150	79		
0.063	66		

Particle density, Mg/m <sup>3</sup>	
2.65	assumed
Dry mass of sample, kg	
6.1	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<math><60\text{mm}</math>
		0	0
		5	5
		29	29
		39	39
*<math><60\text{mm}</math> values to aid description only		27	27

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

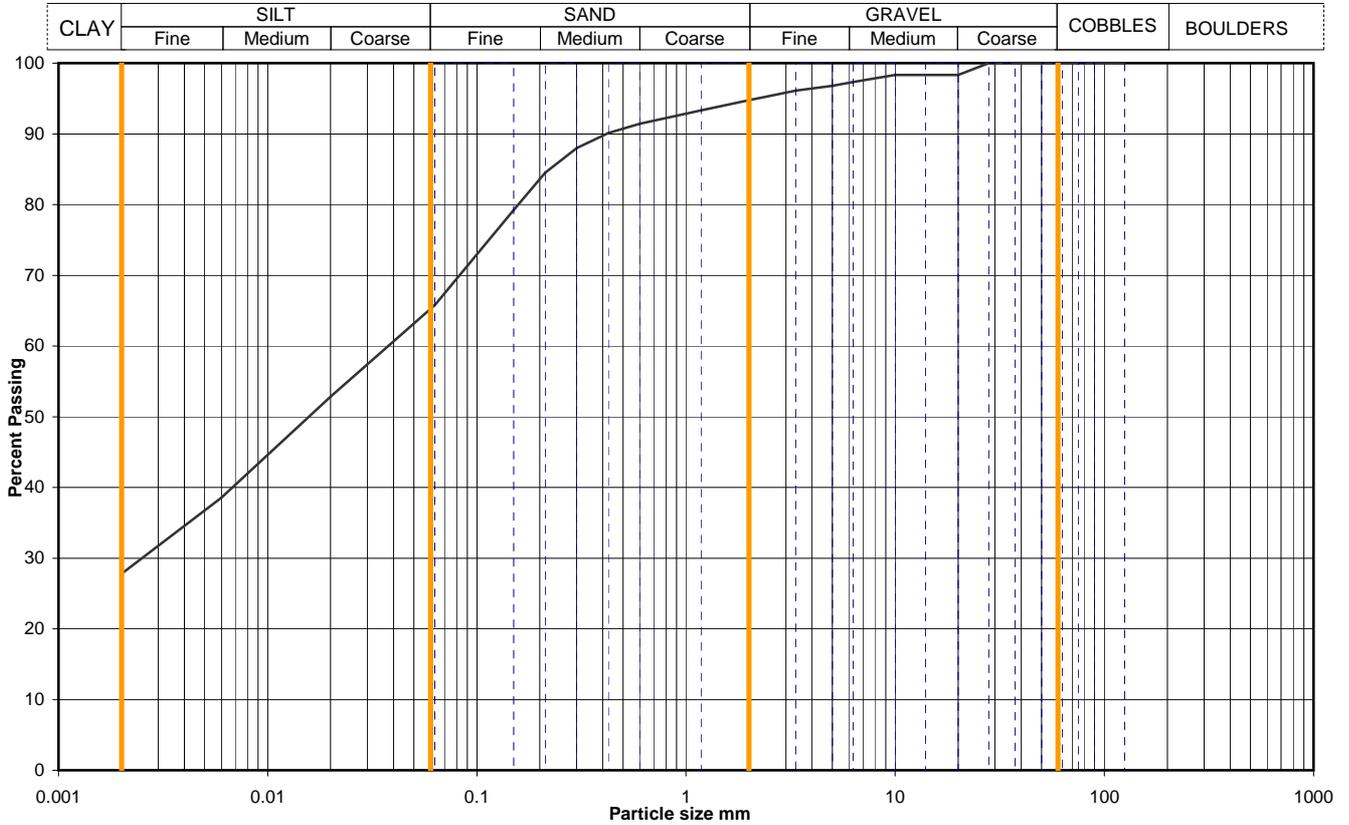


Printed: 18/11/2011 17:30

Figure  
**PSD 8**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	23.15
			Samp No	63
			Type	B
			ID	ESGA1077-11201110100000000064
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	53
90	100	0.0060	39
75	100	0.0020	28
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	98		
10	98		
6.3	97		
5.0	97		
3.35	96		
2.00	95		
1.18	93		
0.600	91		
0.425	90		
0.300	88		
0.212	84		
0.150	79		
0.063	66		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	4.5

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		5	5
		30	30
		37	37
*<60mm values to aid description only		28	28

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

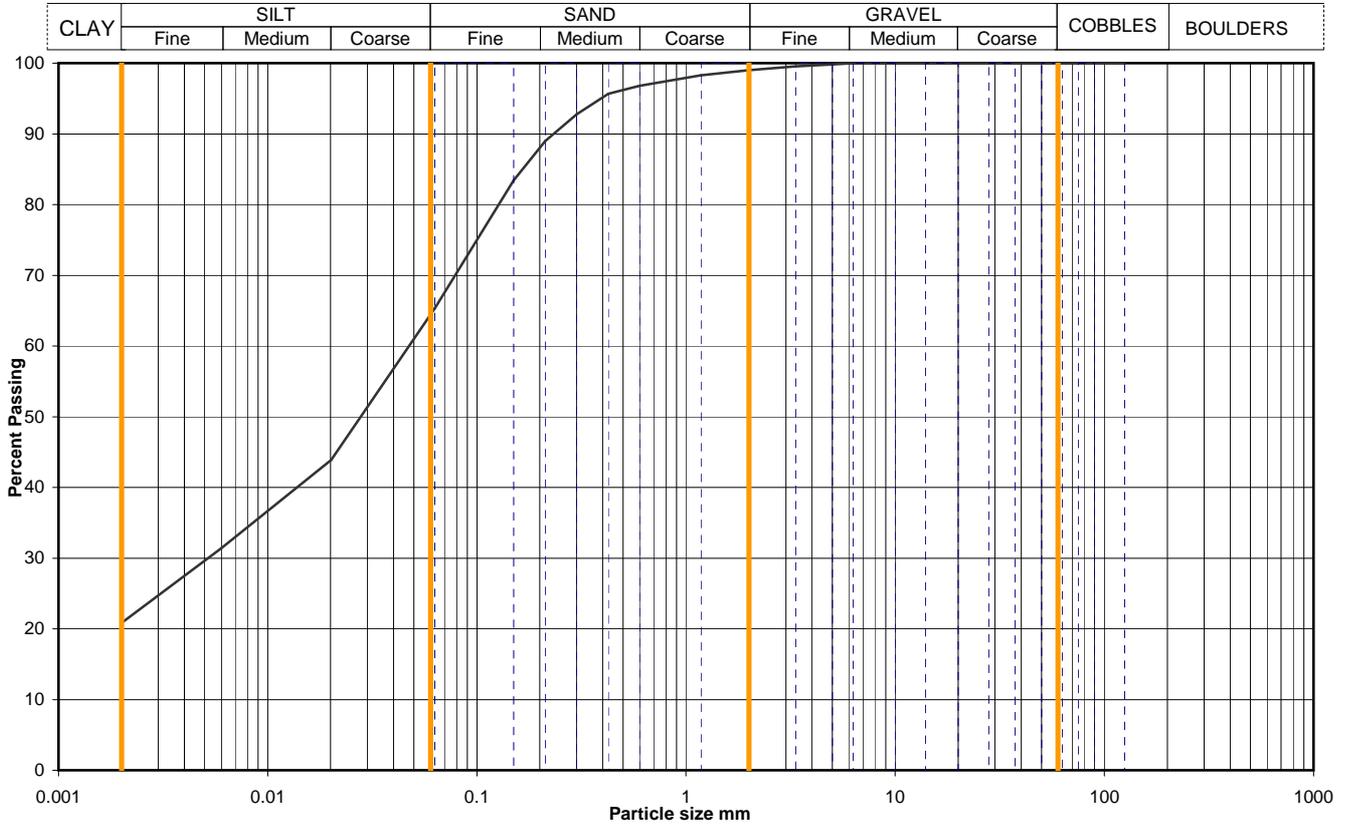


Printed: 18/11/2011 17:30

Figure  
**PSD 9**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110100000000069		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	31
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	100		
2.00	99		
1.18	98		
0.600	97		
0.425	96		
0.300	93		
0.212	89		
0.150	83		
0.063	65		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 7.6
---	-------------------------------

Soil description	Brown slightly gravelly sandy silty CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		1	1
		35	35
		43	43
*<60mm values to aid description only		21	21

<b>Uniformity Coefficient</b>	<b>D<sub>60</sub> / D<sub>10</sub></b>	Not applicable
-------------------------------	--	----------------

<b>Test Method</b>	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

**QA Ref**  
SLR 2,9  
Rev 88  
Aug 11

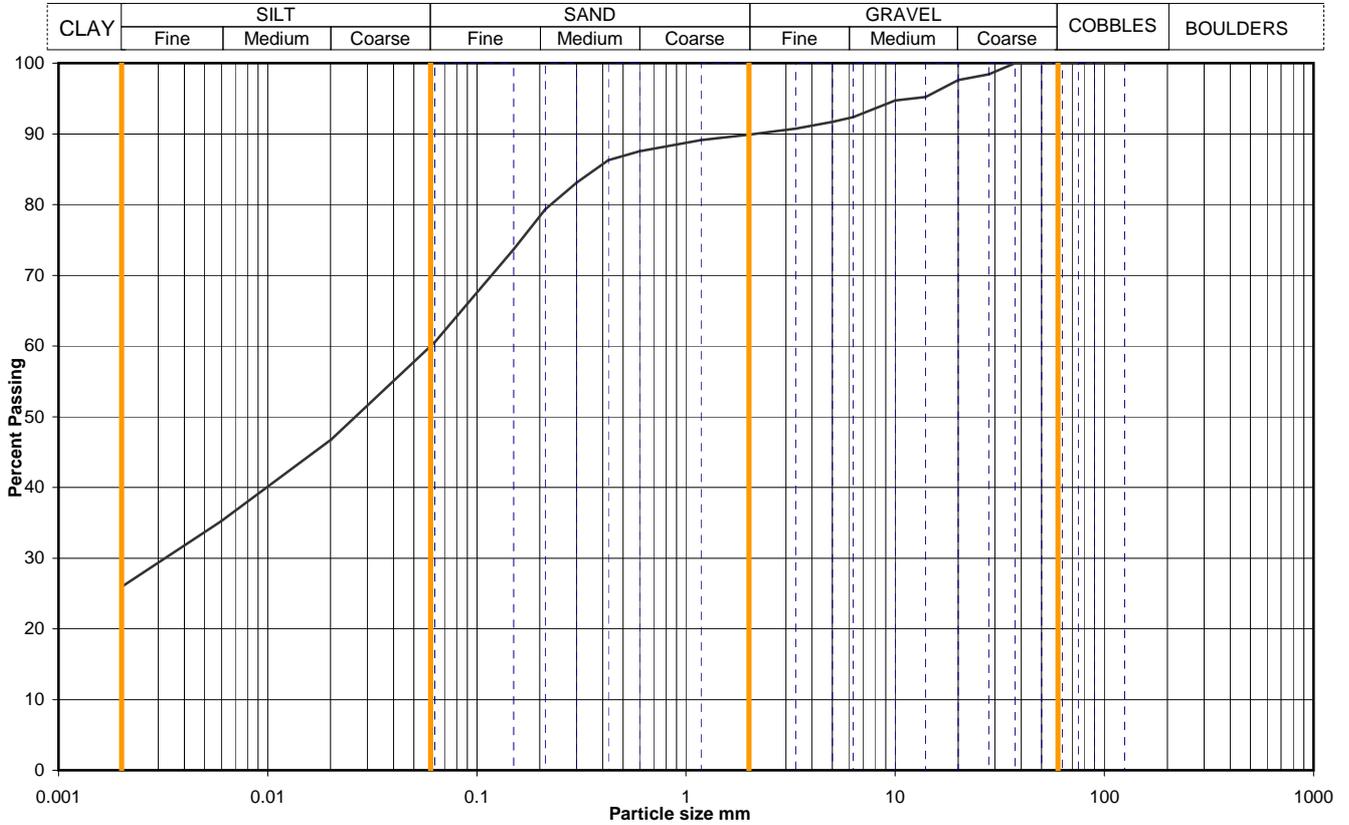


Printed: 18/11/2011 17:30

**Figure**  
**PSD 10**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	7	Type	B
			ID	ESGA1077-11201110100000000073		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	47
90	100	0.0060	35
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	98		
20	98		
14	95		
10	95		
6.3	92		
5.0	92		
3.35	91		
2.00	90		
1.18	89		
0.600	88		
0.425	86		
0.300	83		
0.212	79		
0.150	74		
0.063	61		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	12.2

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		10	10
		30	30
		34	34
*<60mm values to aid description only		26	26

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

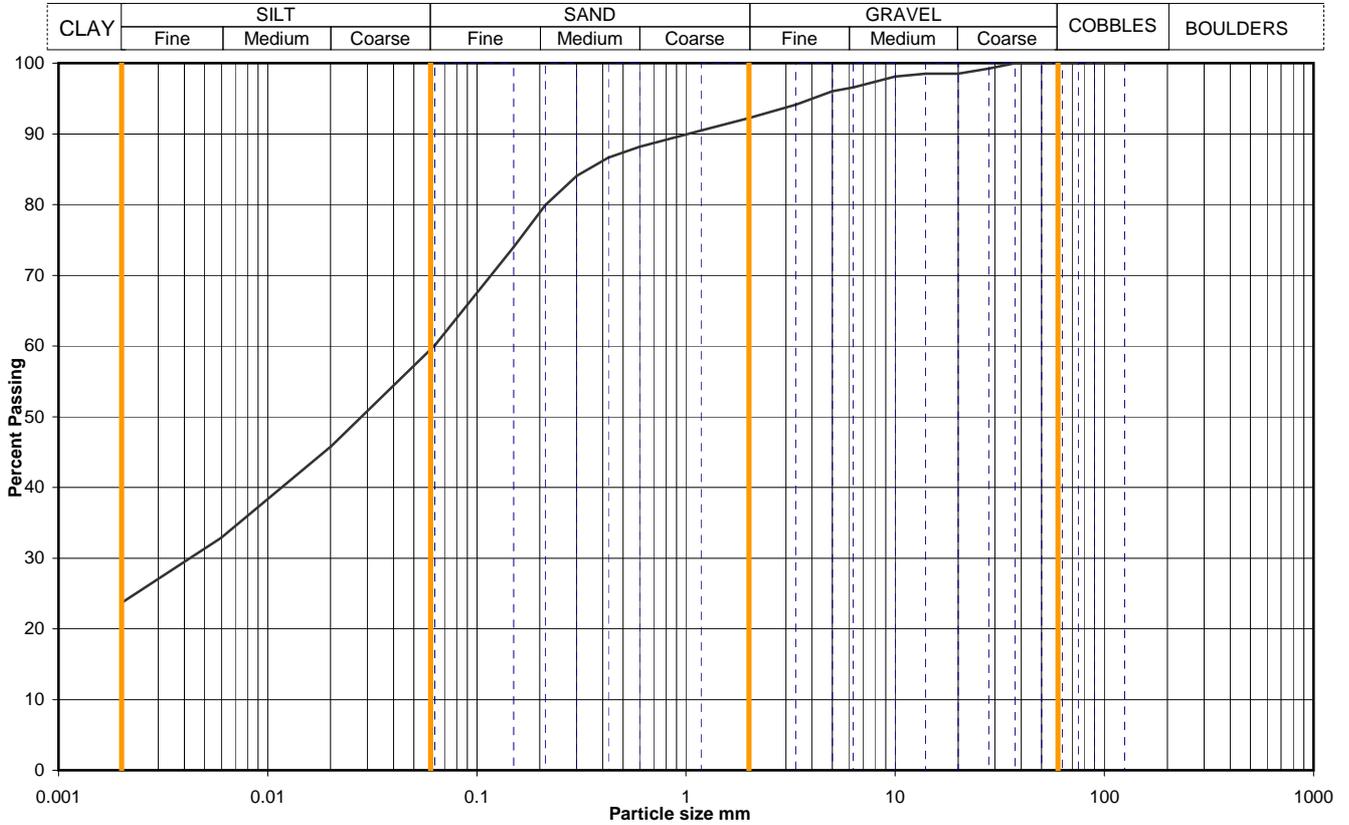


Printed:18/11/2011 17:30

Figure  
**PSD 11**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00		
			Samp No	12	Type	U
			ID	ESGA1077-11201110100000000078		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	46
90	100	0.0060	33
75	100	0.0020	24
63	100		
50	100		
37.5	100		
28	99		
20	98		
14	98		
10	98		
6.3	97		
5.0	96		
3.35	94		
2.00	92		
1.18	90		
0.600	88		
0.425	87		
0.300	84		
0.212	80		
0.150	74		
0.063	60		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	3.8

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		8	8
		33	33
		23	23

\*<60mm values to aid description only

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

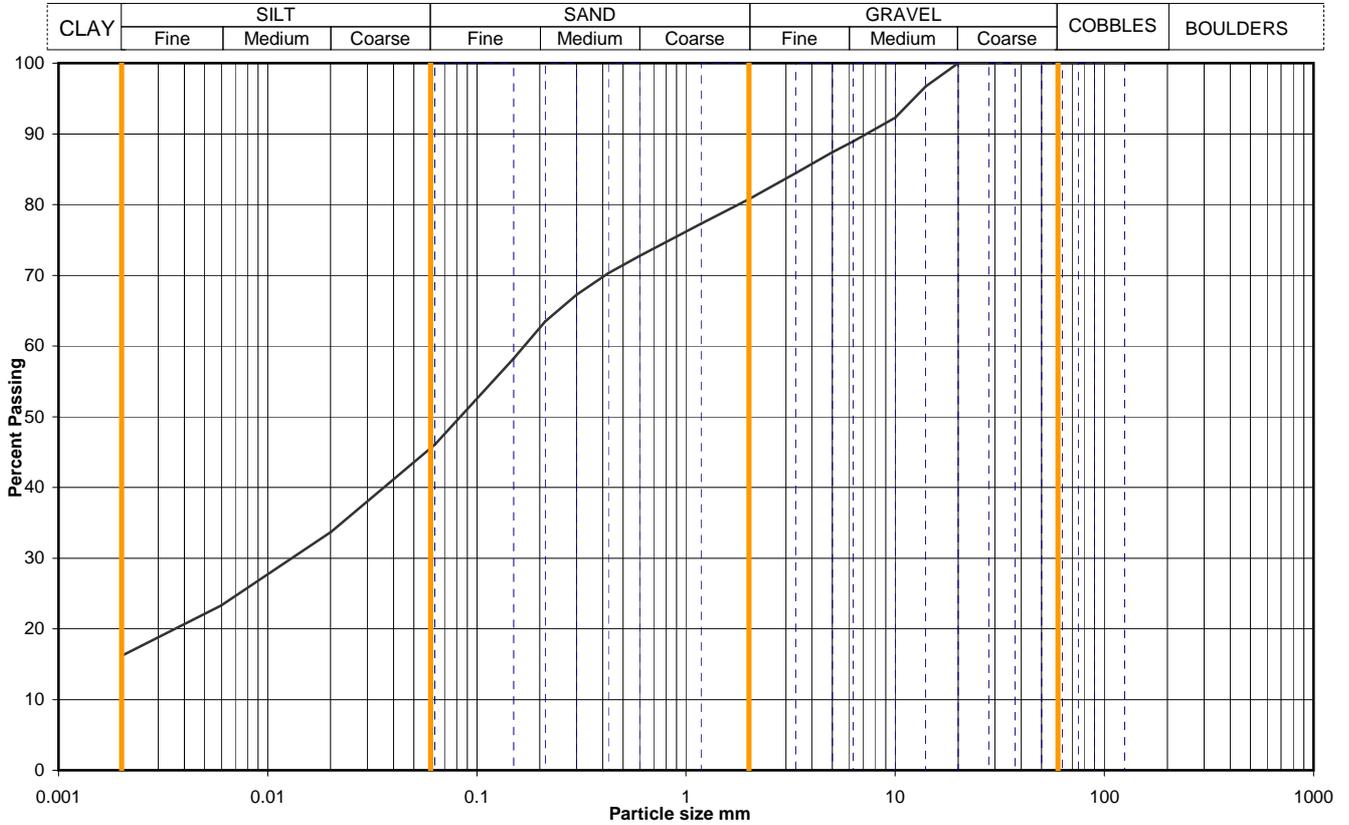


Printed: 18/11/2011 17:30

Figure  
**PSD 12**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.45		
			Samp No	19	Type	D
			ID	ESGA1077-11201110100000000085		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	34
90	100	0.0060	23
75	100	0.0020	16
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	97		
10	92		
6.3	89		
5.0	87		
3.35	84		
2.00	81		
1.18	77		
0.600	73		
0.425	70		
0.300	67		
0.212	63		
0.150	58		
0.063	46		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	1.5

Soil description	Brown slightly gravelly sandy CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		19	19
		35	35
		30	30
*<60mm values to aid description only		16	16

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

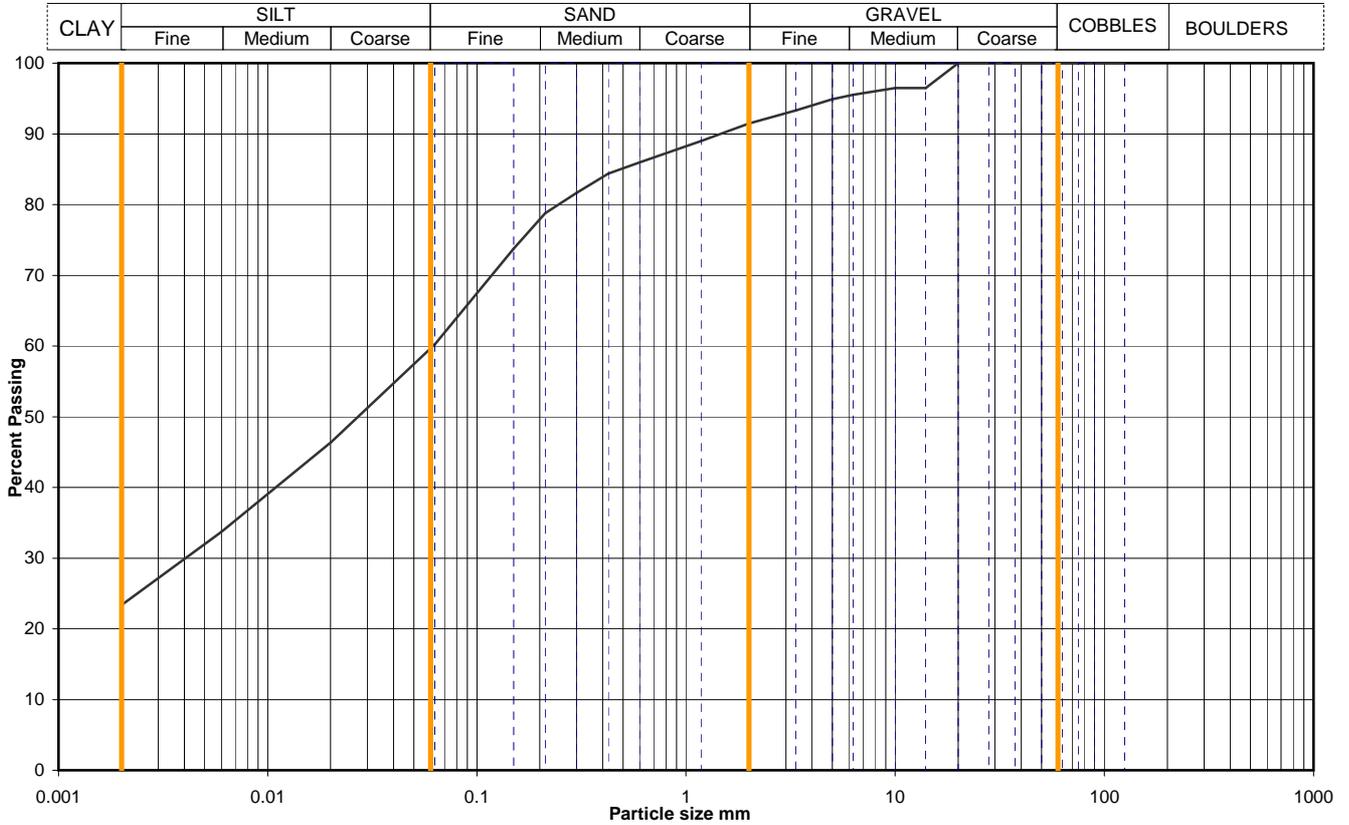


Printed: 18/11/2011 17:30

Figure  
**PSD 13**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	11.45		
			Samp No	25	Type	D
			ID	ESGA1077-1120111010000000091		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	46
90	100	0.0060	34
75	100	0.0020	23
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	96		
10	96		
6.3	96		
5.0	95		
3.35	93		
2.00	91		
1.18	89		
0.600	86	Particle density, Mg/m <sup>3</sup>	
0.425	84	2.65 assumed	
0.300	82	Dry mass of sample, kg	
0.212	79	1.1	
0.150	74		
0.063	60		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions <small>*&lt;60mm values to aid description only</small>	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		9	9
		32	32
		23	23

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

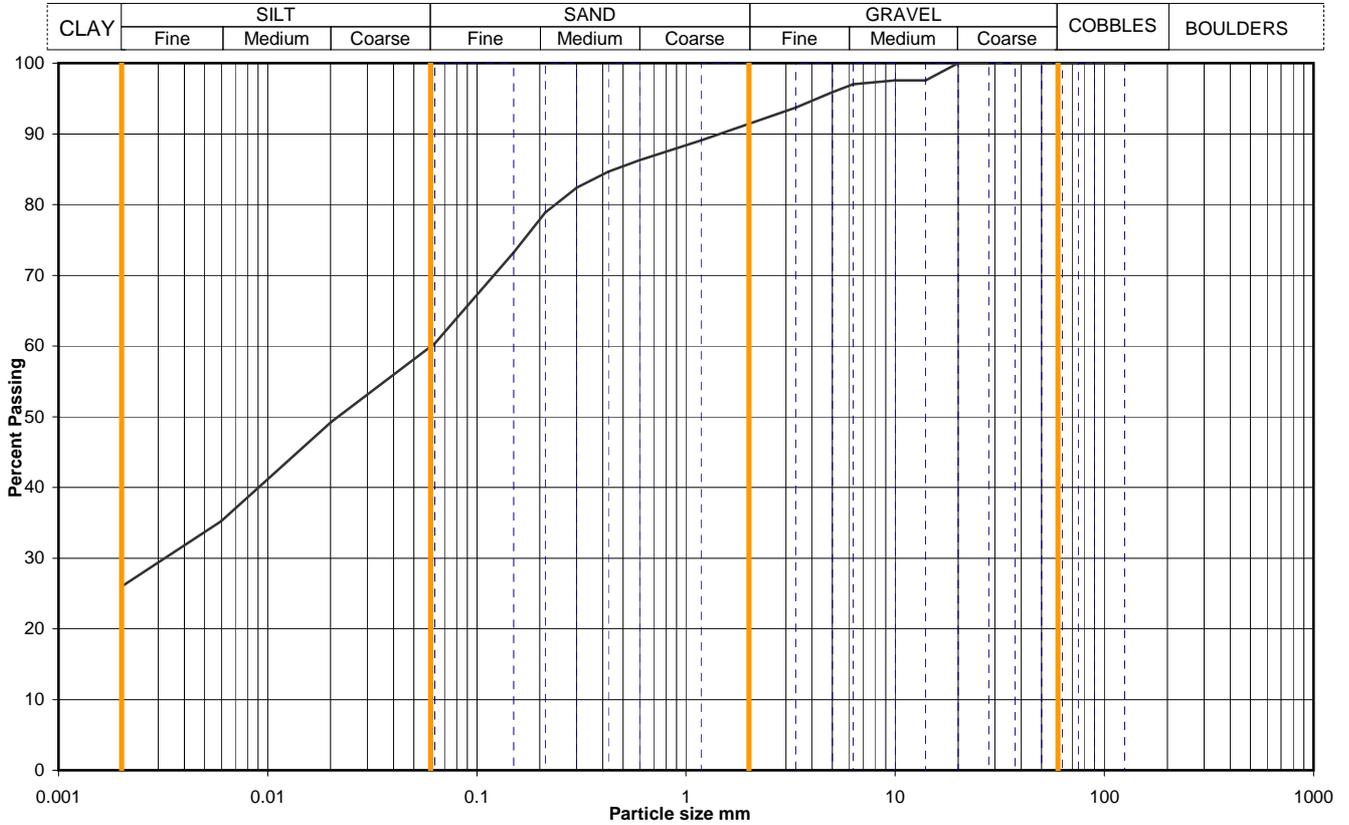


Printed: 18/11/2011 17:30

Figure  
**PSD 14**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANSENDBOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.45		
			Samp No	32	Type	D
			ID	ESGA1077-1120111010000000099		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	49
90	100	0.0060	35
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	98		
10	98		
6.3	97		
5.0	96		
3.35	94		
2.00	91		
1.18	89		
0.600	86	Particle density, Mg/m <sup>3</sup>	
0.425	85	2.65 assumed	
0.300	82	Dry mass of sample, kg	
0.212	79	1.4	
0.150	73		
0.063	60		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		9	9
		32	32
		33	33
*<60mm values to aid description only		26	26

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

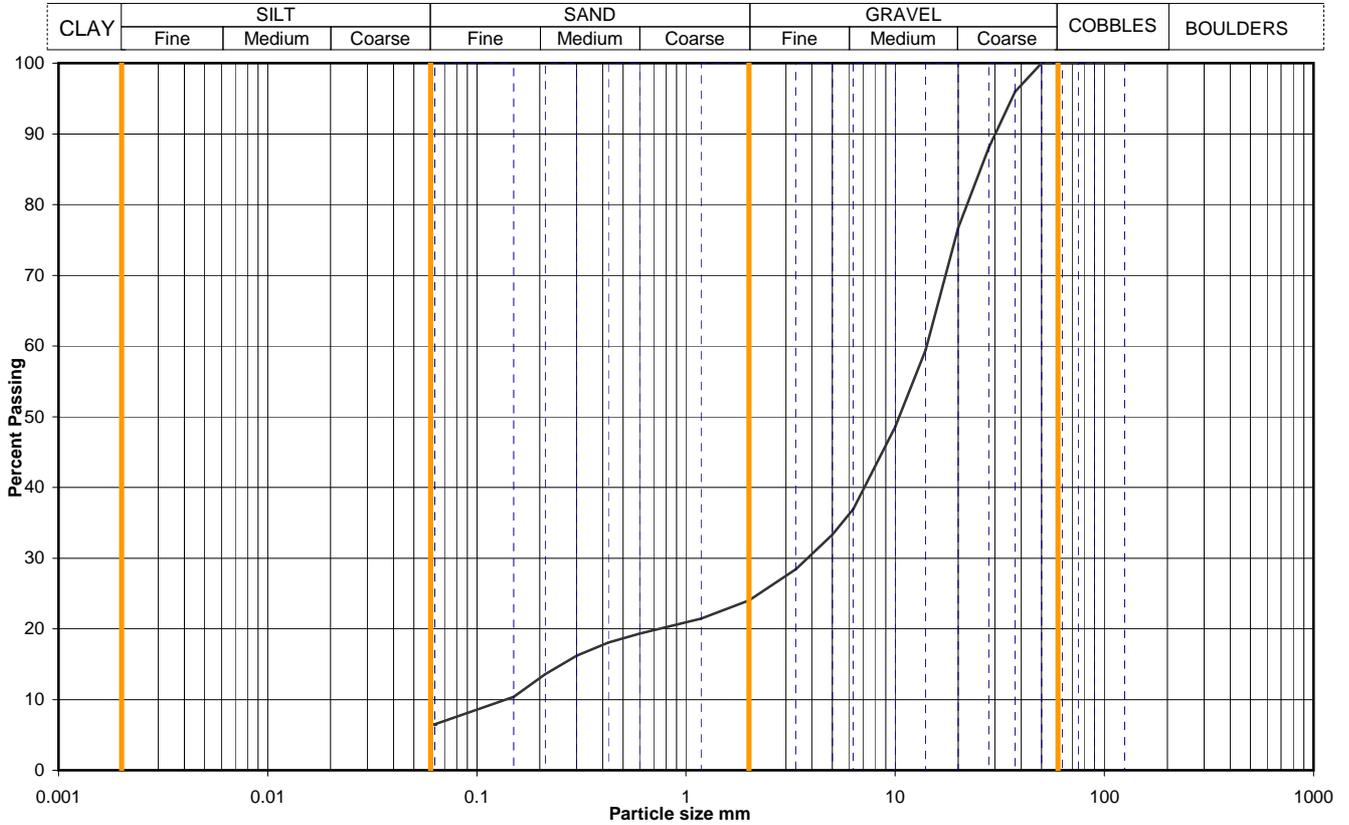


Printed: 18/11/2011 17:30

Figure  
**PSD 15**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	18.00
			Samp No	41
			Type	B
			ID	ESGA1077-11201110100000000108
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	96		
28	88		
20	77		
14	59		
10	49		
6.3	37		
5.0	33		
3.35	28		
2.00	24		
1.18	21		
0.600	19		
0.425	18		
0.300	16		
0.212	14		
0.150	10		
0.063	6		
		Dry mass of sample, kg	
		15.9	

Soil description	Brown clayey sandy GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		76	76
		18	18
		silt+clay =	6

Uniformity Coefficient	$D_{60} / D_{10}$	103
------------------------	-------------------	-----

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

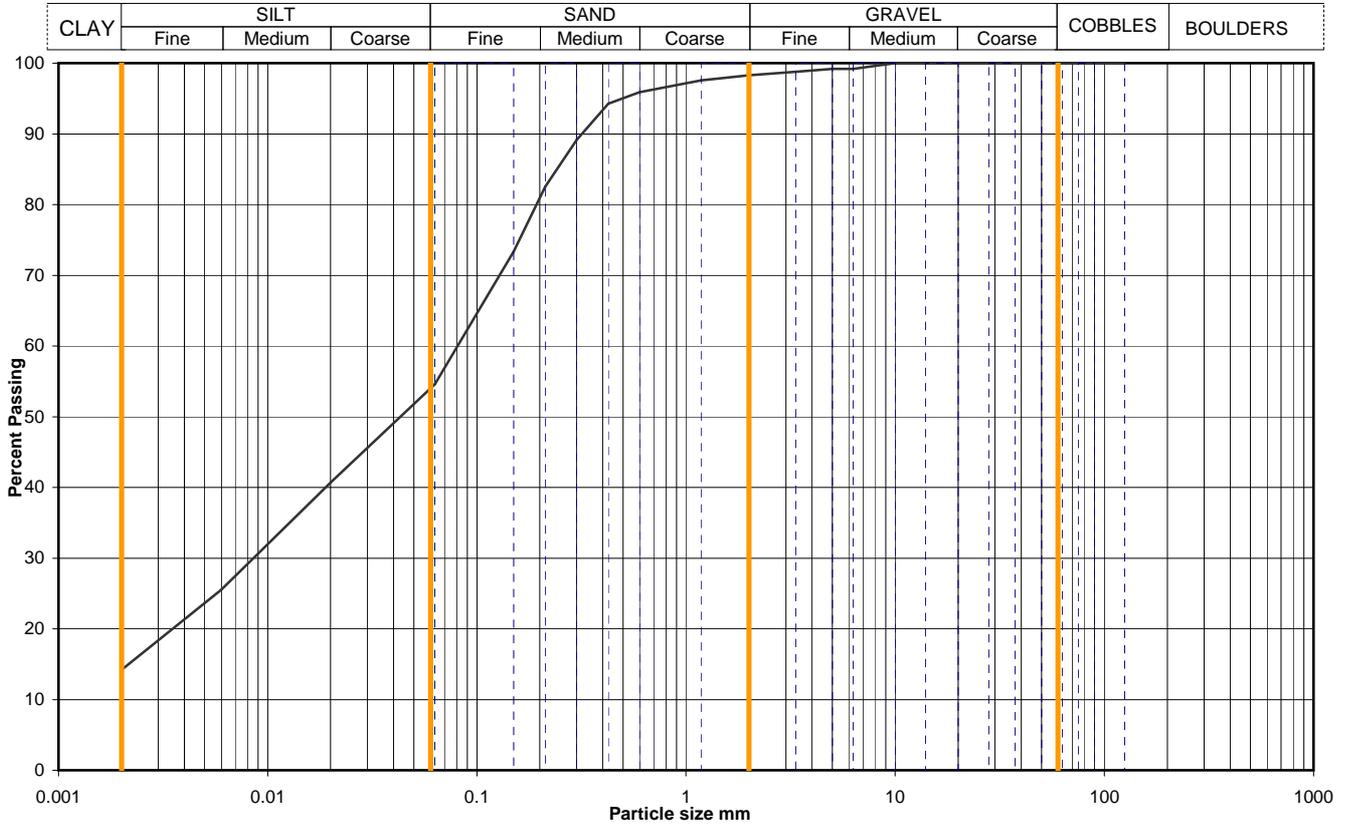


Printed: 18/11/2011 17:30

Figure  
**PSD 16**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110110000000128		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	41
90	100	0.0060	25
75	100	0.0020	14
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	99		
5.0	99		
3.35	99		
2.00	98		
1.18	98		
0.600	96		
0.425	94		
0.300	89		
0.212	83		
0.150	73		
0.063	55		

Particle density, Mg/m <sup>3</sup>	
2.65 assumed	
Dry mass of sample, kg	
3.7	

Soil description	Brown slightly gravelly sandy CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		2	2
		44	44
		14	14

\*<60mm values to aid description only

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

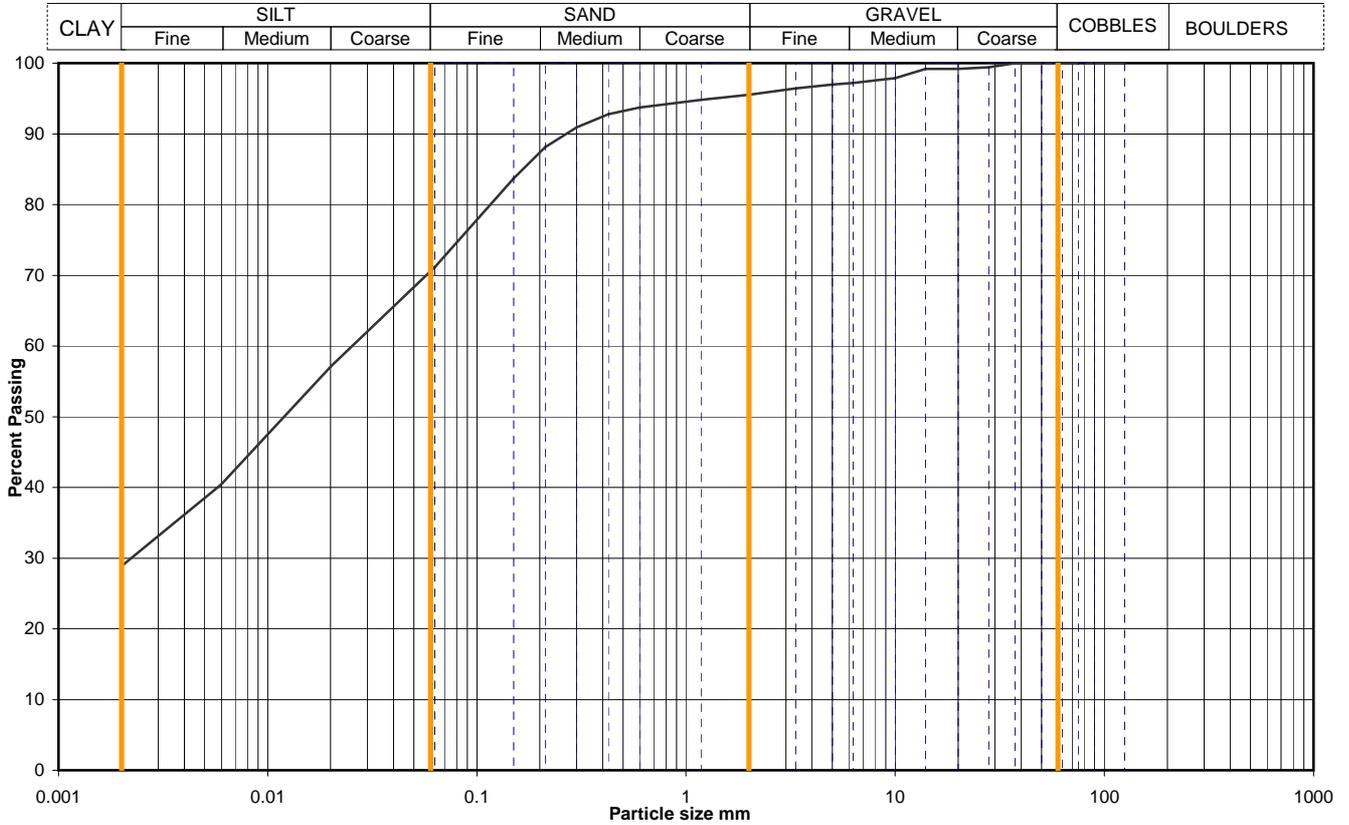


Printed: 18/11/2011 17:30

Figure  
**PSD 17**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANSENDBOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.85		
			Samp No	7	Type	B
			ID	ESGA1077-11201110110000000132		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	57
90	100	0.0060	40
75	100	0.0020	29
63	100		
50	100		
37.5	100		
28	99		
20	99		
14	99		
10	98		
6.3	97		
5.0	97		
3.35	96		
2.00	96		
1.18	95		
0.600	94		
0.425	93		
0.300	91		
0.212	88		
0.150	84		
0.063	71		

Particle density, Mg/m <sup>3</sup>	
2.65	assumed
Dry mass of sample, kg	
11.4	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		4	4
		25	25
		42	42
*<60mm values to aid description only		29	29

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

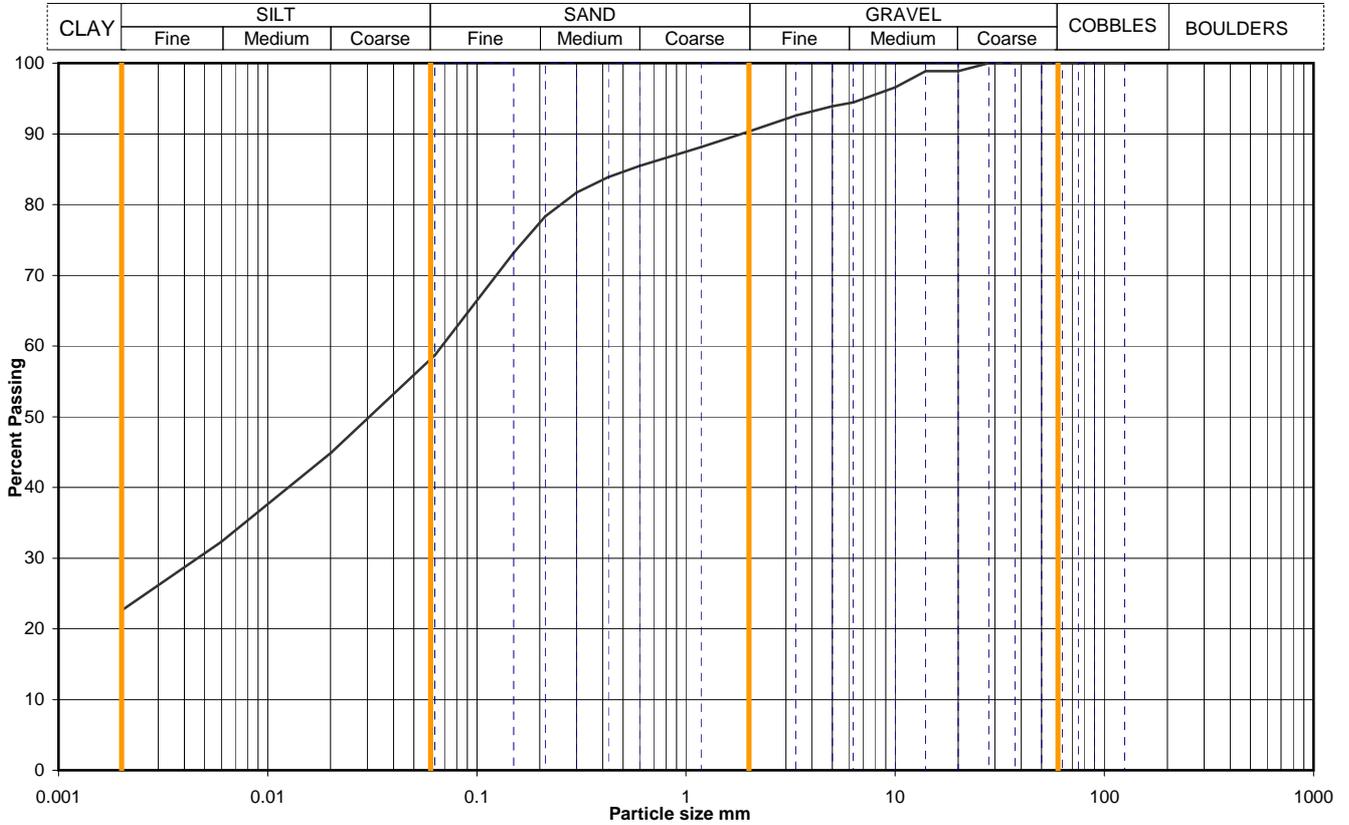


Printed: 18/11/2011 17:30

Figure  
**PSD 18**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65
			Samp No	15
			Type	B
			ID	ESGA1077-11201110110000000140
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	45
90	100	0.0060	32
75	100	0.0020	23
63	100		
50	100		
37.5	100		
28	100		
20	99		
14	99		
10	97		
6.3	94		
5.0	94		
3.35	93		
2.00	90		
1.18	88		
0.600	85		
0.425	84		
0.300	82		
0.212	78		
0.150	73		
0.063	59		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	16.8

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		10	10
		32	32
		23	23

\*<60mm values to aid description only

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

**QA Ref**  
SLR 2,9  
Rev 88  
Aug 11

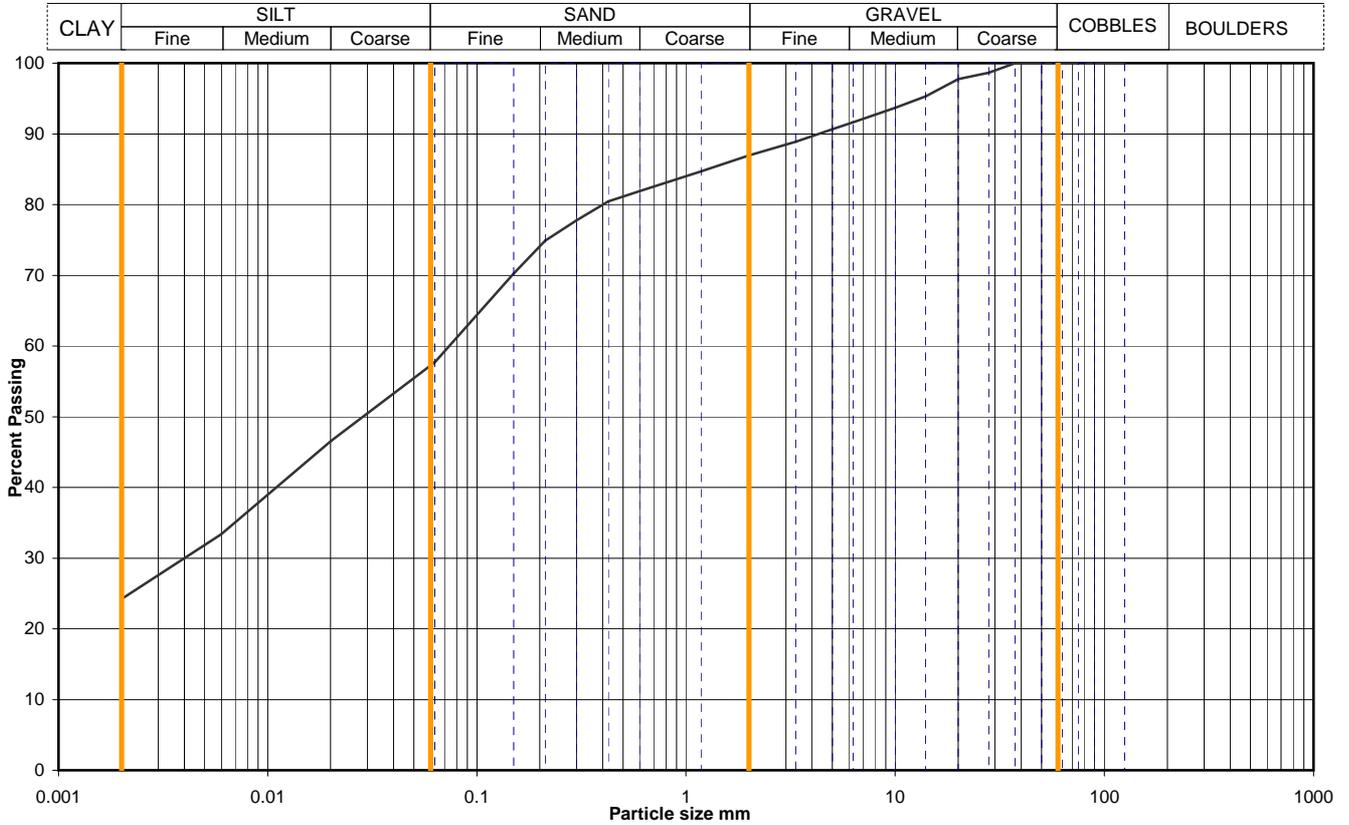


Printed: 18/11/2011 17:30

**Figure**  
**PSD 19**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	7.65		
			Samp No	23	Type	B
			ID	ESGA1077-11201110110000000148		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	47
90	100	0.0060	33
75	100	0.0020	24
63	100		
50	100		
37.5	100		
28	99		
20	98		
14	95		
10	94		
6.3	92		
5.0	91		
3.35	89		
2.00	87		
1.18	85		
0.600	82		
0.425	80		
0.300	78		
0.212	75		
0.150	70		
0.063	58		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	16.4

Soil description	Greyish brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		13	13
		30	30
		33	33
*<60mm values to aid description only		24	24

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

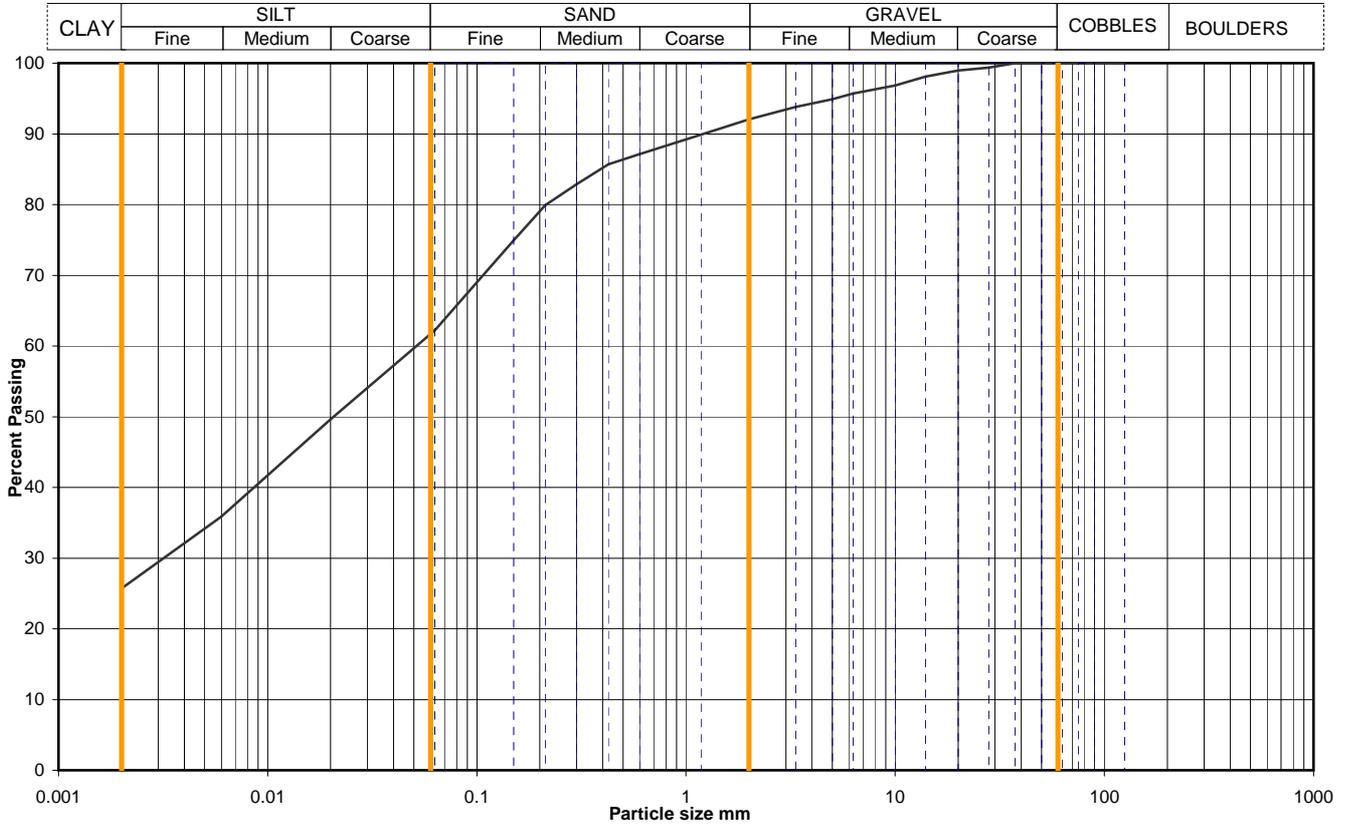


Printed: 18/11/2011 17:30

Figure  
**PSD 20**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	9.15		
			Samp No	27	Type	B
			ID	ESGA1077-11201110110000000152		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	50
90	100	0.0060	36
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	99		
20	99		
14	98		
10	97		
6.3	96		
5.0	95		
3.35	94		
2.00	92		
1.18	90		
0.600	87		
0.425	86		
0.300	83		
0.212	80		
0.150	75		
0.063	62		

Particle density, Mg/m <sup>3</sup>	
2.65	assumed
Dry mass of sample, kg	
14.6	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		8	8
		30	30
		36	36
*<60mm values to aid description only		26	26

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

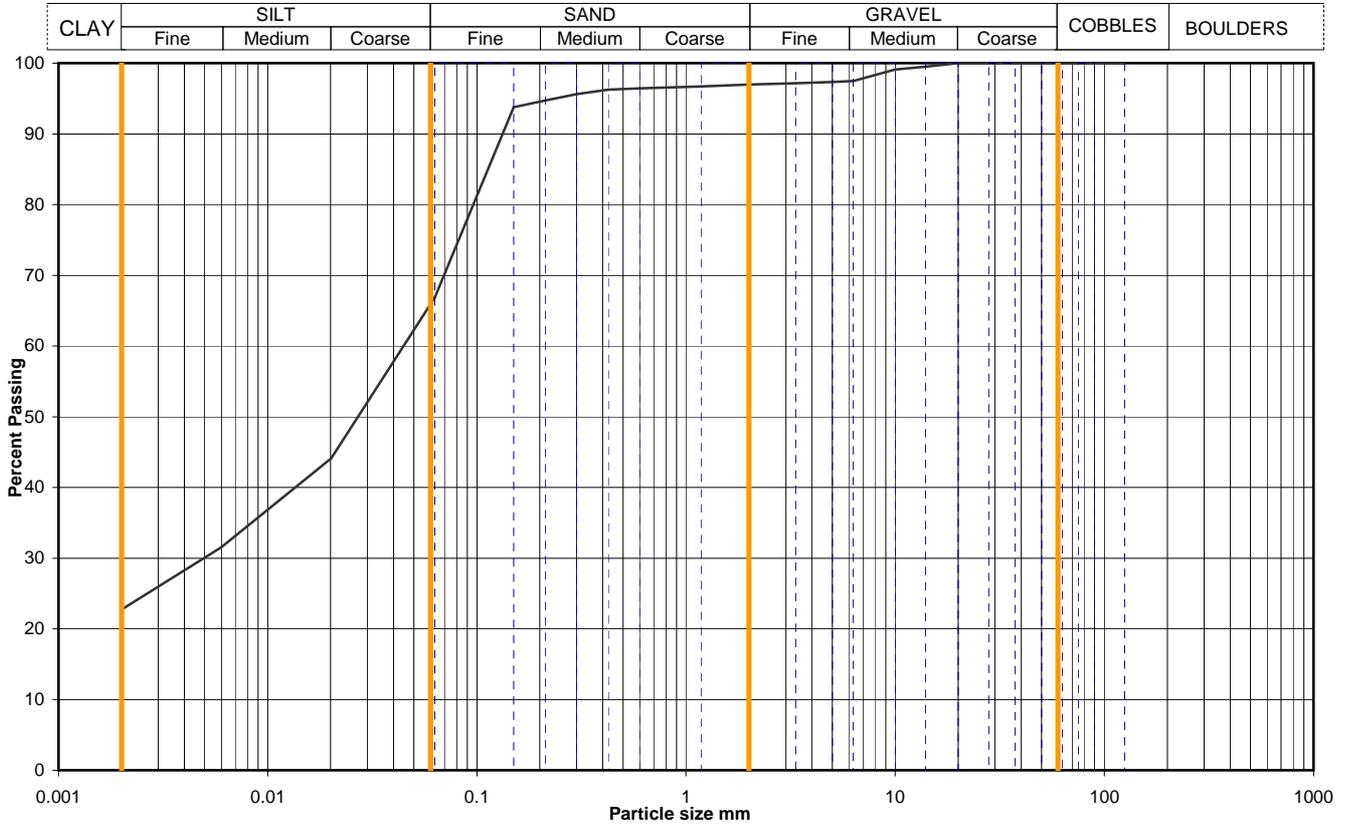


Printed: 18/11/2011 17:30

Figure  
**PSD 21**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	11.50	
		Samp No	34	Type	B
		ID	ESGA1077-11201110110000000159		
		Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	32
75	100	0.0020	23
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	99		
10	99		
6.3	97		
5.0	97		
3.35	97		
2.00	97		
1.18	97		
0.600	96		
0.425	96		
0.300	96		
0.212	95		
0.150	94		
0.063	67		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	14.6

Soil description	Light brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		3	3
		31	31
		43	43
*<60mm values to aid description only		23	23

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

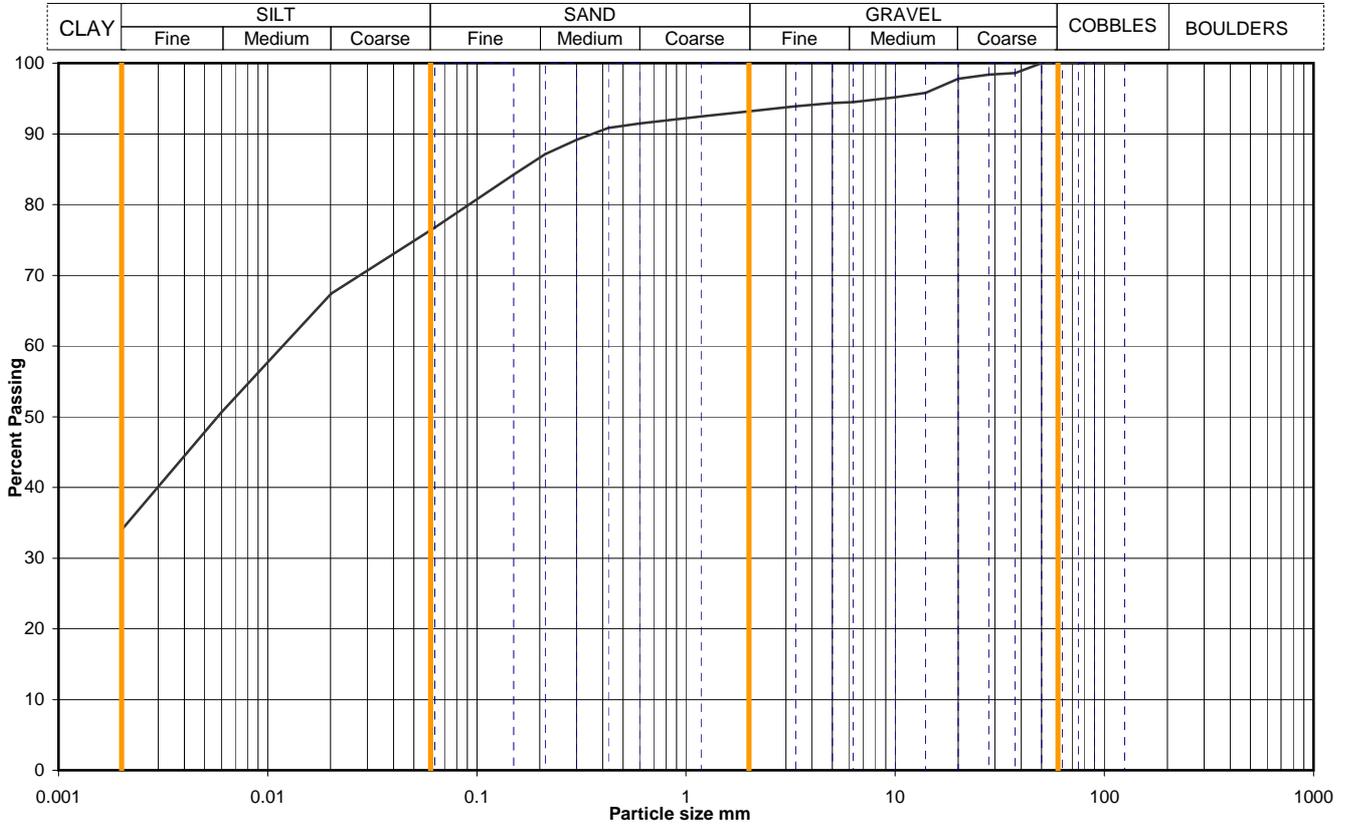


Printed: 18/11/2011 17:30

Figure  
**PSD 22**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	13.50
			Samp No	38
			Type	B
			ID	ESGA1077-11201110110000000163
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	67
90	100	0.0060	51
75	100	0.0020	34
63	100		
50	100		
37.5	99		
28	98		
20	98		
14	96		
10	95		
6.3	94		
5.0	94		
3.35	94		
2.00	93		
1.18	92		
0.600	91		
0.425	91		
0.300	89		
0.212	87		
0.150	84		
0.063	77		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 13.1
---	--------------------------------

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		7	7
		17	17
		42	42
*<60mm values to aid description only		34	34

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

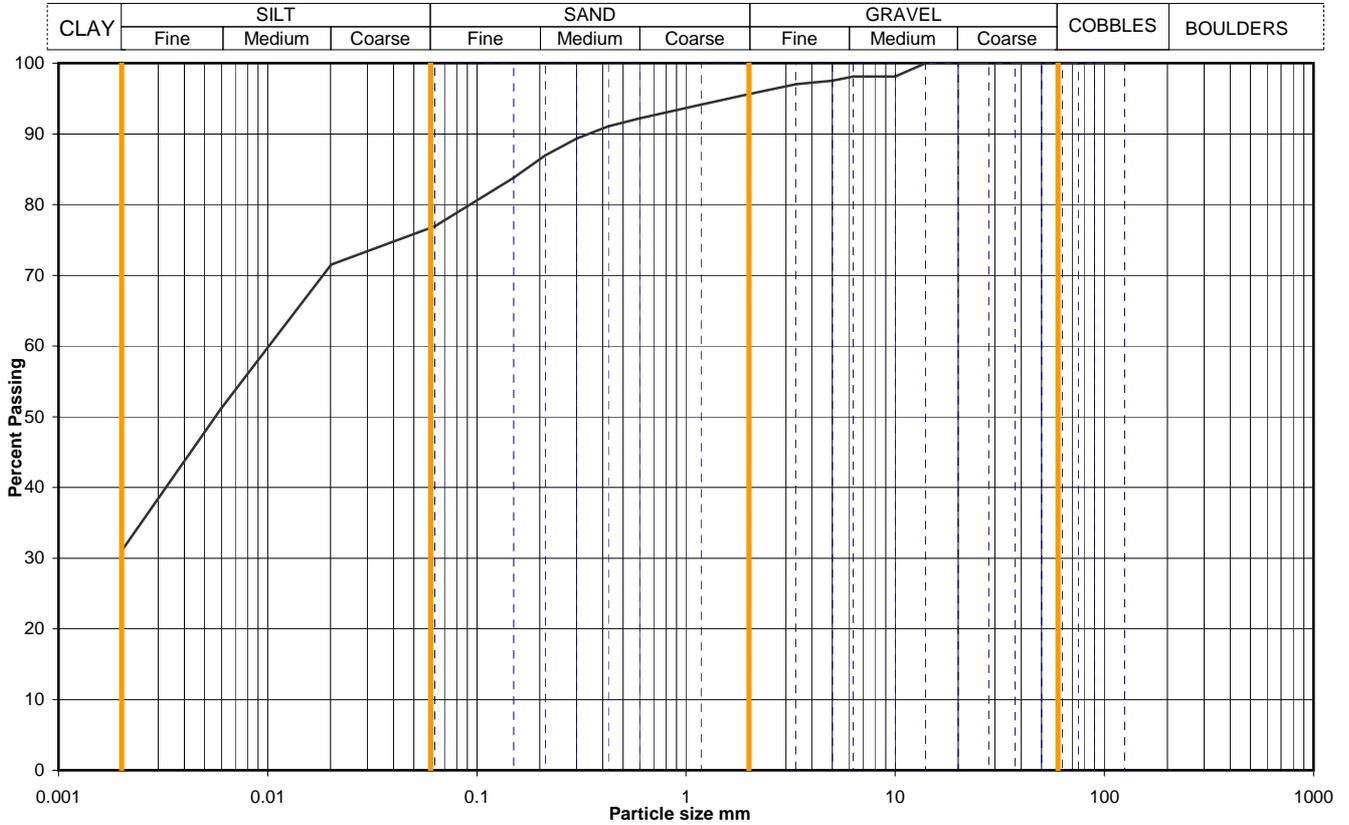


Printed: 18/11/2011 17:30

Figure  
**PSD 23**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	13.50		
			Samp No	37	Type	D
			ID	ESGA1077-11201110110000000162		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	72
90	100	0.0060	51
75	100	0.0020	31
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	98		
6.3	98		
5.0	98		
3.35	97		
2.00	96		
1.18	94		
0.600	92		
0.425	91		
0.300	89		
0.212	87		
0.150	84		
0.063	77		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	0.4

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: pre dried, Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		4	4
		19	19
		46	46
*<60mm values to aid description only		31	31

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

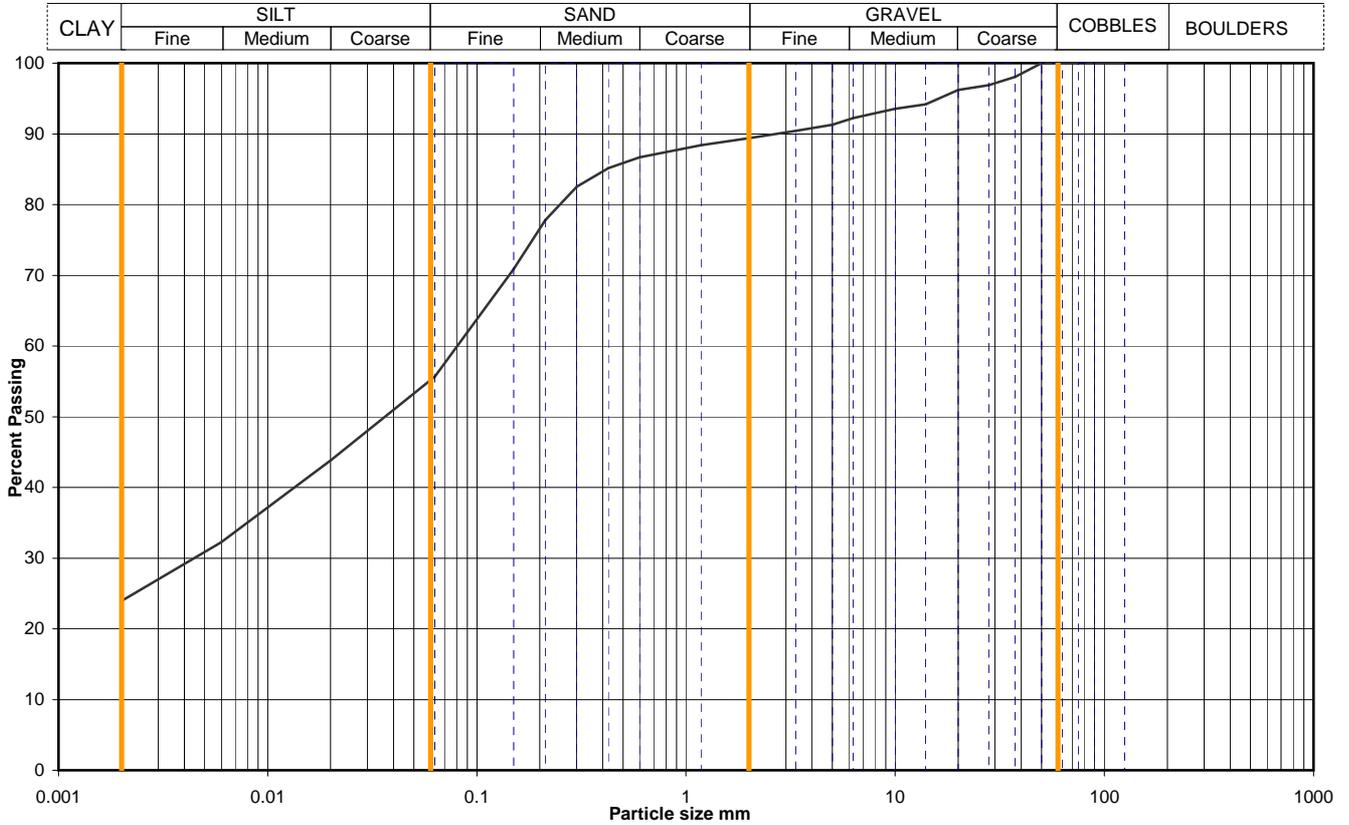


Printed: 18/11/2011 17:30

Figure  
**PSD 24**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	16.65		
			Samp No	46	Type	B
			ID	ESGA1077-11201110110000000170		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	44
90	100	0.0060	32
75	100	0.0020	24
63	100		
50	100		
37.5	98		
28	97		
20	96		
14	94		
10	94		
6.3	92		
5.0	91		
3.35	90		
2.00	89		
1.18	88		
0.600	87		
0.425	85		
0.300	83		
0.212	78		
0.150	71		
0.063	56		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	15.0

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		11	11
		34	34
		31	31
*<60mm values to aid description only		24	24

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

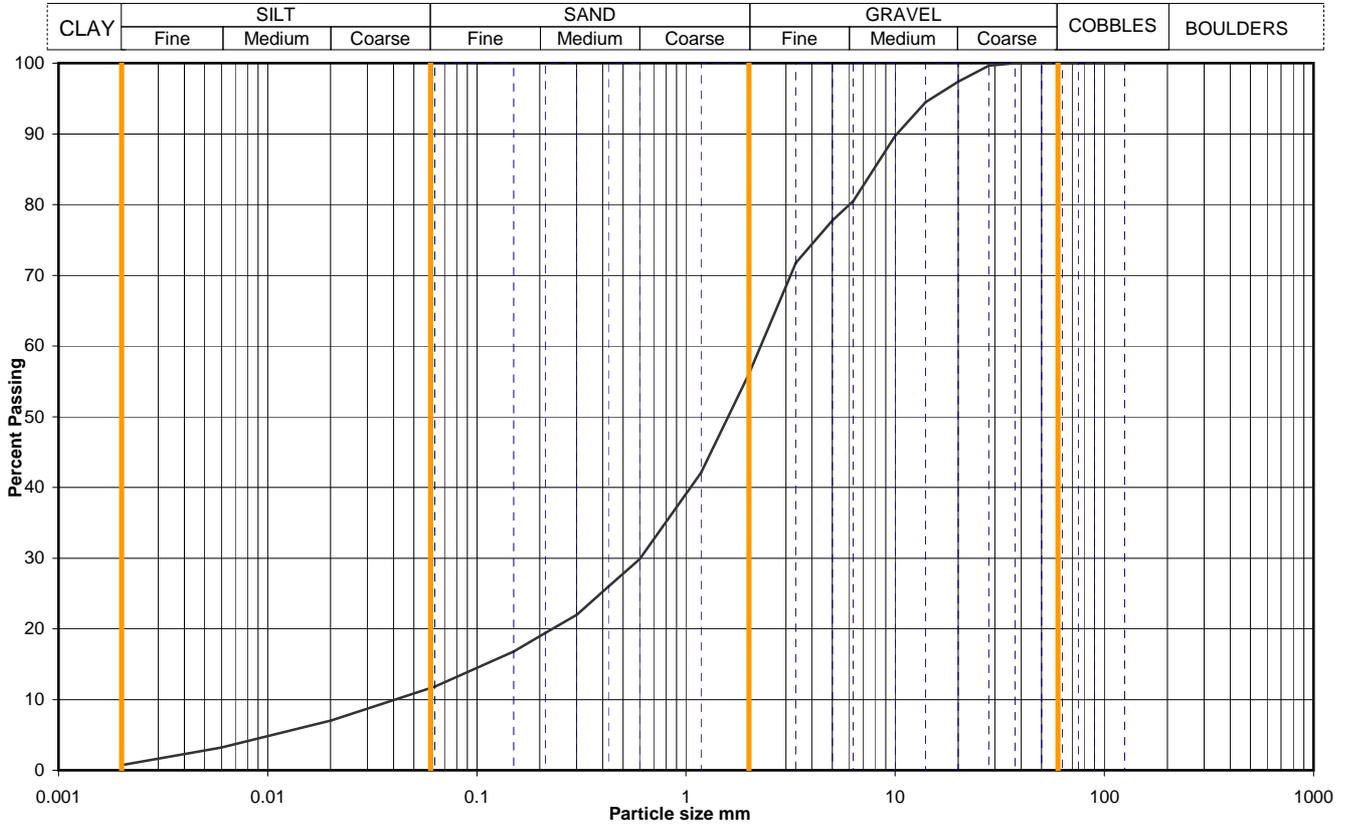


Printed: 18/11/2011 17:30

Figure  
**PSD 25**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH4		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.20		
			Samp No	4	Type	B
			ID	ESGA1077-11201110110000000184		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	7
90	100	0.0060	3
75	100	0.0020	1
63	100		
50	100		
37.5	100		
28	100		
20	97		
14	95		
10	90		
6.3	81		
5.0	78		
3.35	72		
2.00	56		
1.18	42		
0.600	30	Particle density, Mg/m <sup>3</sup>	
0.425	26	2.65 assumed	
0.300	22	Dry mass of sample, kg	
0.212	19	8.0	
0.150	17		
0.063	12		

Soil description	Light grey sandy gravelly SILT.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions <small>*&lt;60mm values to aid description only</small>	Cobbles / boulders	Whole	*<60mm
	Gravel	0	0
	Sand	44	44
	Silt	44	44
	Clay	11	11

Uniformity Coefficient	$D_{60} / D_{10}$	56
------------------------	-------------------	----

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

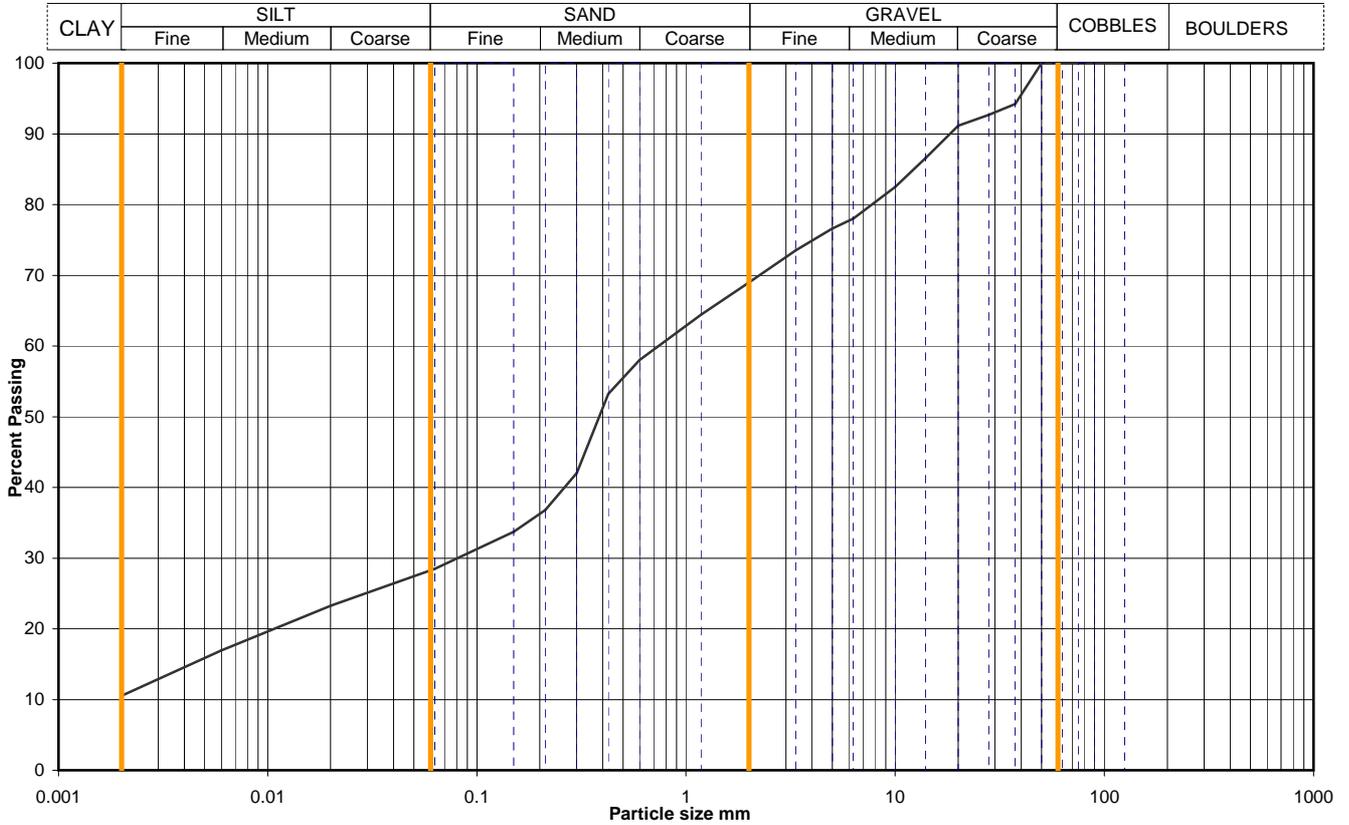


Printed: 18/11/2011 17:30

Figure  
**PSD 26**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH4
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.80
			Samp No	8
			Type	B
			ID	ESGA1077-11201110110000000188
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	23
90	100	0.0060	17
75	100	0.0020	11
63	100		
50	100		
37.5	94		
28	93		
20	91		
14	87		
10	82		
6.3	78		
5.0	77		
3.35	74		
2.00	69		
1.18	64		
0.600	58		
0.425	53		
0.300	42		
0.212	37		
0.150	34		
0.063	28		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	5.0

Soil description	Brown slightly gravelly sandy CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		31	31
		41	41
		18	18
*<60mm values to aid description only		10	10

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

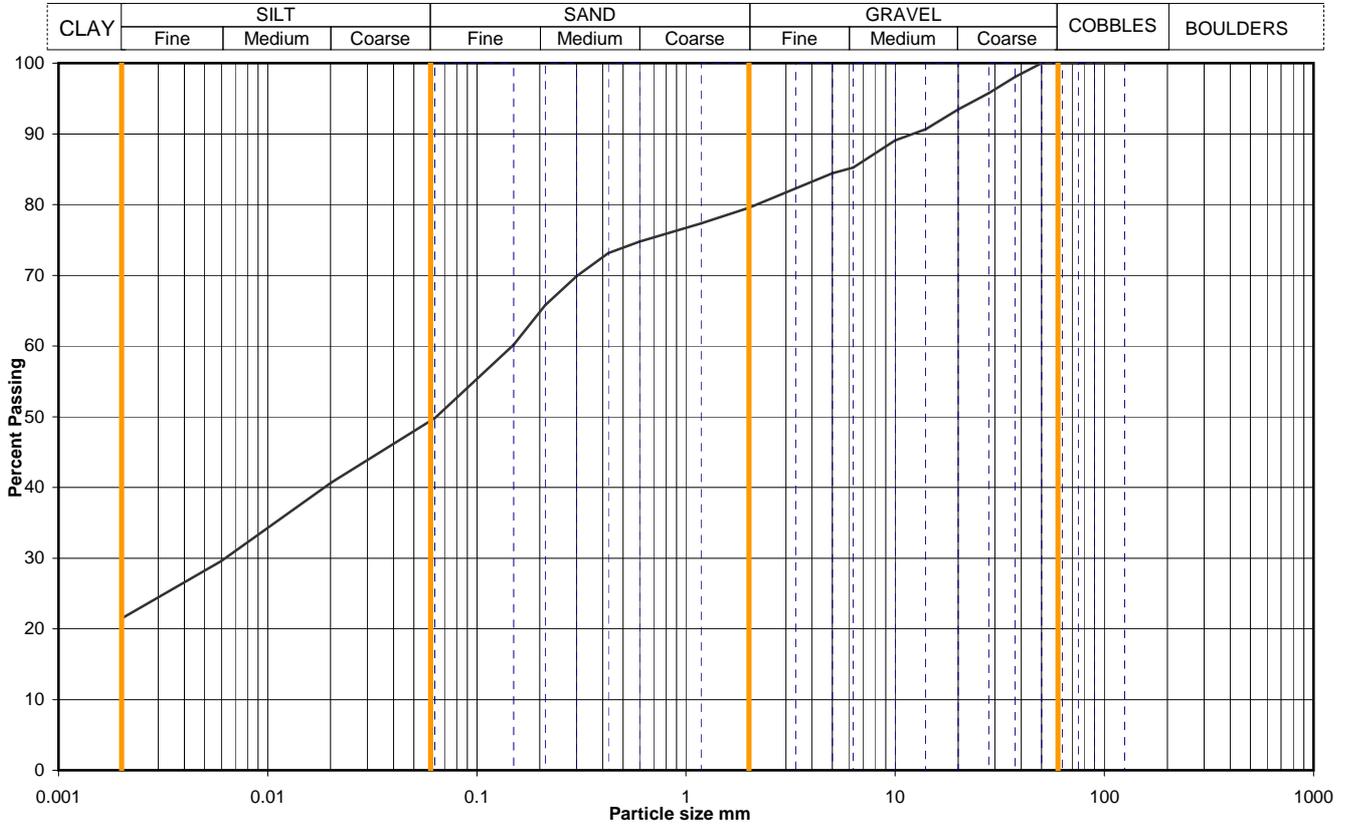


Printed: 18/11/2011 17:30

Figure  
**PSD 27**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH4
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	14
			Type	B
			ID	ESGA1077-11201110110000000195
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	41
90	100	0.0060	30
75	100	0.0020	21
63	100		
50	100		
37.5	98		
28	96		
20	93		
14	91		
10	89		
6.3	85		
5.0	84		
3.35	82		
2.00	80		
1.18	77		
0.600	75		
0.425	73		
0.300	70		
0.212	66		
0.150	60		
0.063	50		

Particle density, Mg/m <sup>3</sup>	2.65 assumed
Dry mass of sample, kg	10.8

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		20	20
		30	30
		28	28
		22	22

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

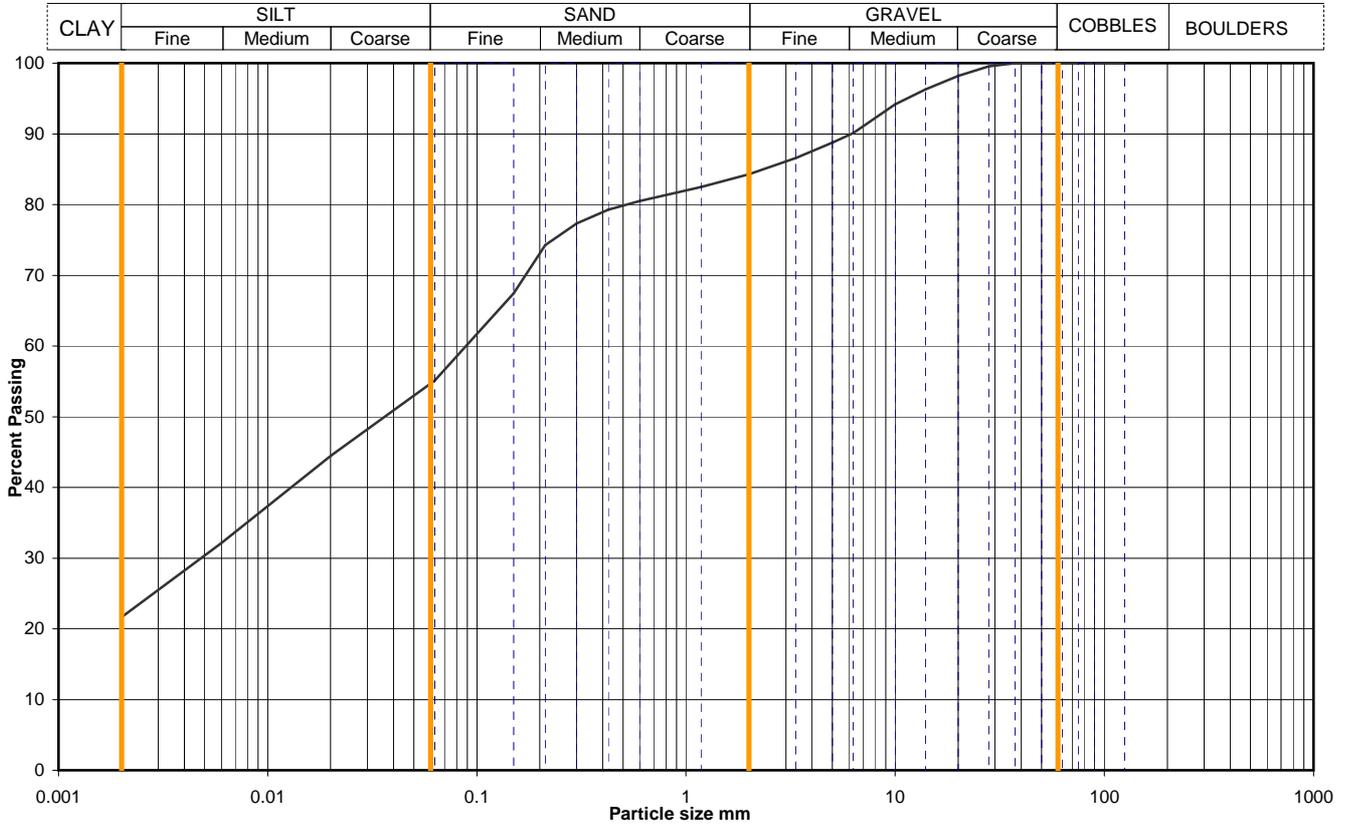


Printed: 18/11/2011 17:30

Figure  
**PSD 28**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH4		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	20	Type	B
			ID	ESGA1077-11201110110000000201		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	45
90	100	0.0060	32
75	100	0.0020	22
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	96		
10	94		
6.3	90		
5.0	89		
3.35	87		
2.00	84		
1.18	82		
0.600	80		
0.425	79		
0.300	77		
0.212	74		
0.150	67		
0.063	55		
		Particle density, Mg/m <sup>3</sup>	
		2.65 assumed	
		Dry mass of sample, kg	
		9.1	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		16	16
		30	30
		33	33
*<60mm values to aid description only		21	21

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

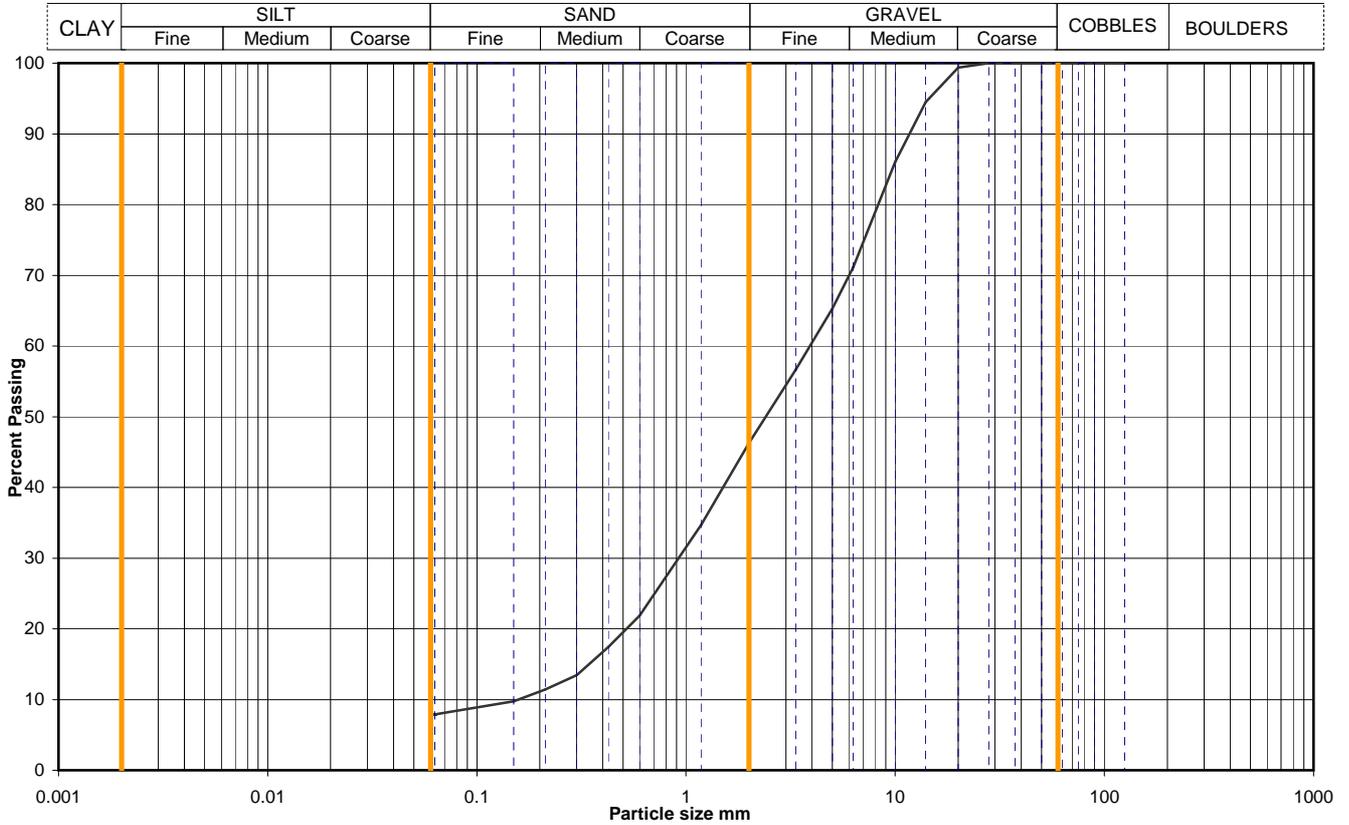


Printed: 18/11/2011 17:30

Figure  
**PSD 29**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.10		
			Samp No	2	Type	B
			ID	ESGA1077-11201110130000000250		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	99		
14	94		
10	86		
6.3	71		
5.0	65		
3.35	57		
2.00	46		
1.18	35		
0.600	22		
0.425	17		
0.300	13		
0.212	11		
0.150	10		
0.063	8		
		Dry mass of sample, kg	
		11.8	

Soil description	Dark grey very sandy clayey GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		54	54
		38	38
		silt+clay =	8

Uniformity Coefficient	$D_{60} / D_{10}$	25
------------------------	-------------------	----

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

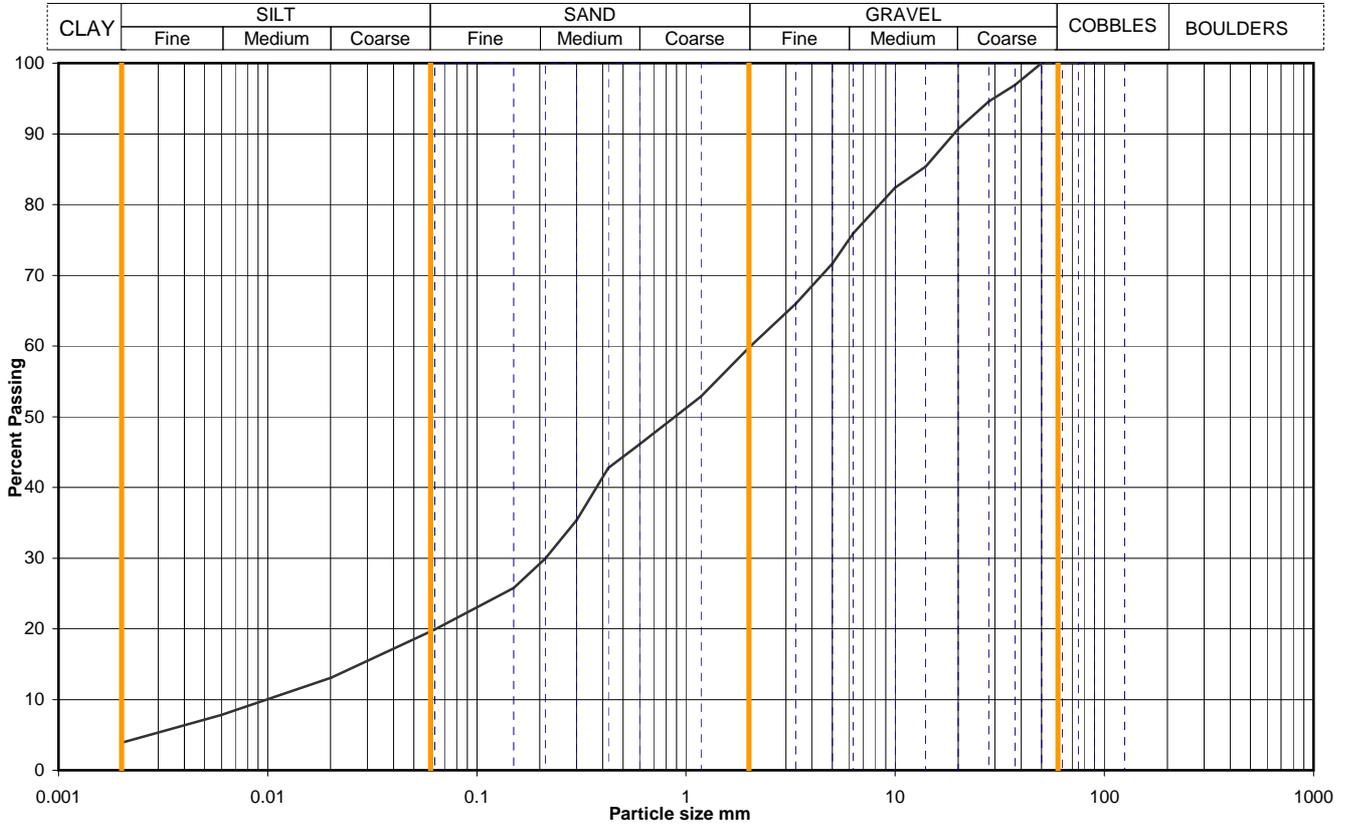


Printed: 18/11/2011 17:31

Figure  
**PSD 30**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.90		
			Samp No	6	Type	B
			ID	ESGA1077-1120111013000000254		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	13
90	100	0.0060	8
75	100	0.0020	4
63	100		
50	100		
37.5	97		
28	95		
20	91		
14	85		
10	82		
6.3	76		
5.0	72		
3.35	66		
2.00	60		
1.18	53	Particle density, Mg/m <sup>3</sup> 2.65 assumed	
0.600	46		
0.425	43		
0.300	35	Dry mass of sample, kg 10.1	
0.212	30		
0.150	26		
0.063	20		

Soil description	Grey sandy gravelly SILT.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions <small>*&lt;60mm values to aid description only</small>	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		40	40
		40	40
		16	16
		4	4

Uniformity Coefficient	$D_{60} / D_{10}$	206
------------------------	-------------------	-----

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

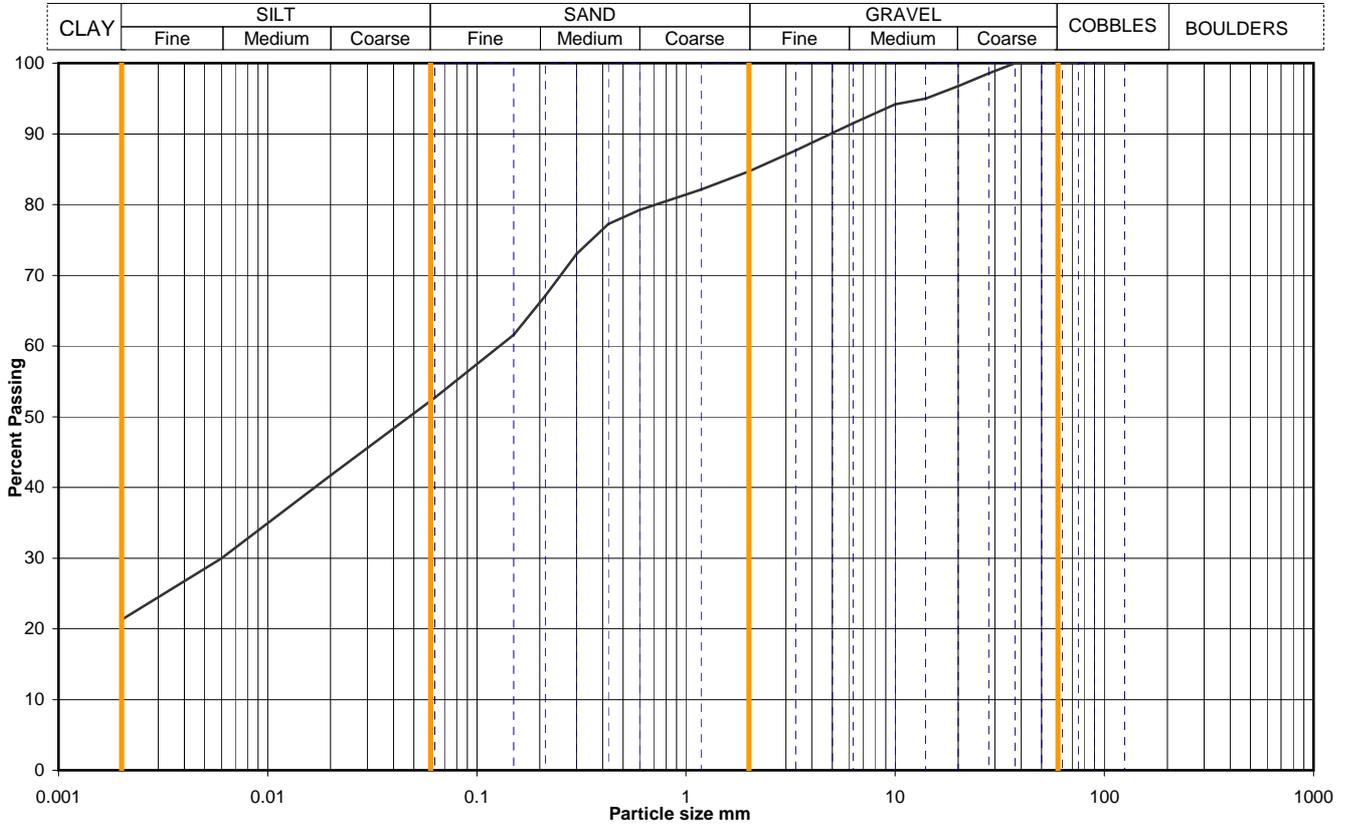


Printed: 18/11/2011 17:31

Figure  
**PSD 31**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.20		
			Samp No	9	Type	B
			ID	ESGA1077-11201110130000000257		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	42
90	100	0.0060	30
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	99		
20	97		
14	95		
10	94		
6.3	92		
5.0	90		
3.35	88		
2.00	85		
1.18	82		
0.600	79		
0.425	77		
0.300	73		
0.212	67		
0.150	62		
0.063	53		

Particle density, Mg/m <sup>3</sup>	
2.65	assumed
Dry mass of sample, kg	
7.2	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions <small>*&lt;60mm values to aid description only</small>	Cobbles / boulders	Whole	*<60mm
	Gravel	0	0
	Sand	15	15
	Silt	33	33
	Clay	21	21

<b>Uniformity Coefficient</b>	<b>D<sub>60</sub> / D<sub>10</sub></b>	Not applicable
-------------------------------	--	----------------

<b>Test Method</b>	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

**QA Ref**  
SLR 2,9  
Rev 88  
Aug 11

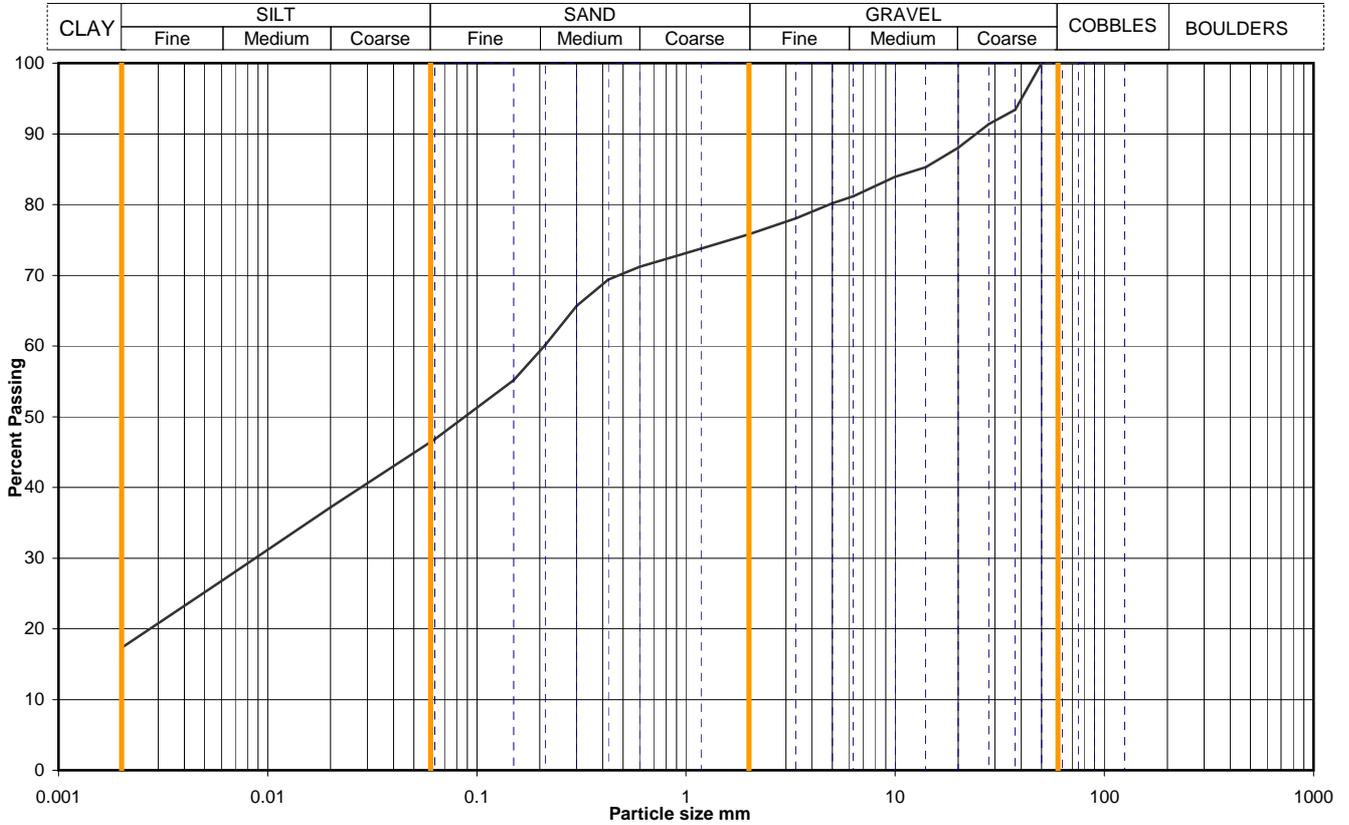


Printed:18/11/2011 17:31

**Figure**  
**PSD 32**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	12	Type	B
			ID	ESGA1077-1120111013000000260		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	37
90	100	0.0060	27
75	100	0.0020	17
63	100		
50	100		
37.5	93		
28	91		
20	88		
14	85		
10	84		
6.3	81		
5.0	80		
3.35	78		
2.00	76		
1.18	74		
0.600	71		
0.425	69		
0.300	66		
0.212	60		
0.150	55		
0.063	47		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 4.4
---	-------------------------------

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions <small>*&lt;60mm values to aid description only</small>	Cobbles / boulders	Whole	*<60mm
	Gravel	0	0
	Sand	24	24
	Silt	29	29
	Clay	18	18

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

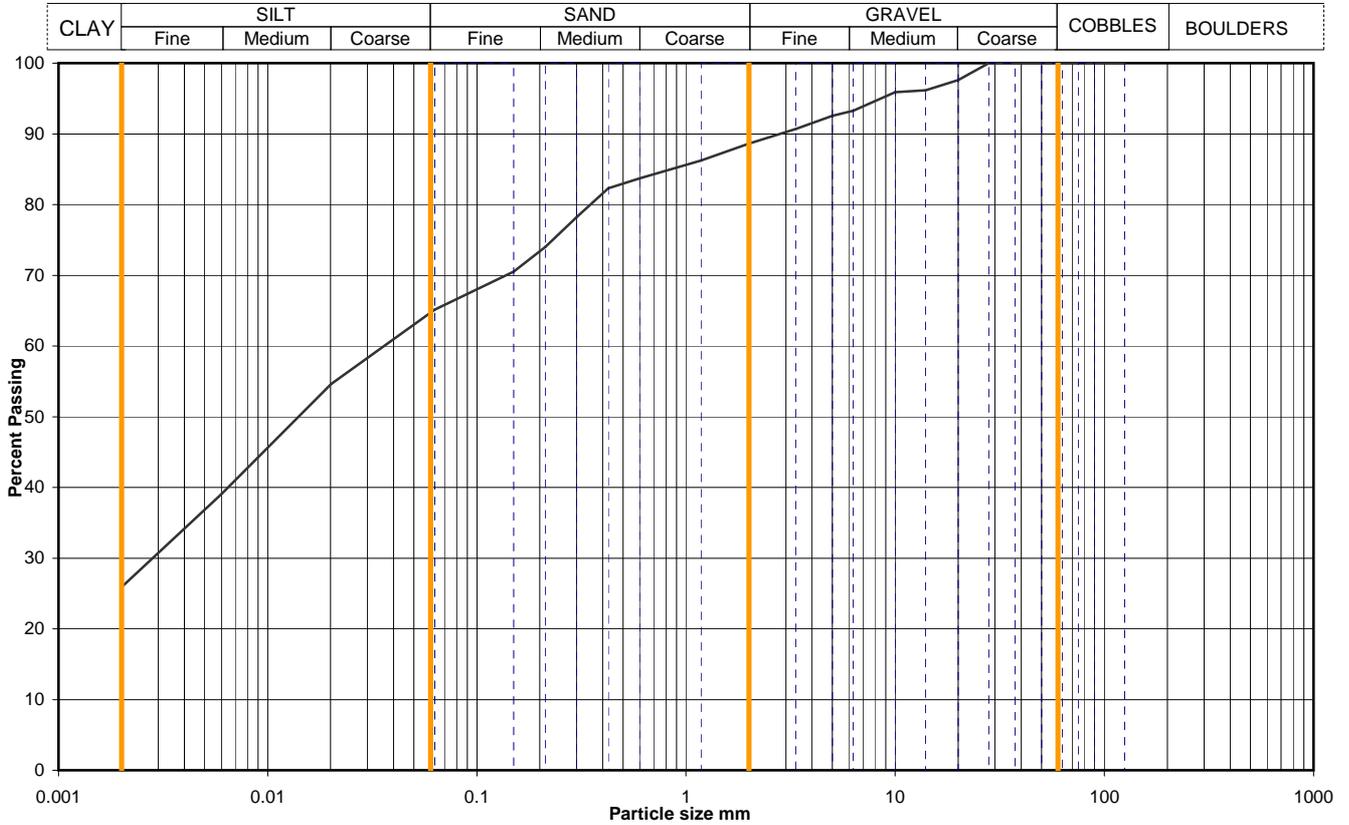


Printed: 18/11/2011 17:31

Figure  
**PSD 33**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH5
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.50
			Samp No	16
			Type	B
			ID	ESGA1077-1120111013000000264
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	55
90	100	0.0060	39
75	100	0.0020	26
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	96		
10	96		
6.3	93		
5.0	93		
3.35	91		
2.00	89		
1.18	86		
0.600	84		
0.425	82		
0.300	78		
0.212	74		
0.150	71		
0.063	65		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 3.6
---	-------------------------------

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		11	11
		24	24
		39	39
*<60mm values to aid description only		26	26

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

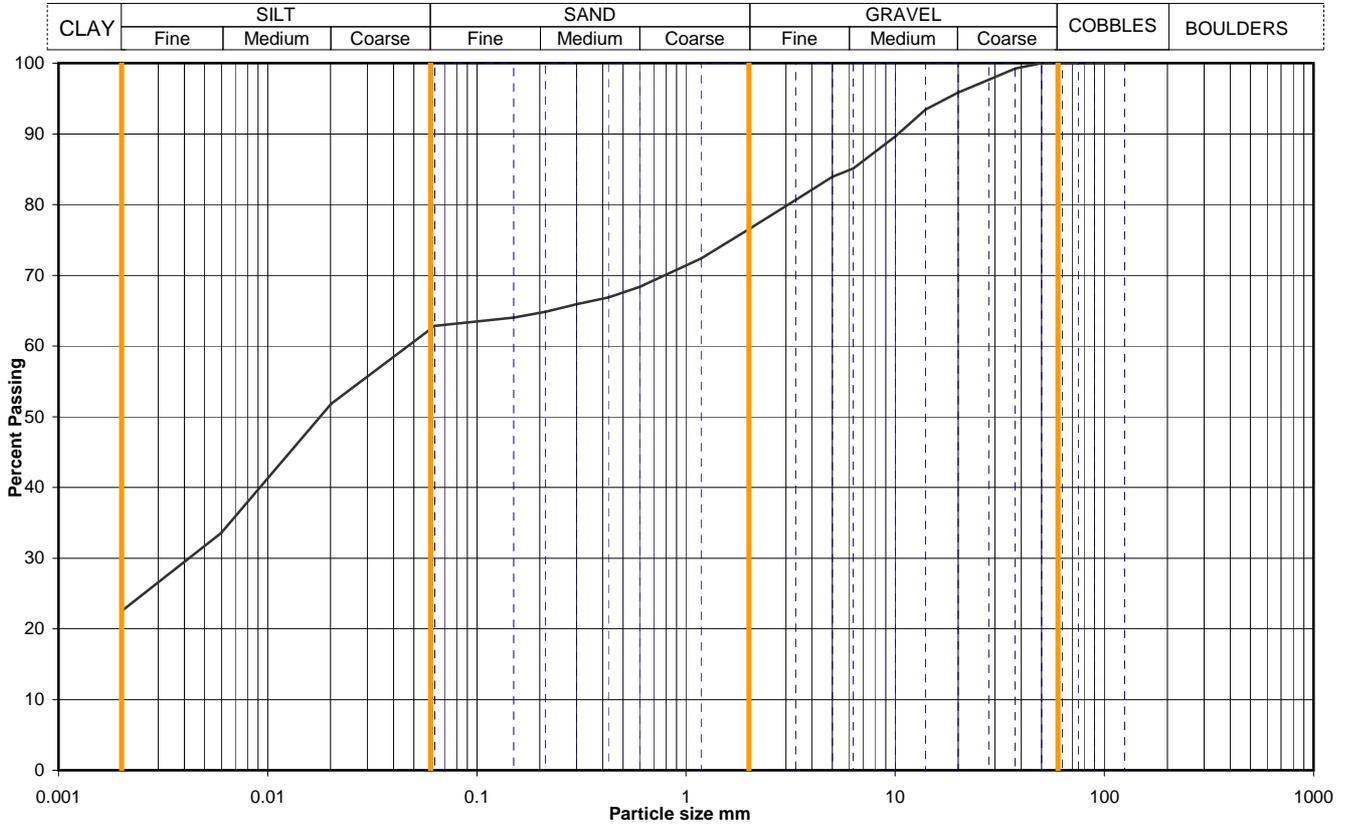


Printed: 18/11/2011 17:31

Figure  
**PSD 34**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	19	Type	B
			ID	ESGA1077-11201110130000000267		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	52
90	100	0.0060	34
75	100	0.0020	22
63	100		
50	100		
37.5	99		
28	98		
20	96		
14	93		
10	90		
6.3	85		
5.0	84		
3.35	81		
2.00	77		
1.18	72		
0.600	68	Particle density, Mg/m <sup>3</sup>	
0.425	67	2.65 assumed	
0.300	66	Dry mass of sample, kg	
0.212	65	12.1	
0.150	64		
0.063	63		

Soil description	Brownish grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		23	23
		14	14
		40	40
*<60mm values to aid description only		23	23

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

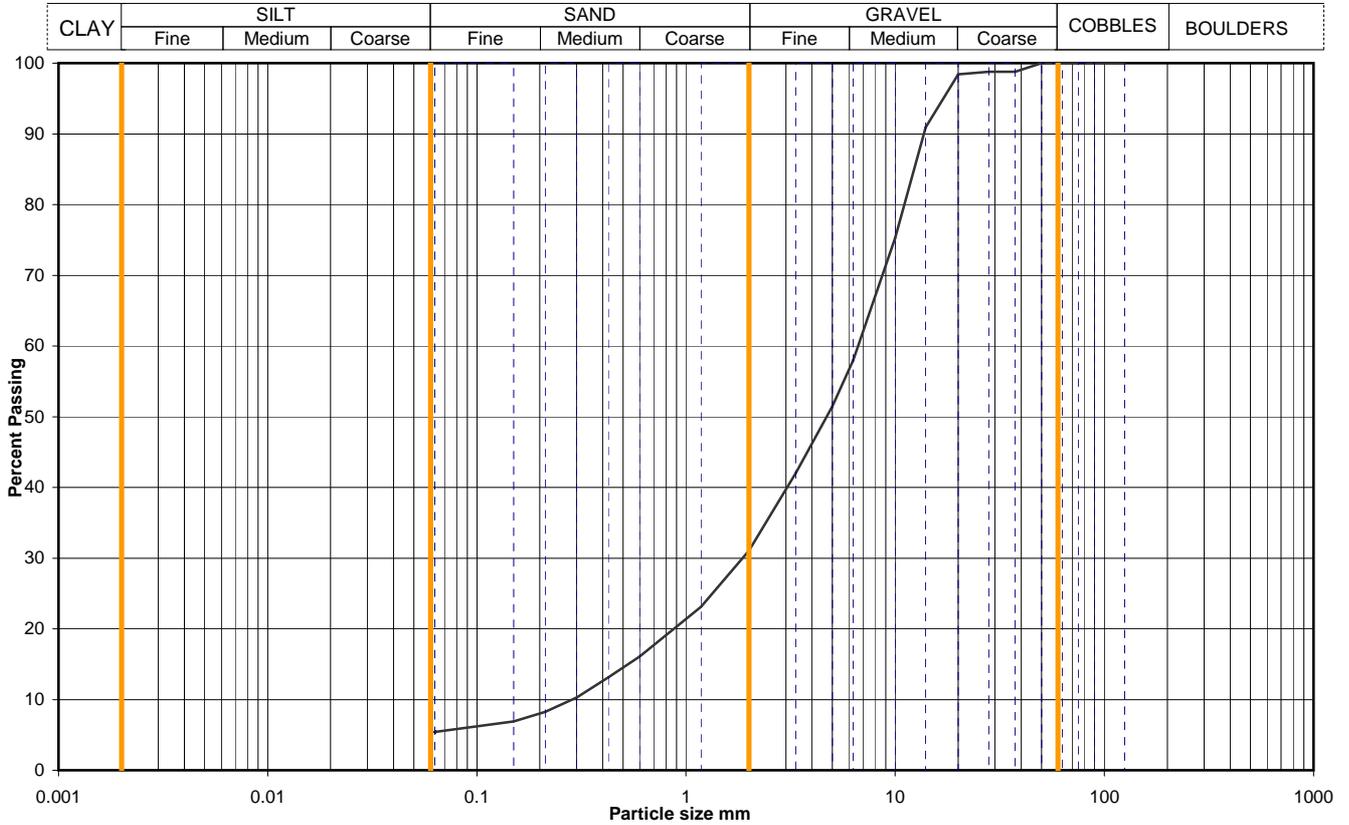


Printed: 18/11/2011 17:31

Figure  
**PSD 35**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.10		
			Samp No	2	Type	B
			ID	ESGA1077-11201110110000000207		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	99		
28	99		
20	98		
14	91		
10	75		
6.3	58		
5.0	51		
3.35	42		
2.00	31		
1.18	23		
0.600	16		
0.425	13		
0.300	10		
0.212	8		
0.150	7		
0.063	5		
		Dry mass of sample, kg	
		15.4	

Soil description	Dark grey very sandy GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	* <60mm
		0	0
		69	69
		26	26
		silt+clay =	5

Uniformity Coefficient	$D_{60} / D_{10}$	23
------------------------	-------------------	----

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

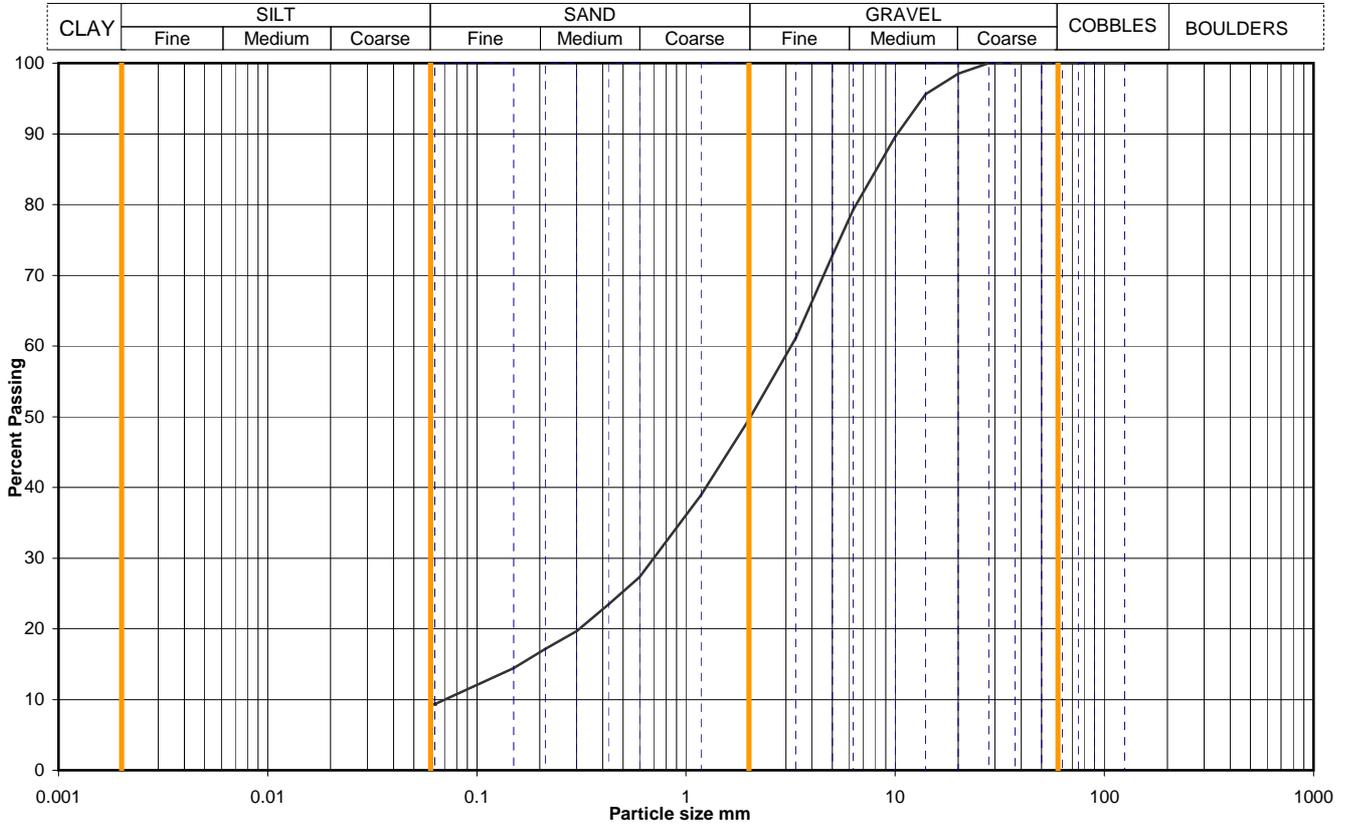


Printed: 18/11/2011 17:31

Figure  
**PSD 36**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.40		
			Samp No	4	Type	B
			ID	ESGA1077-11201110110000000209		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	96		
10	90		
6.3	79		
5.0	73		
3.35	61		
2.00	50		
1.18	39		
0.600	27		
0.425	23		
0.300	20		
0.212	17		
0.150	14		
0.063	9		
		Dry mass of sample, kg	
		13.2	

Soil description	Grey silty very sandy GRAVEL.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		50	50
		40	40
		silt+clay =	10
*<60mm values to aid description only			

<b>Uniformity Coefficient</b>	<b>D<sub>60</sub> / D<sub>10</sub></b>	45
-------------------------------	--	----

<b>Test Method</b>	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

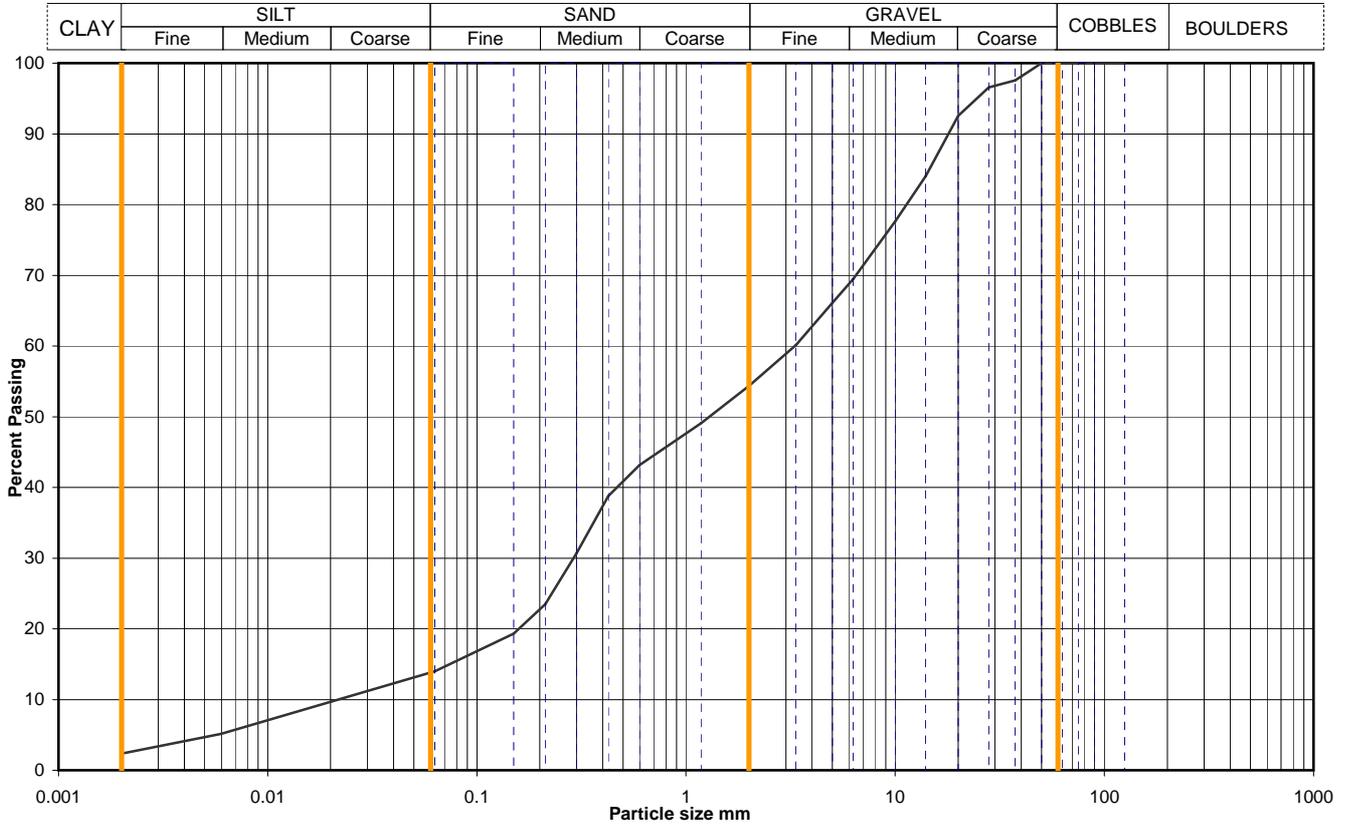


Printed: 18/11/2011 17:31

Figure  
**PSD 37**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.80		
			Samp No	6	Type	B
			ID	ESGA1077-11201110110000000211		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	10
90	100	0.0060	5
75	100	0.0020	2
63	100		
50	100		
37.5	98		
28	97		
20	93		
14	84		
10	78		
6.3	69		
5.0	66		
3.35	60		
2.00	54		
1.18	49		
0.600	43		
0.425	39		
0.300	31		
0.212	23		
0.150	19		
0.063	14		

Particle density, Mg/m <sup>3</sup>	
2.65 assumed	
Dry mass of sample, kg	
5.1	

Soil description	Dark grey sandy gravelly SILT.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		46	46
		41	41
		11	11
2	2	2	2

<b>Uniformity Coefficient</b>	<b>D<sub>60</sub> / D<sub>10</sub></b>	152
-------------------------------	--	-----

<b>Test Method</b>	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

**QA Ref**  
SLR 2,9  
Rev 88  
Aug 11

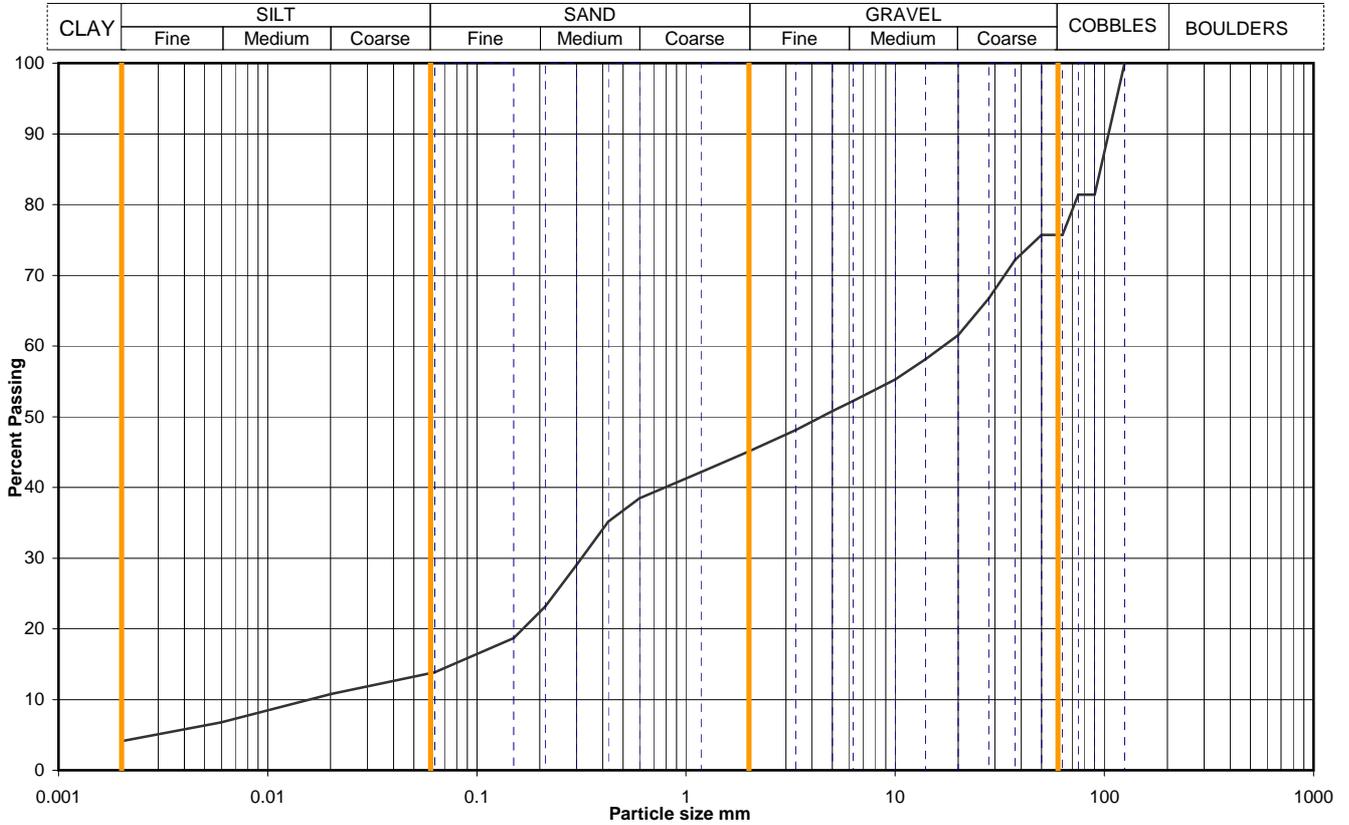


Printed: 18/11/2011 17:31

**Figure**  
**PSD 38**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.20		
			Samp No	9	Type	B
			ID	ESGA1077-11201110110000000215		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	11
90	81	0.0060	7
75	81	0.0020	4
63	76		
50	76		
37.5	72		
28	67		
20	62		
14	58		
10	55		
6.3	52		
5.0	51		
3.35	48		
2.00	45		
1.18	42		
0.600	38		
0.425	35		
0.300	29		
0.212	23		
0.150	19		
0.063	14		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 7.5
---	-------------------------------

Soil description	Brown slightly sandy slightly gravelly CLAY with 2 cobbles.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		24	0
		31	41
		31	41
		10	13
		4	5

Uniformity Coefficient	$D_{60} / D_{10}$	1068
------------------------	-------------------	------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

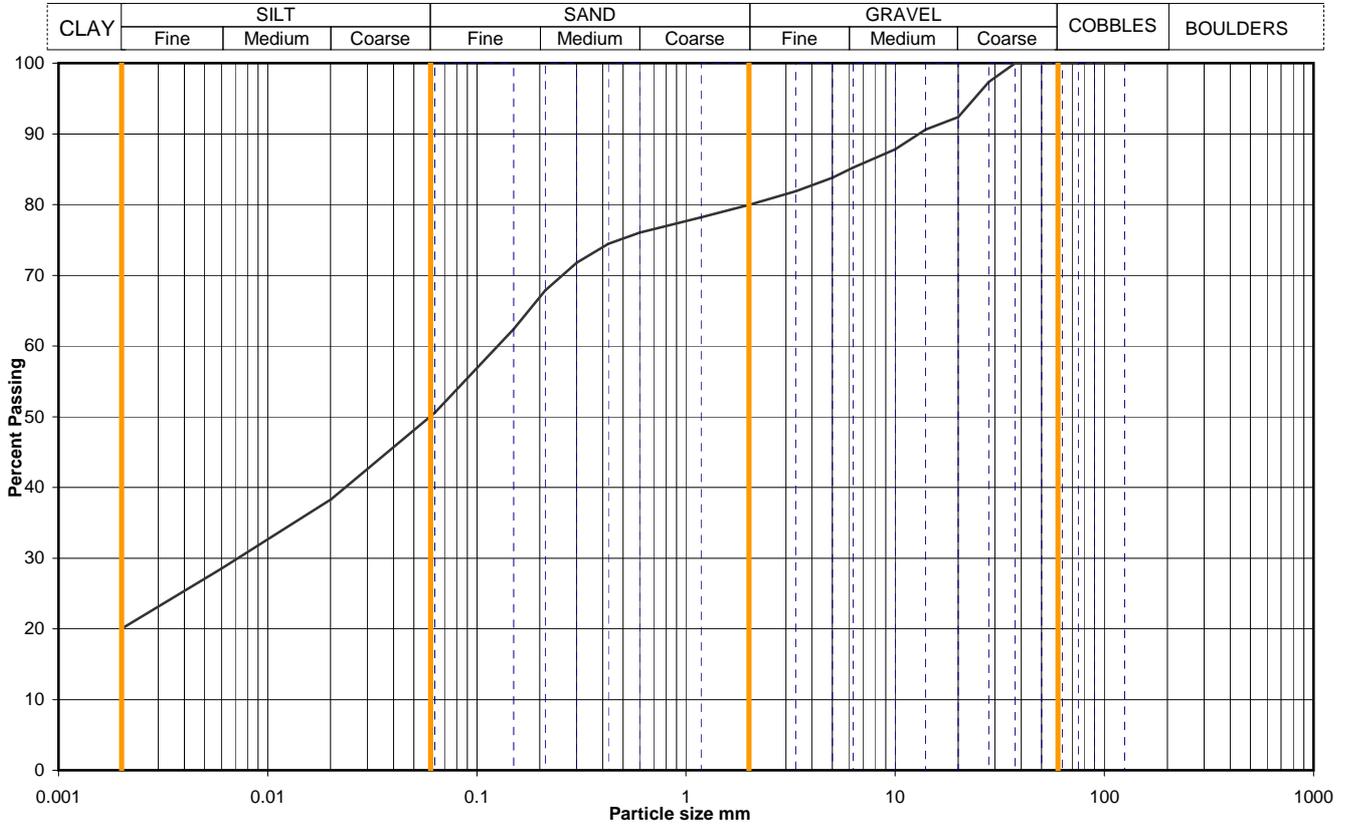


Printed: 18/11/2011 17:31

Figure  
**PSD 39**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	12
			Type	B
			ID	ESGA1077-11201110110000000219
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	38
90	100	0.0060	29
75	100	0.0020	20
63	100		
50	100		
37.5	100		
28	97		
20	92		
14	91		
10	88		
6.3	85		
5.0	84		
3.35	82		
2.00	80		
1.18	78		
0.600	76	Particle density, Mg/m <sup>3</sup>	
0.425	74	2.65 assumed	
0.300	72	Dry mass of sample, kg	
0.212	68	4.2	
0.150	62		
0.063	51		

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		20	20
		30	30
		30	30
*<60mm values to aid description only		20	20

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

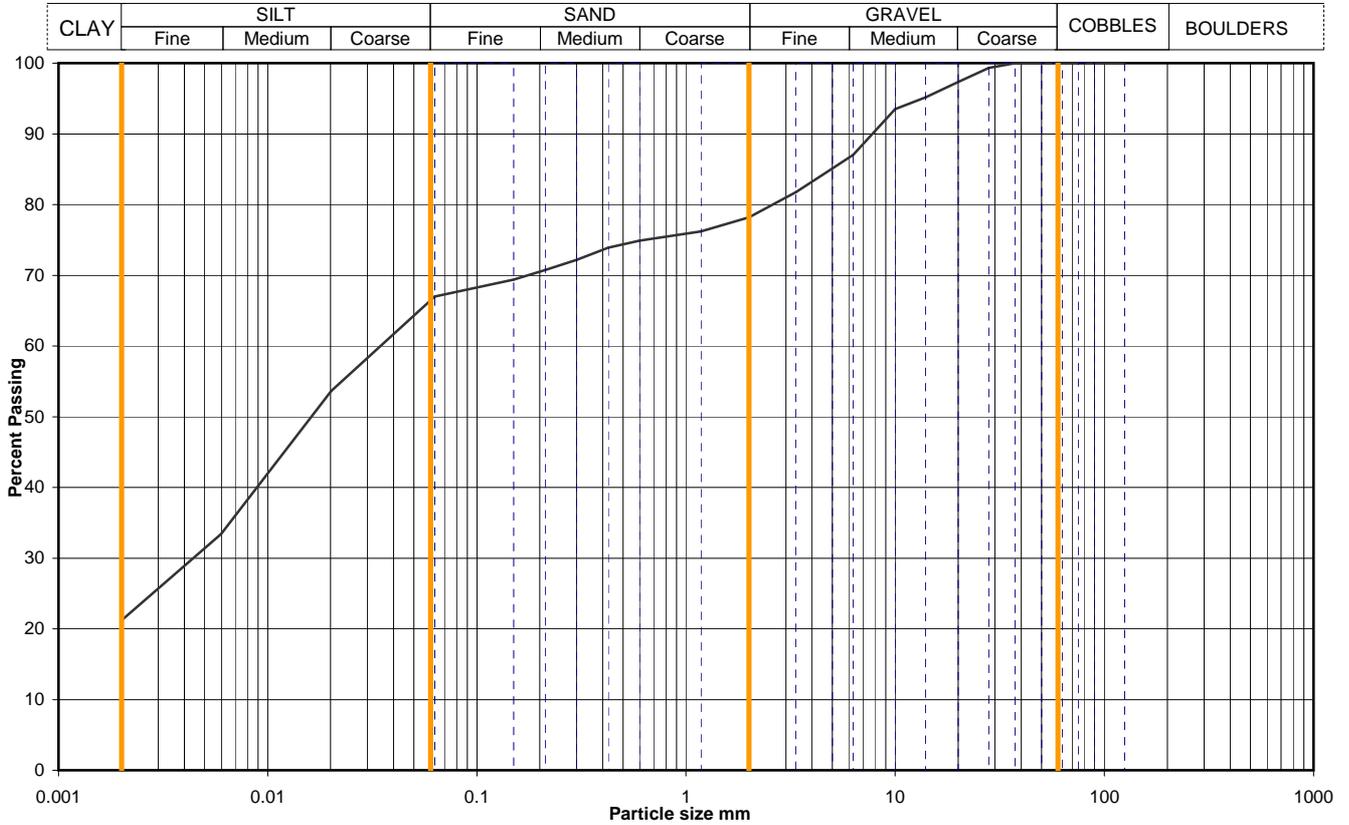


Printed:18/11/2011 17:31

Figure  
**PSD 40**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.50
			Samp No	16
			Type	B
			ID	ESGA1077-11201110110000000223
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	54
90	100	0.0060	33
75	100	0.0020	21
63	100		
50	100		
37.5	100		
28	99		
20	97		
14	95		
10	93		
6.3	87		
5.0	85		
3.35	82		
2.00	78		
1.18	76		
0.600	75		
0.425	74		
0.300	72		
0.212	71		
0.150	69		
0.063	67		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 5.5
---	-------------------------------

Soil description	Grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		22	22
		12	12
		45	45
*<60mm values to aid description only		21	21

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

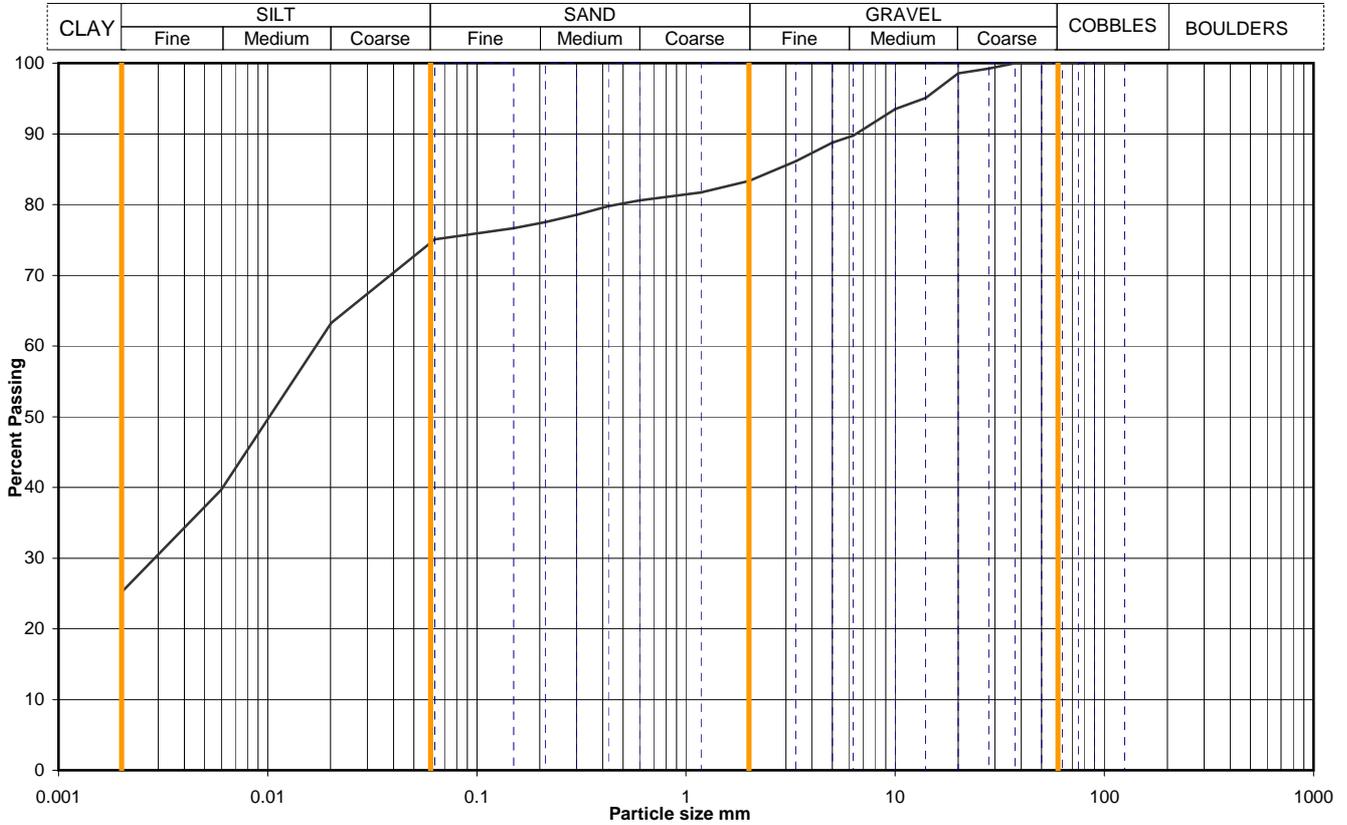


Printed: 18/11/2011 17:31

Figure  
**PSD 41**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	19	Type	B
			ID	ESGA1077-11201110110000000226		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	63
90	100	0.0060	40
75	100	0.0020	25
63	100		
50	100		
37.5	100		
28	99		
20	99		
14	95		
10	94		
6.3	90		
5.0	89		
3.35	86		
2.00	83		
1.18	82		
0.600	81		
0.425	80		
0.300	79		
0.212	78		
0.150	77		
0.063	75		

Particle density, Mg/m <sup>3</sup> 2.65 assumed	Dry mass of sample, kg 12.1
---	--------------------------------

Soil description	Grey slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		17	17
		9	9
		49	49
*<60mm values to aid description only		25	25

<b>Uniformity Coefficient</b>	<b>D<sub>60</sub> / D<sub>10</sub></b>	Not applicable
-------------------------------	--	----------------

<b>Test Method</b>	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

**QA Ref**  
SLR 2,9  
Rev 88  
Aug 11

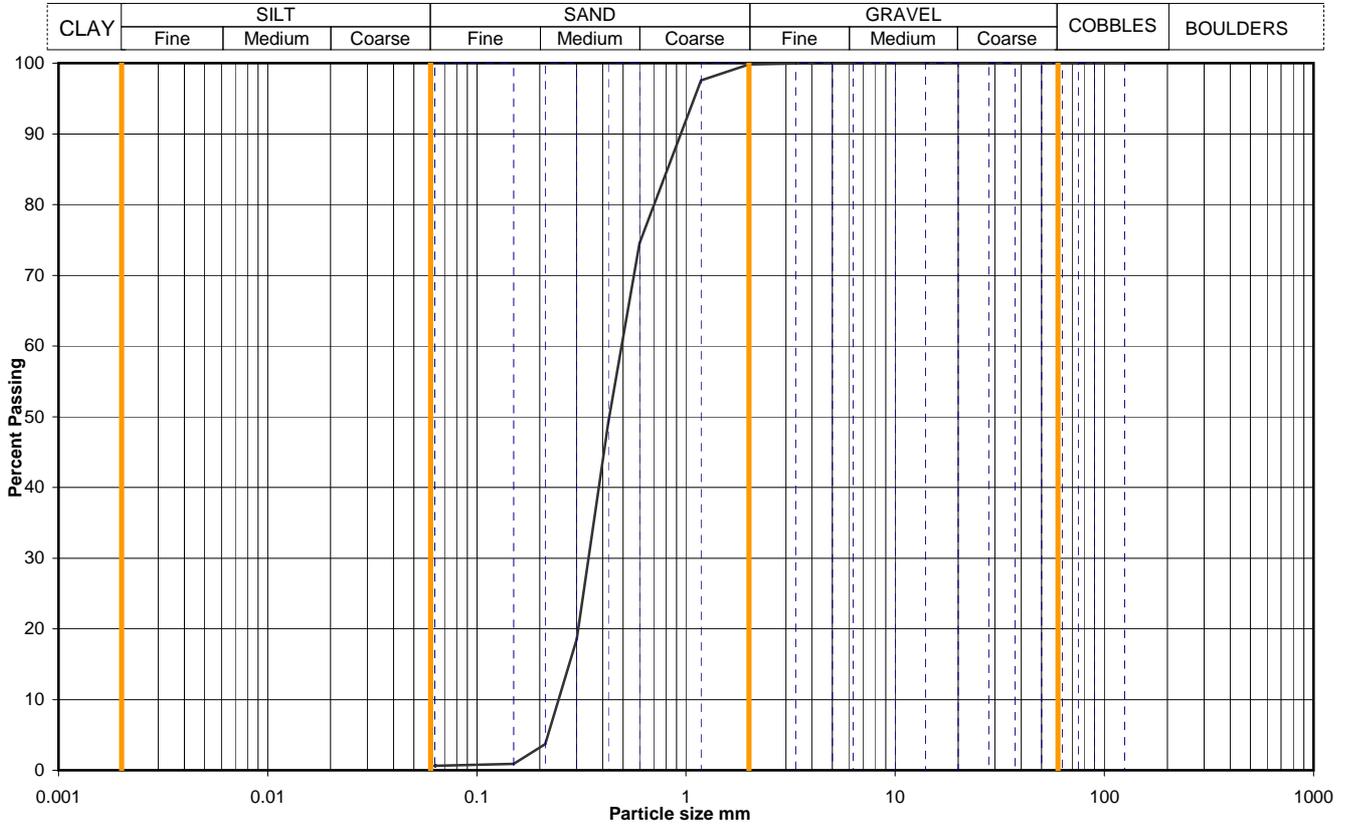


Printed: 18/11/2011 17:31

**Figure**  
**PSD 42**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.30		
			Samp No	2	Type	B
			ID	ESGA1077-1120111013000000229		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	100		
2.00	100		
1.18	98		
0.600	75		
0.425	49		
0.300	19		
0.212	4		
0.150	1		
0.063	1		
		Dry mass of sample, kg	
		3.0	

Soil description	Brown SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
<b>Sample Proportions</b> <small>*&lt;60mm values to aid description only</small>	Cobbles / boulders	Whole	*<60mm
	Gravel	0	0
	Sand	99	99
	Silt	silt+clay =	
	Clay	1	1

<b>Uniformity Coefficient</b>	$D_{60} / D_{10}$	2
-------------------------------	-------------------	---

<b>Test Method</b>	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

**QA Ref**  
SLR 2,9  
Rev 88  
Aug 11

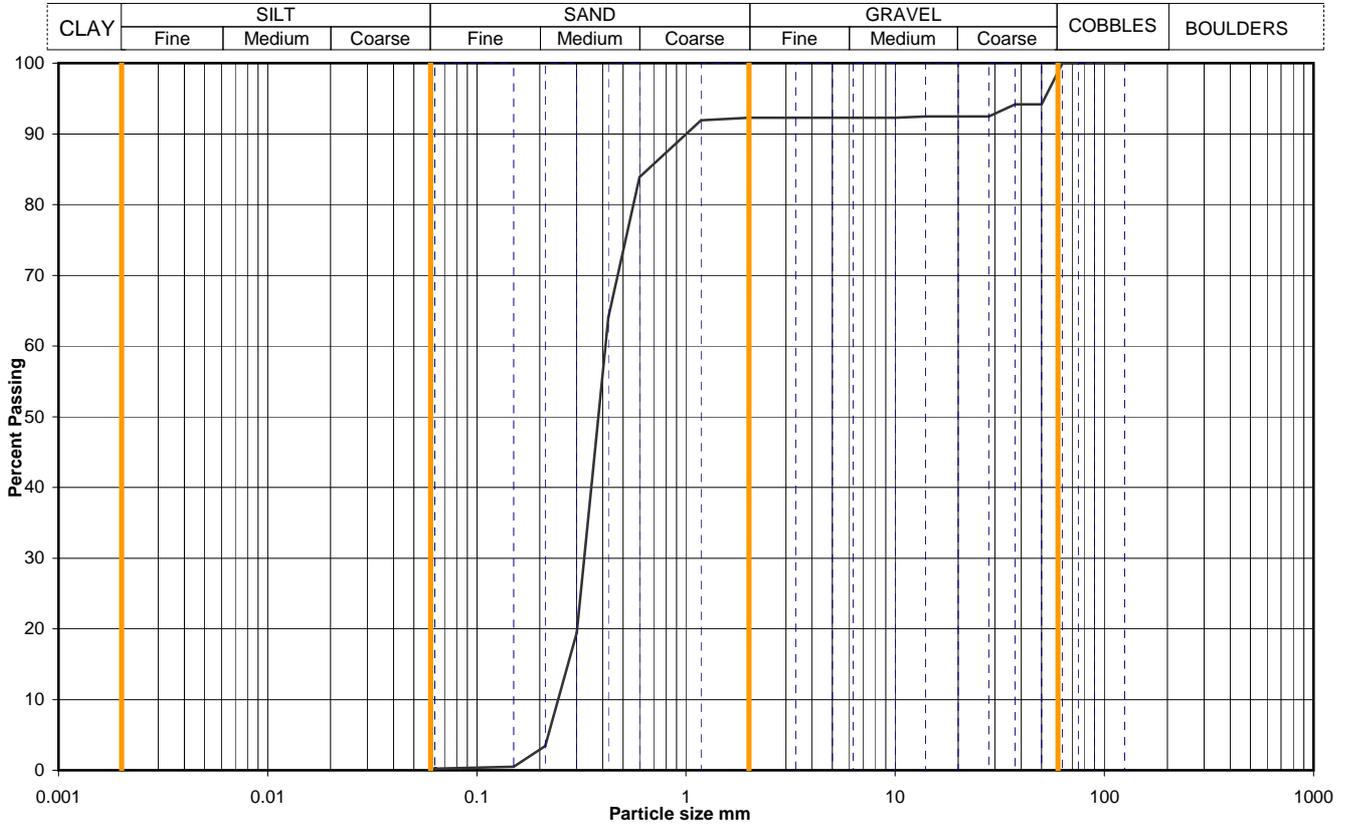


Printed: 18/11/2011 17:31

**Figure**  
**PSD 43**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.30		
			Samp No	2	Type	B
			ID	ESGA1077-11201110130000000270		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	94		
37.5	94		
28	92		
20	92		
14	92		
10	92		
6.3	92		
5.0	92		
3.35	92		
2.00	92		
1.18	92		
0.600	84		
0.425	64		
0.300	19		
0.212	3		
0.150	0		
0.063	0		

Dry mass of sample, kg	4.8
------------------------	-----

Soil description	Brownish grey gravelly SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		1	0
		7	7
		92	93
		silt+clay =	0

Uniformity Coefficient	$D_{60} / D_{10}$	2
------------------------	-------------------	---

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

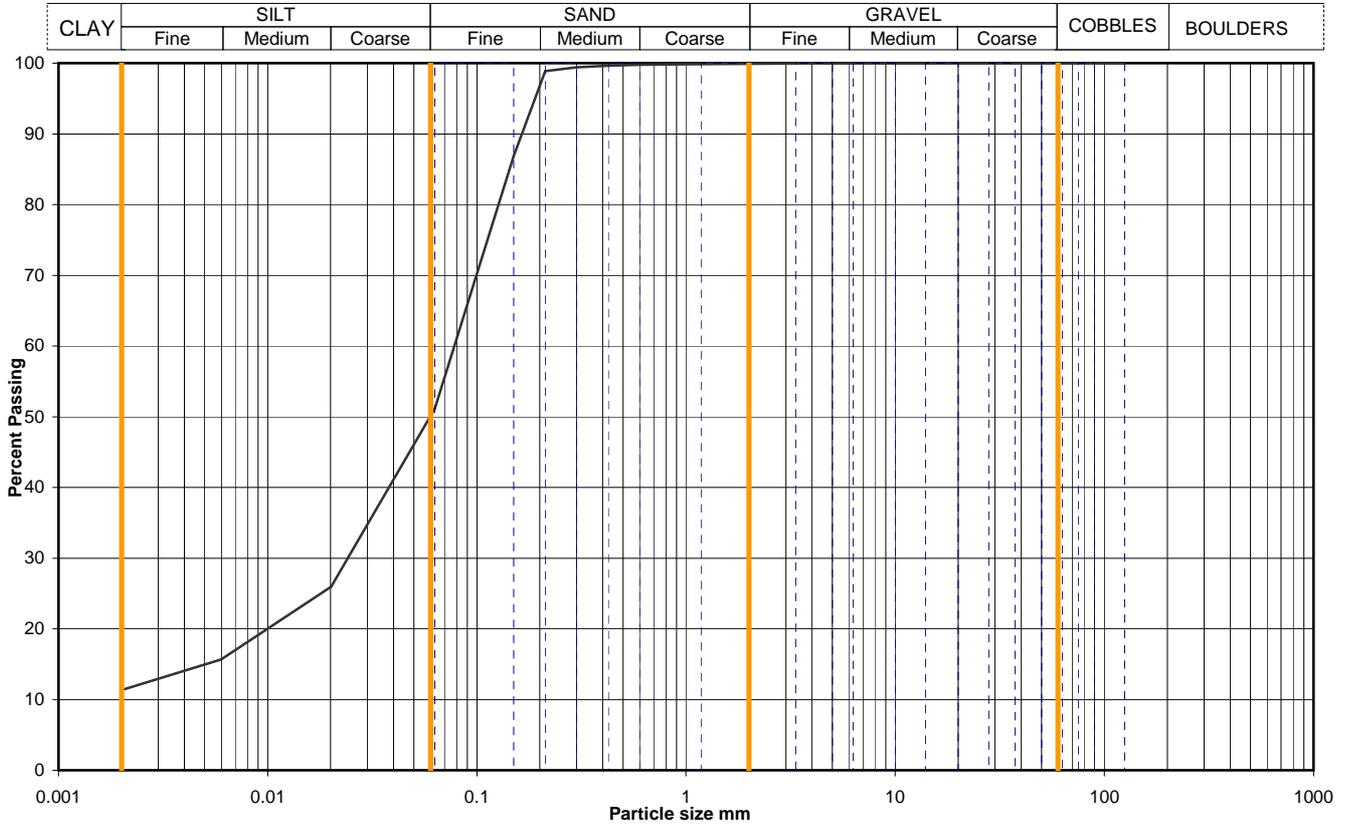


Printed: 18/11/2011 17:31

Figure  
**PSD 44**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.80		
			Samp No	7	Type	D
			ID	ESGA1077-11201110130000000275		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	26
90	100	0.0060	16
75	100	0.0020	11
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	100		
5.0	100		
3.35	100		
2.00	100		
1.18	100		
0.600	100		
0.425	100		
0.300	99		
0.212	99		
0.150	87		
0.063	51		

Particle density, Mg/m <sup>3</sup>	
2.65	assumed
Dry mass of sample, kg	
1.1	

Soil description	Grey sandy clayey SILT.		
Preparation / Pretreatment	Sieve: natural material    Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		0	0
		50	50
		39	39
*<60mm values to aid description only		11	11

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

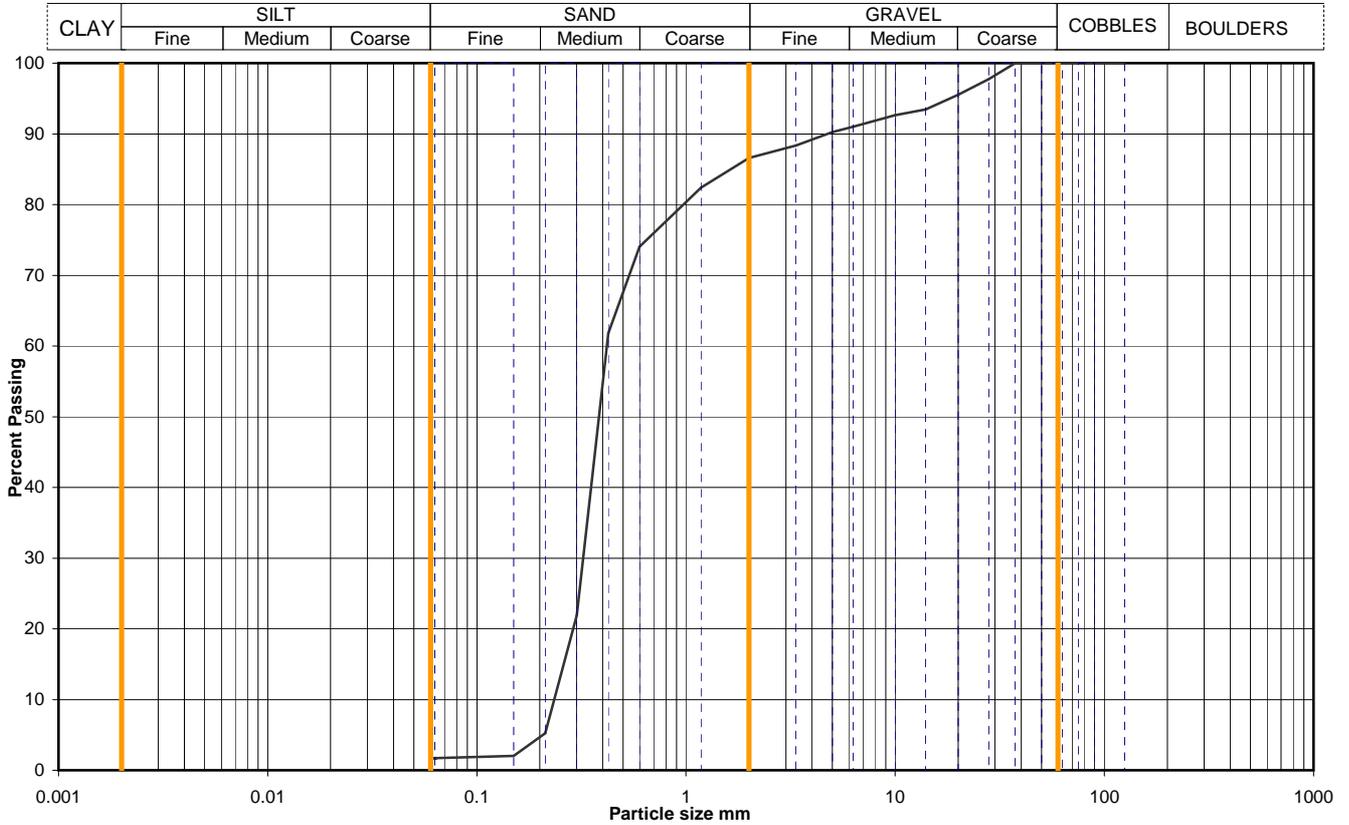


Printed: 18/11/2011 17:31

Figure  
**PSD 45**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.30
			Samp No	2
			Type	B
			ID	ESGA1077-1120111013000000232
			Spec Ref	



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	98		
20	96		
14	93		
10	93		
6.3	91		
5.0	90		
3.35	88		
2.00	87		
1.18	82		
0.600	74		
0.425	62		
0.300	22		
0.212	5		
0.150	2		
0.063	2		
		Dry mass of sample, kg	
		6.7	

Soil description	Brown gravelly SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions <small>*&lt;math&gt; &lt; 60\text{mm}&lt;/math&gt; values to aid description only</small>	Cobbles / boulders	Whole	*<math> < 60\text{mm}</math>
	Gravel	0	0
	Sand	13	13
	Silt	85	85
	Clay	silt+clay =	
		2	2

Uniformity Coefficient	$D_{60} / D_{10}$	2
------------------------	-------------------	---

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

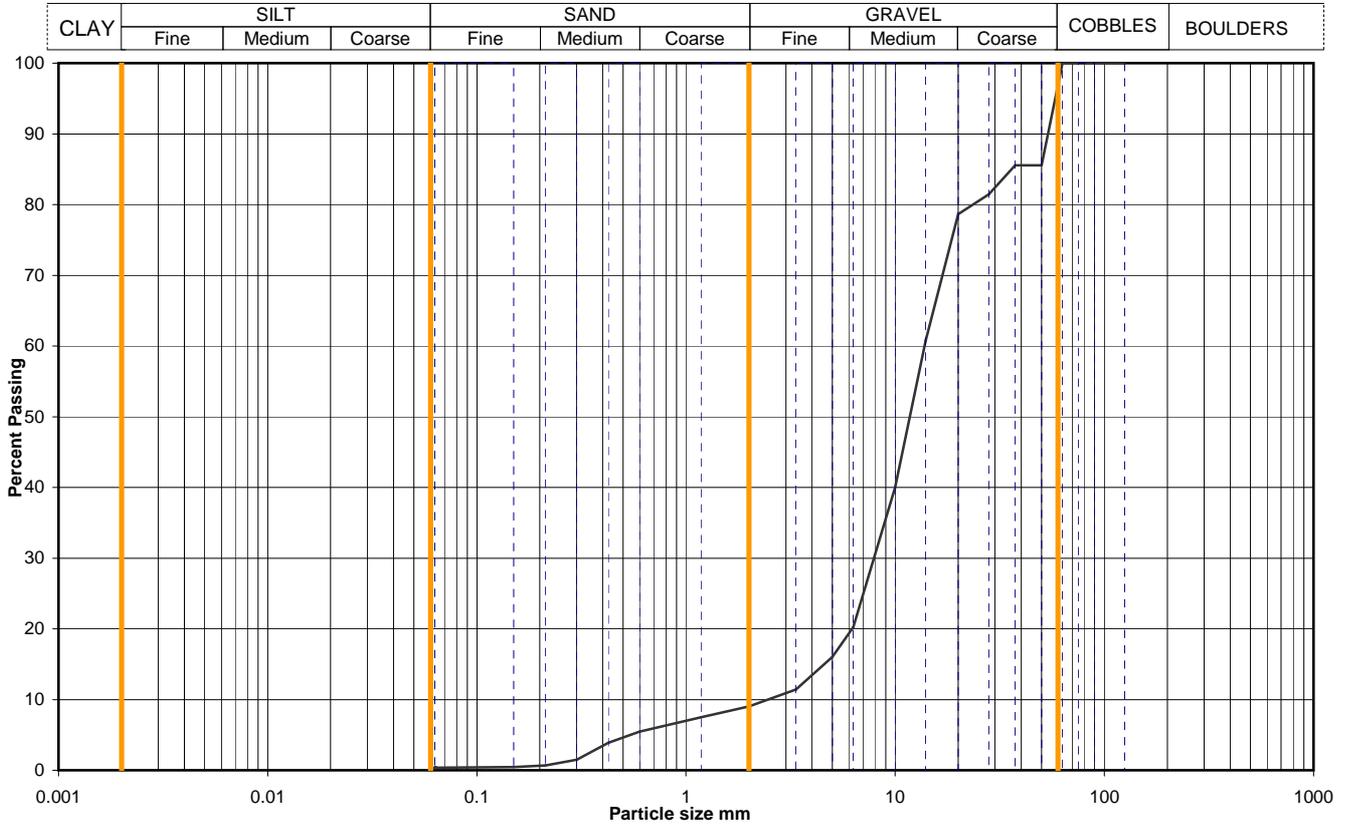


Printed: 18/11/2011 17:31

Figure  
**PSD 46**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS3	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.80	
		Samp No	4	Type	D
		ID	ESGA1077-1120111013000000234		
		Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	86		
37.5	86		
28	81		
20	79		
14	61		
10	40		
6.3	20		
5.0	16		
3.35	11		
2.00	9		
1.18	8		
0.600	5		
0.425	4		
0.300	1		
0.212	1		
0.150	0		
0.063	0		
		Dry mass of sample, kg	
		1.8	

Soil description	Brownish grey sandy GRAVEL.		
Preparation / Pretreatment	Sieve: pre dried,		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<math><60\text{mm}</math>
		3	0
		88	91
		9	9
		silt+clay =	
*<math><60\text{mm}</math> values to aid description only		0	0

Uniformity Coefficient	$D_{60} / D_{10}$	6
------------------------	-------------------	---

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

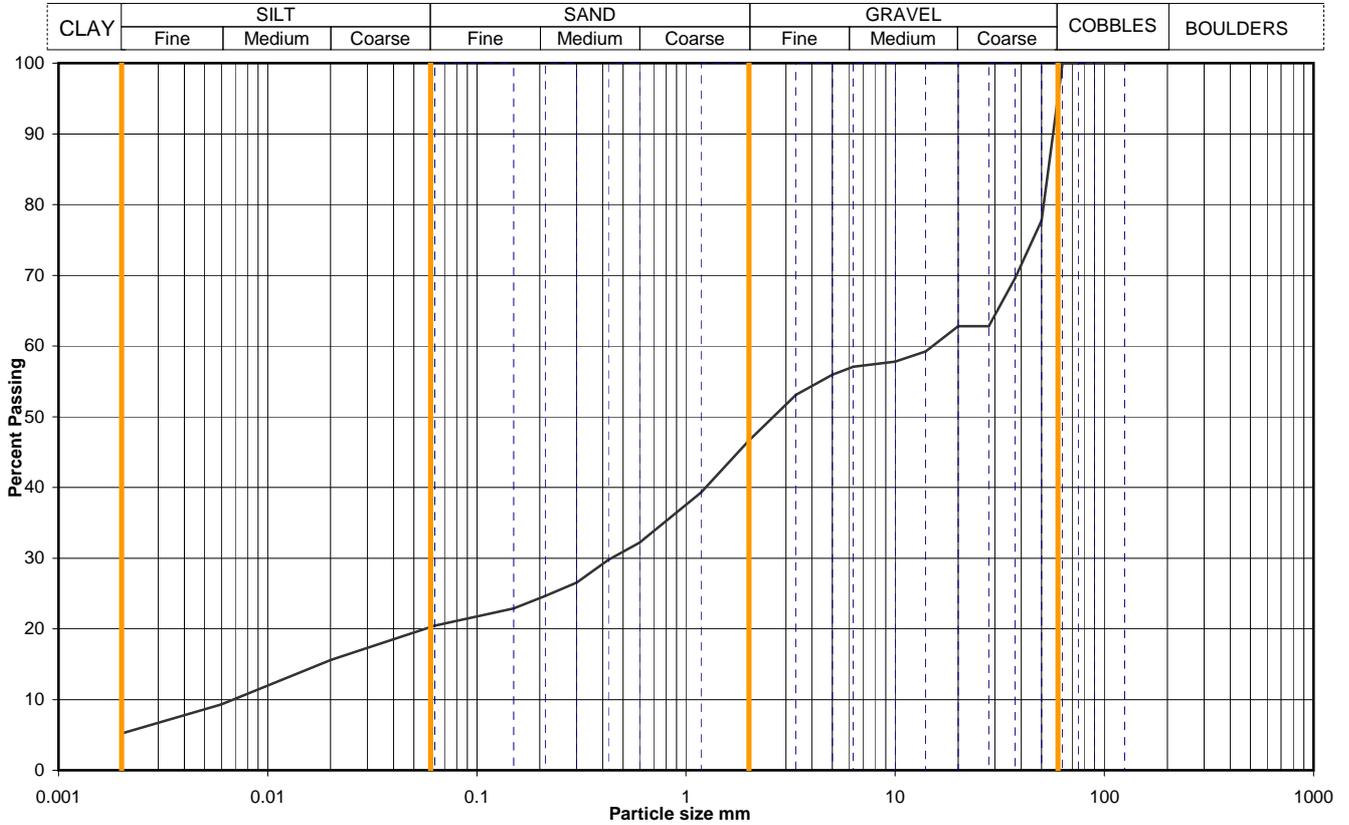


Printed:24/11/2011 10:01

Figure  
**PSD 47**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.50		
			Samp No	5	Type	D
			ID	ESGA1077-1120111013000000235		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	16
90	100	0.0060	9
75	100	0.0020	5
63	100		
50	78		
37.5	70		
28	63		
20	63		
14	59		
10	58		
6.3	57		
5.0	56		
3.35	53		
2.00	47		
1.18	39		
0.600	32		
0.425	30		
0.300	27		
0.212	25		
0.150	23		
0.063	20		

Particle density, Mg/m <sup>3</sup>	
2.65	assumed
Dry mass of sample, kg	
1.0	

Soil description	Brownish grey slightly sandy gravelly CLAY.		
Preparation / Pretreatment	Sieve: pre dried, Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		5	0
		48	51
		26	27
		15	16
*<60mm values to aid description only		6	6

Uniformity Coefficient	$D_{60} / D_{10}$	2203
------------------------	-------------------	------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

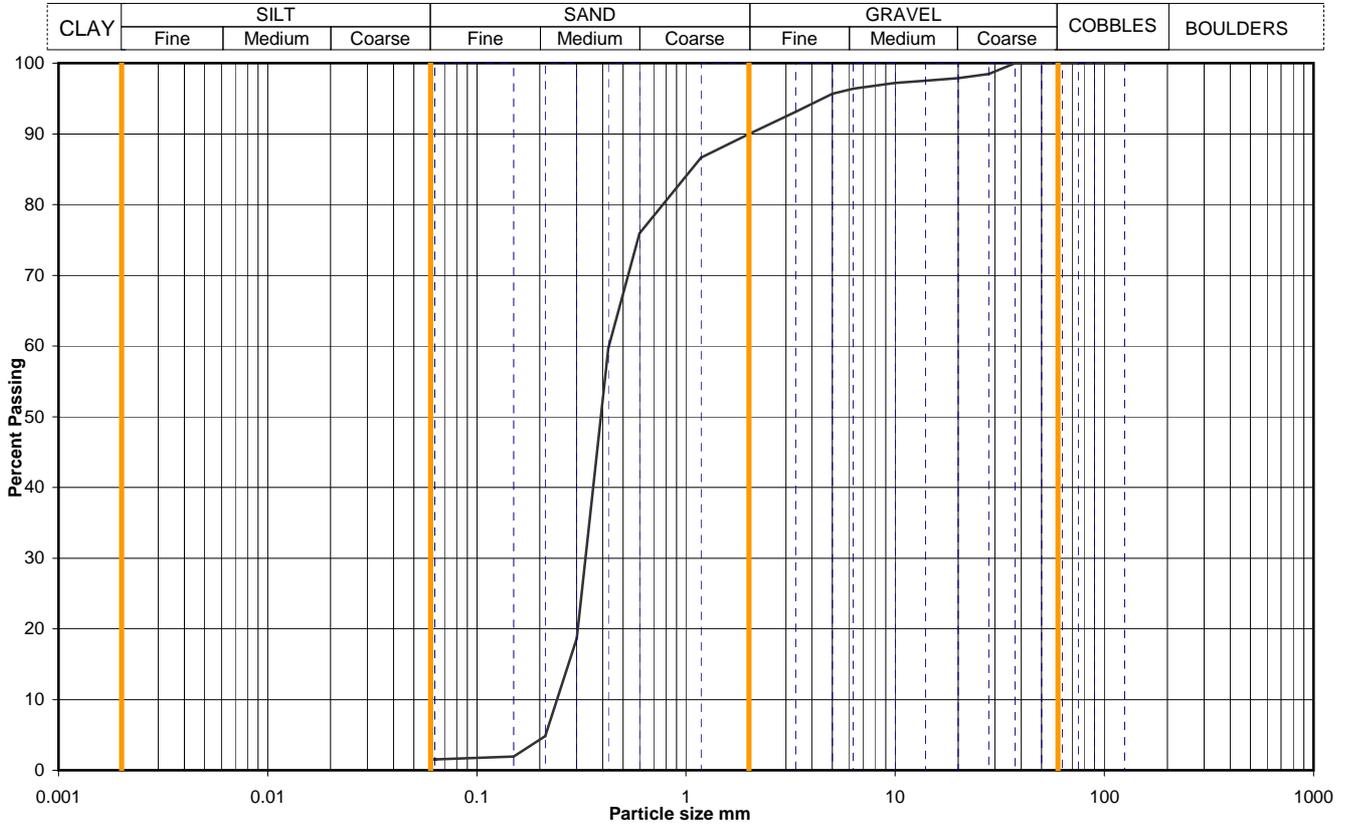


Printed: 18/11/2011 17:31

Figure  
**PSD 48**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.60		
			Samp No	3	Type	B
			ID	ESGA1077-1120111013000000241		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	98		
20	98		
14	97		
10	97		
6.3	96		
5.0	96		
3.35	93		
2.00	90		
1.18	87		
0.600	76		
0.425	60		
0.300	19		
0.212	5		
0.150	2		
0.063	2		
		Dry mass of sample, kg	
		3.7	

Soil description	Brown gravelly SAND.		
Preparation / Pretreatment	Sieve: natural material		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		10	10
		88	88
		silt+clay =	2
*<60mm values to aid description only			

Uniformity Coefficient	$D_{60} / D_{10}$	2
------------------------	-------------------	---

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	none

QA Ref  
SLR 2,9  
Rev 88  
Aug 11

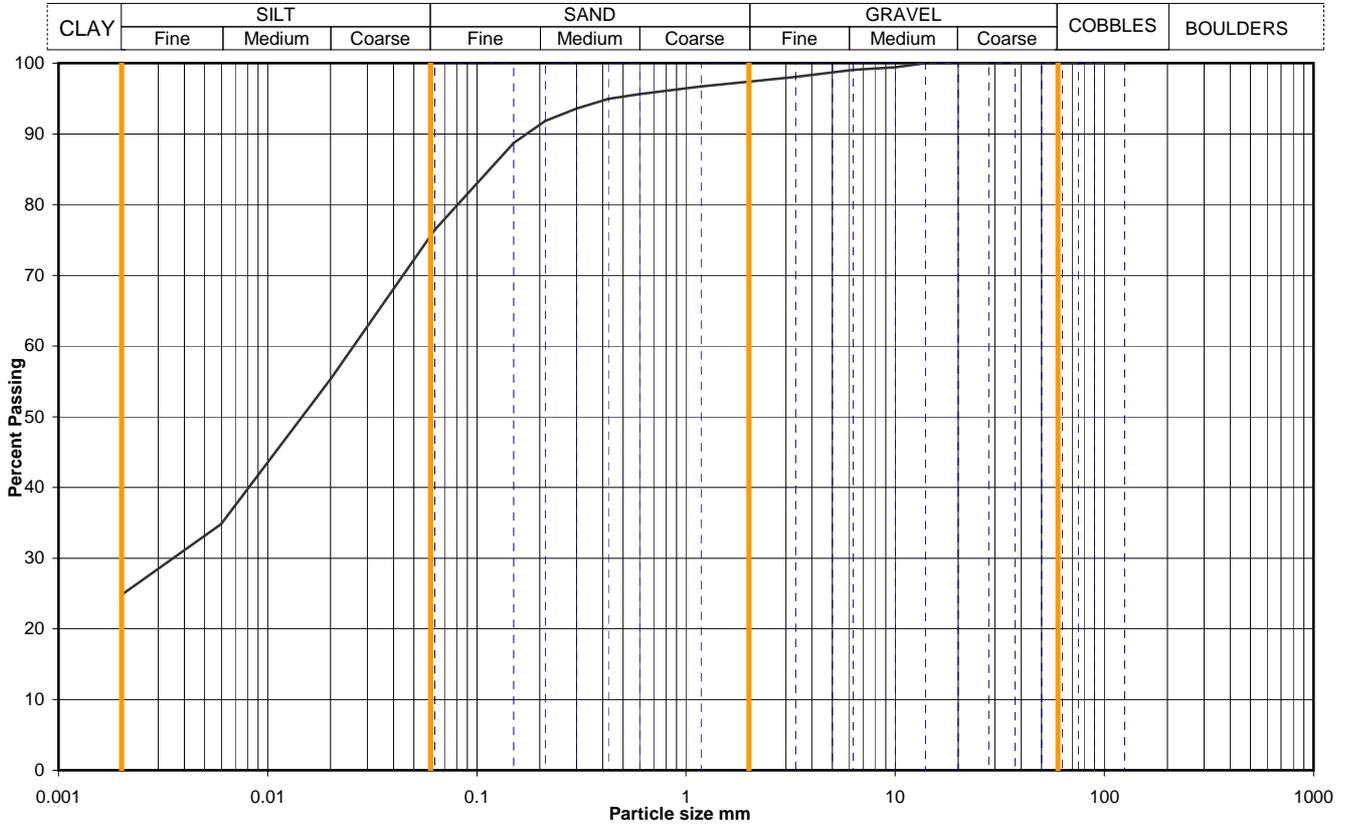


Printed: 18/11/2011 17:31

Figure  
**PSD 49**

# Particle Size Distribution Analysis

Project No	A1077-11	Sample Details:	Hole No	WS5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00		
			Samp No	6	Type	D
			ID	ESGA1077-11201110130000000245		
			Spec Ref			



Sieving		Sedimentation	
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.0201	55
90	100	0.0060	35
75	100	0.0020	25
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	99		
6.3	99		
5.0	99		
3.35	98		
2.00	97		
1.18	97		
0.600	96		
0.425	95		
0.300	94		
0.212	92		
0.150	89		
0.063	76		
		Particle density, Mg/m <sup>3</sup>	
		2.65 assumed	
		Dry mass of sample, kg	
		1.0	

Soil description	Brown slightly sandy slightly gravelly CLAY.		
Preparation / Pretreatment	Sieve: natural material Pipette: as BS1377		
Remarks			
Sample Proportions	Cobbles / boulders Gravel Sand Silt Clay	Whole	*<60mm
		0	0
		3	3
		22	22
		50	50
*<60mm values to aid description only		25	25

Uniformity Coefficient	$D_{60} / D_{10}$	Not applicable
------------------------	-------------------	----------------

Test Method	BS 1377 : Part 2 : 1990	
	Sieving	9.2 wet sieve
	Sedimentation	9.4 pipette

QA Ref  
SLR 2,9  
Rev 88  
Aug 11



Printed: 18/11/2011 17:31

Figure  
**PSD 50**

**UNCONSOLIDATED UNDRAINED TRIAXIAL COMPRESSION TESTS WITHOUT MEASUREMENT OF PORE PRESSURE - SUMMARY OF RESULTS**

Project No	Project Name															
A1077-11	SANDSEND BOREHOLES, NORTH YORKSHIRE															
Hole No.	Sample				Soil Description	Density		w	Test type	Dia.	$\sigma_3$	At failure / end of stage				Remarks
	No.	Depth (m)		type		bulk	dry					Axial strain	$\sigma_1 - \sigma_3$	$C_u$	M O D E	
from		to	Mg/m <sup>3</sup>		%	mm	kPa	%	kPa	kPa						
BH1	9	3.00	3.45	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.18	1.89	15	UUM	102.6 102.6 102.6	50 70 90	5.9 7.9 19.7	115 127 154	58 64 77	P	
BH1	25	9.50	9.95	U	Firm to stiff brown slightly sandy slightly gravelly CLAY.	2.24	1.95	15	UUM	101.1 101.1 101.1	140 200 250	6.4 8.8 19.2	109 123 148	54 62 74	P	
BH1	45	17.00	17.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.18	1.86	17	UUM	101.9 101.9 101.9	240 350 430	12.3 14.3 18.8	300 312 324	150 156 162	P	
BH1	56	21.00	21.45	U	Stiff to very stiff brown slightly sandy slightly gravelly CLAY.	2.20	1.94	13	UUM	102.6 102.6 102.6	300 430 530	19.6	385 0 0	193 0 0	P	20% axial strain achieved at 300kPa
BH2	8	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.16	1.90	14	UUM	102.4 102.4 102.4	50 70 90	10.4 14.3 19.7	350 378 397	175 189 199	P	
BH2	16	7.00	7.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.15	1.87	15	UUM	101.9 101.9 101.9	105 150 180	9.8 12.3 19.2	175 188 202	88 94 101	C	
BH2	34	16.00	16.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.11	1.74	21	UUM	102.7 102.7 102.7	230 330 410	7.9 9.9 12.4	148 160 169	74 80 84	C	
BH2	47	21.00	21.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.13	1.87	14	UUM	102.3 102.3 102.3	300 430 430	16.7 19.7	277 297	139 148	P	20% axial strain achieved at 430kPa
BH2	53	24.00	24.45	U	Stiff brown slightly sandy slightly gravelly CLAY.	2.16	1.90	14	UUM	101.6 101.6 101.6	340 490 600	16.8 19.7	366 385 0	183 193 0	P	20% axial strain achieved at 490kPa
BH3	8	2.50	2.95	U	Firm to very stiff brown slightly sandy slightly gravelly CLAY.	2.15	1.85	16	UUM	102.3 102.3 102.3	40 60 75	9.9 12.9 19.3	371 392 408	186 196 204	C	
BH3	16	5.50	5.95	U	Firm brown slightly sandy slightly gravelly CLAY.	2.17	1.86	17	UUM	103.2 103.2 103.2	85 120 150	4.9 6.4 19.8	83 92 124	41 46 62	P	
BH3	39	14.50	14.95	U	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.	2.21	1.95	14	UUM	103.1 103.1 103.1	150 185 205	19.7	270 0 0	135 0 0	P	20% axial strain achieved at 150kPa
BH3	51	19.00	19.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.	2.23	1.97	13	UUM	102.7 102.7 102.7	270 390 485	19.7	461 0 0	231 0 0	P	20% axial strain achieved at 270kPa

General notes: Tests carried out in accordance with BS1377: Part 7: 1990, clause 8 for single stage, clause 9 for multistage tests. Specimens nominally 2:1 height diameter ratio and tested at a rate of strain of 2%/minute, unless annotated otherwise. See individual test reports for further details.

Legend  
 UU - single stage test ( may be in sets of specimens )       $\sigma_3$       cell pressure      Mode of failure      P plastic  
 UUM - multistage test on a single specimen       $\sigma_1 - \sigma_3$       deviator stress      B brittle  
 suffix R - remoulded or recompactd       $C_u$       undrained shear strength      C compound

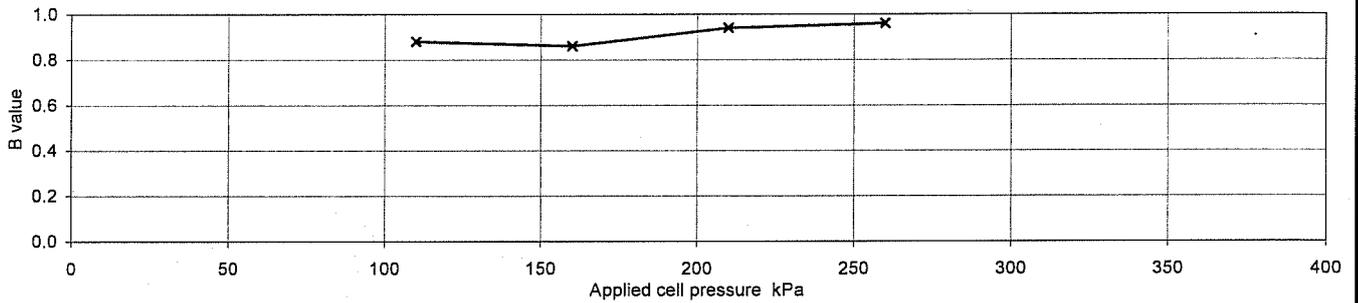
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00-5.45		
			No	13	Type	U
			ID			
			Spec Ref			

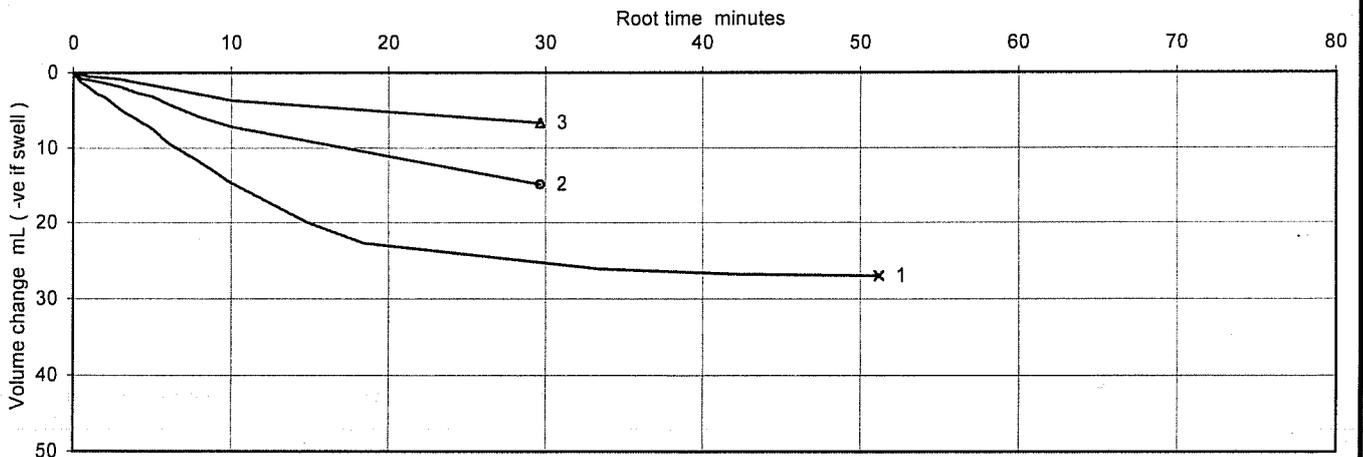
Specimen Details		
Initial		
Length	mm	203.85
Diameter	mm	101.92
Bulk Density	Mg/m <sup>3</sup>	2.26
Water Content	%	14
Dry density	Mg/m <sup>3</sup>	1.97
After test		
Bulk Density	Mg/m <sup>3</sup>	2.28
Water Content	%	13
Dry density	Mg/m <sup>3</sup>	2.01

Soil Description	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	260
Final pore water pressure	kPa	200
Final B Value		0.96



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		380	420	450	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		80	120	150	kPa
	Pore pressure at start of consolidation		368	376	376	kPa
	Pore pressure at end of consolidation		303	300	302	kPa
	Pore pressure dissipation at end of consolidation		96	100	97	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	0.62	0.55	0.70	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.25	0.12	0.06	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	4.7E-11	2.0E-11	1.2E-11	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 09:48

Figure

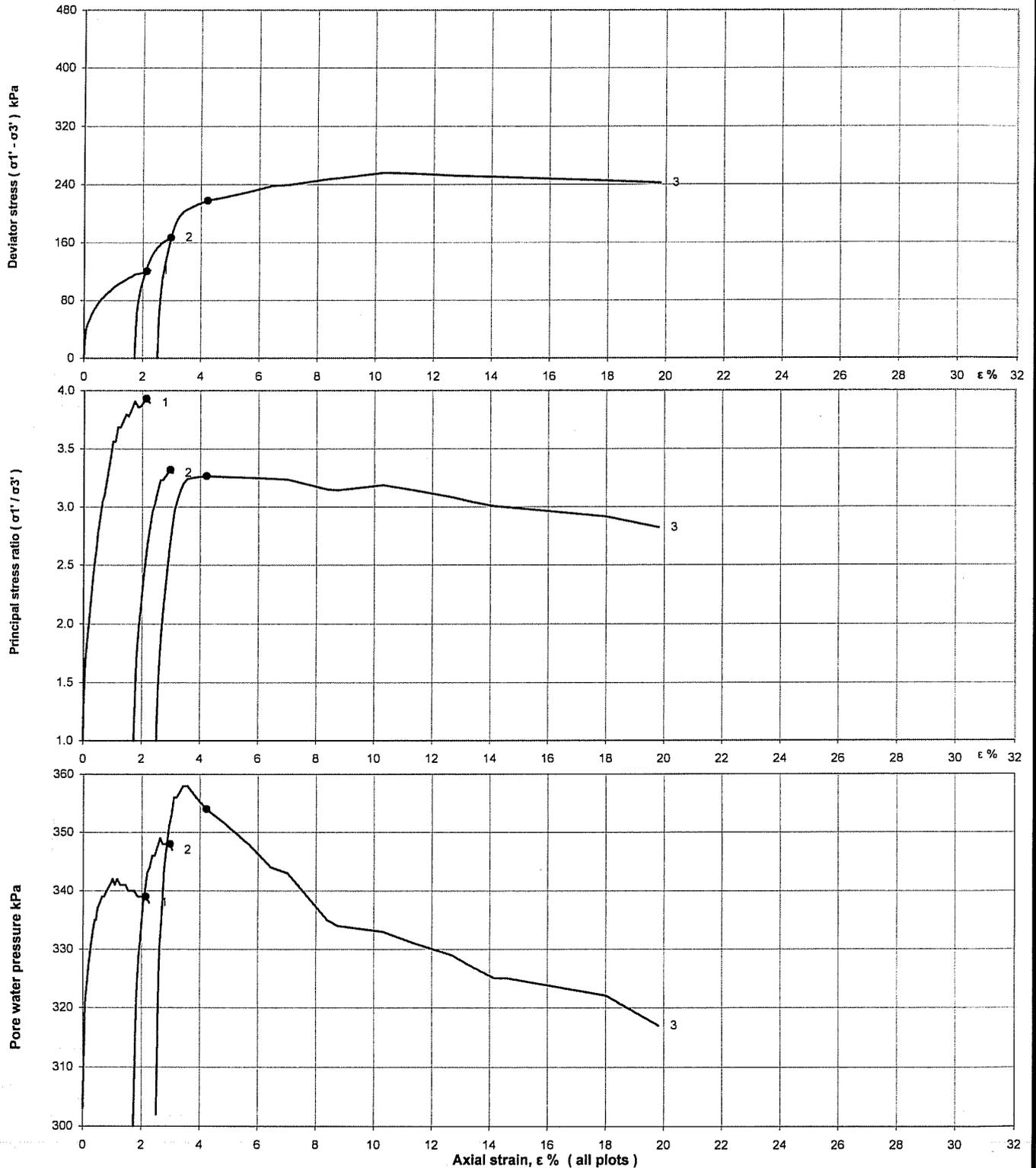
**CUM 1**

sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00-5.45	
		No	13	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**



Ref

SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 09:48

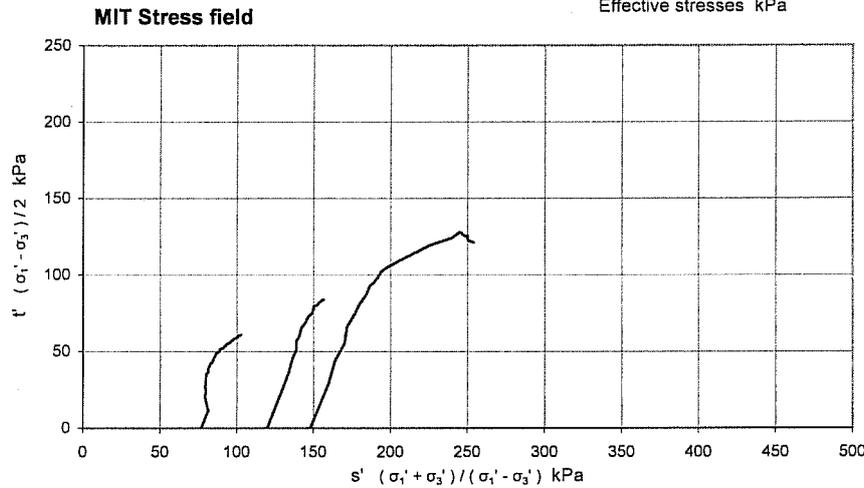
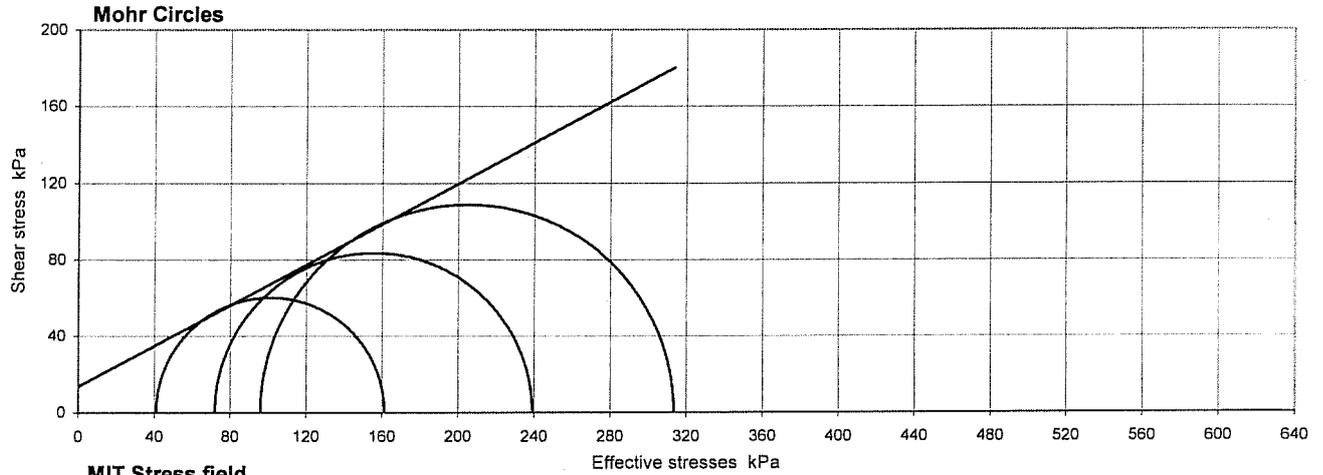
Figure

**CUM 1**

sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00-5.45		
			No	13	Type	U
			ID			
			Spec Ref			

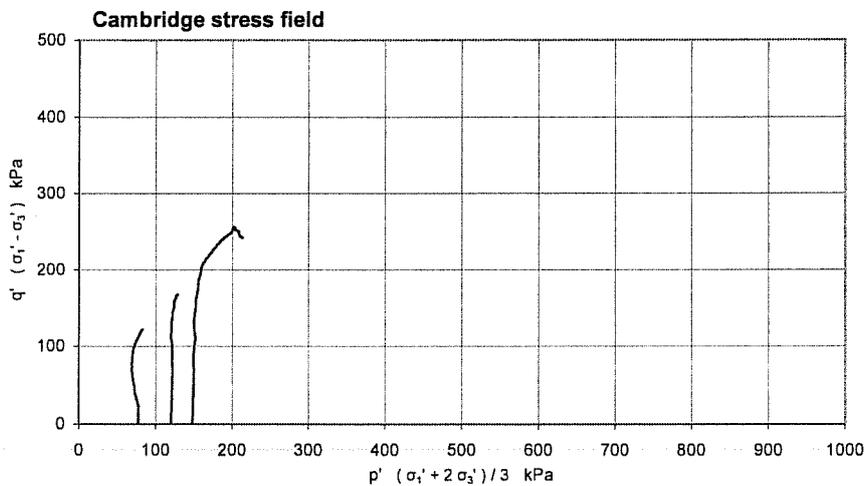


**Compression stages**

Stage	1	2	3	
Cell pressure	380	420	450	kPa
Initial pwp	303	300	302	kPa
Initial $\sigma_3'$	77	120	148	kPa
Rate of strain	0.48	0.48	0.48	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.13	2.97	4.22	%
$(\sigma_1'/\sigma_3')_f$	3.932	3.320	3.266	
$(\sigma_1' - \sigma_3')_f$	120.2	167.0	217.6	kPa
$u_f$	339	348	354	kPa
$\sigma_3'_f$	41	72	96	kPa
$\sigma_1'_f$	161	239	314	kPa
$A_f$	0.30	0.29	0.24	
Time to failure	4.4	6.2	8.8	hrs



**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	13.6
$\phi'$	degrees	28.0
		Manual re-assessment
$c'$	kPa	-
$\phi'$	degrees	-

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



Ref

SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 09:48

Figure

**CUM 1**  
sheet 3 of 3

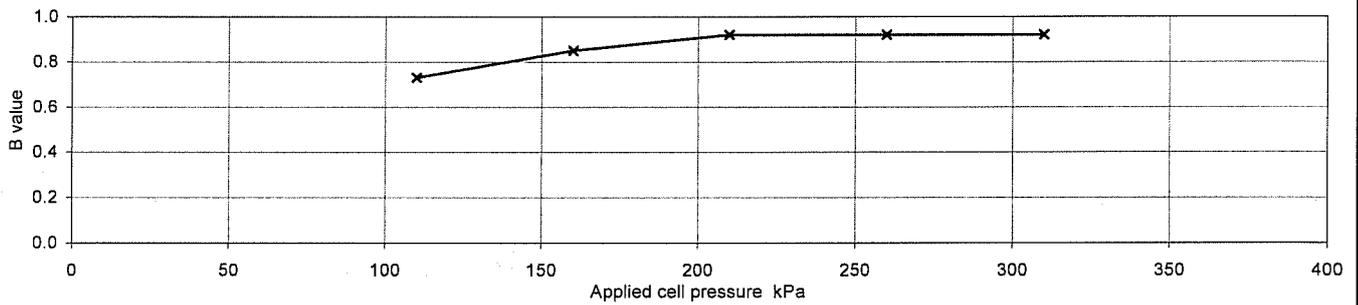
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.00-8.45		
			No	21	Type	U
			ID			
			Spec Ref			

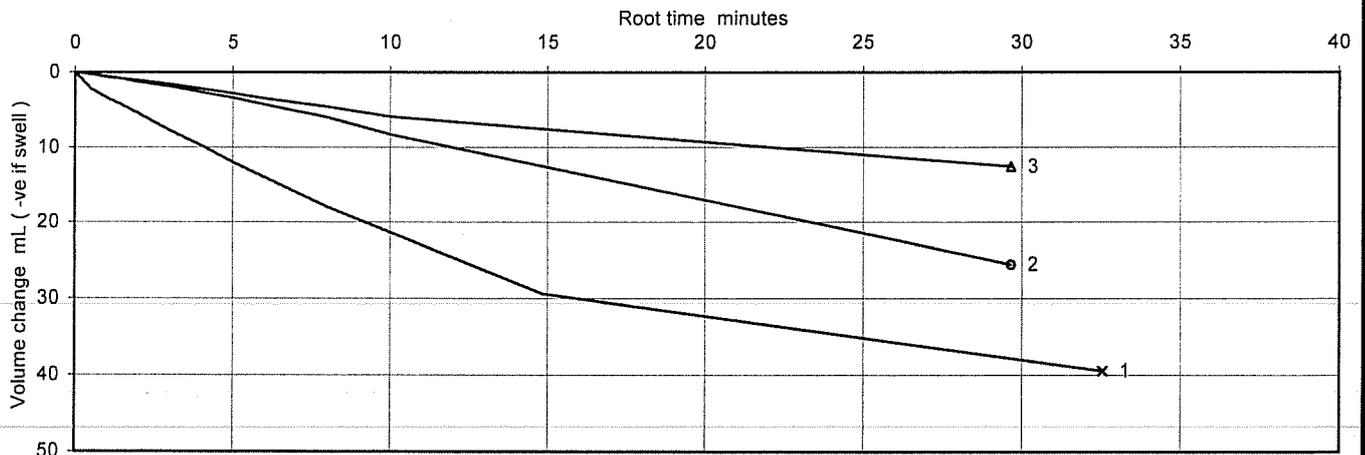
Specimen Details		
Initial		
Length	mm	204.04
Diameter	mm	103.12
Bulk Density	Mg/m <sup>3</sup>	2.20
Water Content	%	15
Dry density	Mg/m <sup>3</sup>	1.91
After test		
Bulk Density	Mg/m <sup>3</sup>	2.25
Water Content	%	13
Dry density	Mg/m <sup>3</sup>	1.99

Soil Description	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	310
Final pore water pressure	kPa	250
Final B Value		0.92



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		420	470	520	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		120	170	220	kPa
	Pore pressure at start of consolidation		406	406	419	kPa
	Pore pressure at end of consolidation		301	303	303	kPa
	Pore pressure dissipation at end of consolidation		100	98	97	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	0.66	0.25	0.49	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.22	0.15	0.07	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	4.4E-11	1.1E-11	9.9E-12	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 10:06

Figure

**CUM 2**

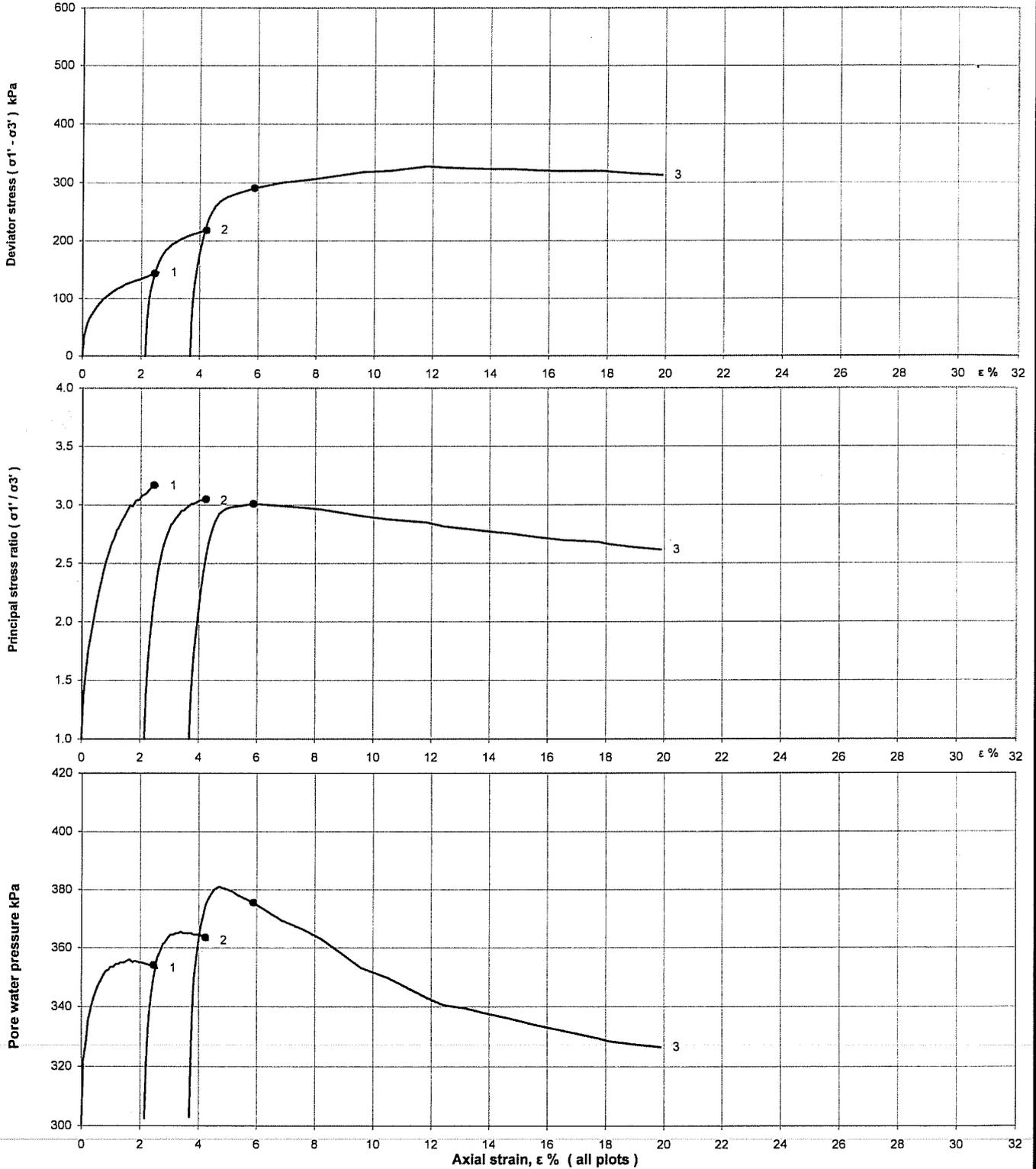
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.00-8.45	
		No	21	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



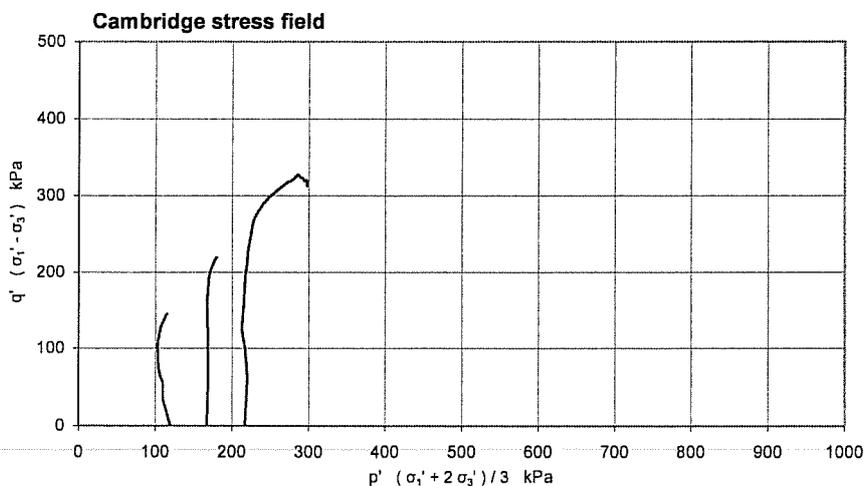
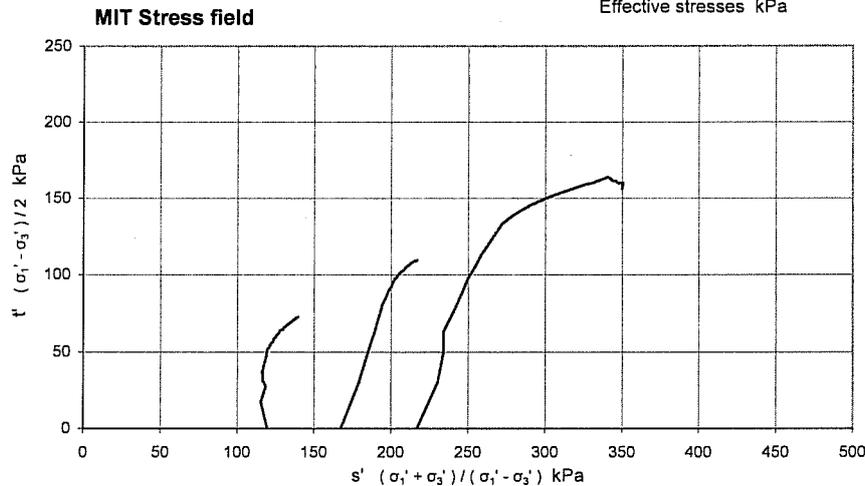
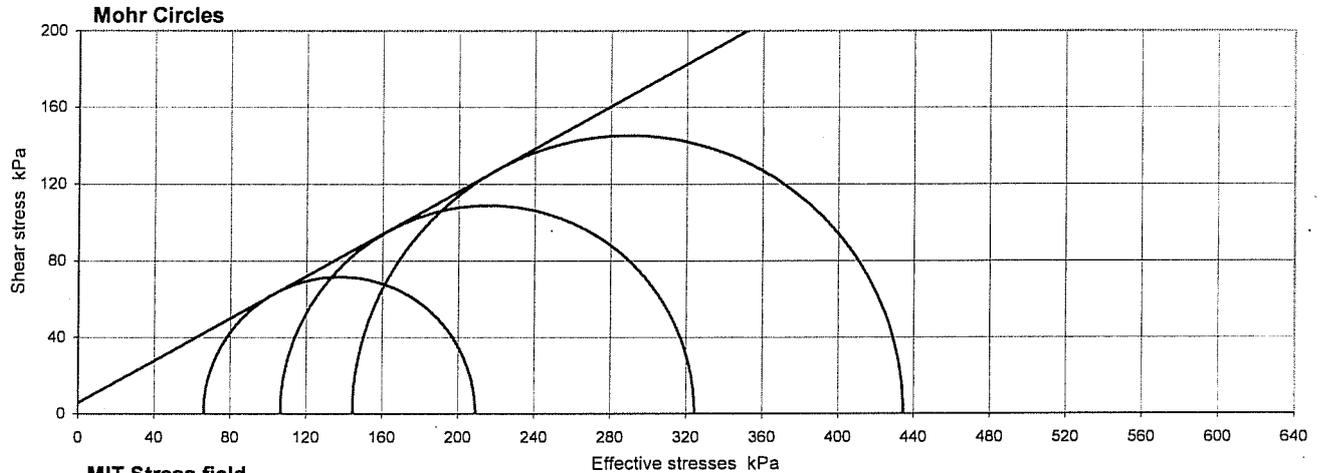
Printed:10/11/2011 10:06

Figure

**CUM 2**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	8.00-8.45	
		No	21	Type	U
		ID			
		Spec Ref			



**Compression stages**

Stage	1	2	3	
Cell pressure	420	470	520	kPa
Initial pwp	301	303	303	kPa
Initial $\sigma_3'$	120	168	217	kPa
Rate of strain	0.49	0.49	0.49	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.46	4.23	5.88	%
$(\sigma_1' / \sigma_3')_f$	3.168	3.047	3.008	
$(\sigma_1' - \sigma_3')_f$	143.1	218.0	290.2	kPa
$u_f$	354	364	376	kPa
$\sigma_3'_f$	66	107	145	kPa
$\sigma_1'_f$	209	325	435	kPa
$A_f$	0.37	0.28	0.25	
Time to failure	5.0	8.6	12.0	hrs

**Shear Strength Parameters**

at peak stress ratio

		Linear regression	
$c'$	kPa	5.6	
$\phi'$	degrees	28.9	
		Manual re-assessment	
$c'$	kPa	-	
$\phi'$	degrees	-	

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Ref

SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 10:06

Figure

**CUM 2**

sheet 3 of 3

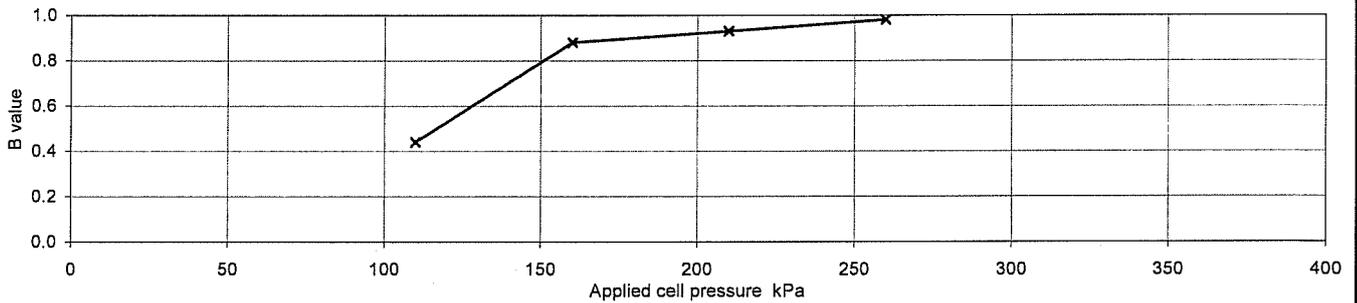
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.50-15.95		
			No	41	Type	U
			ID			
			Spec Ref			

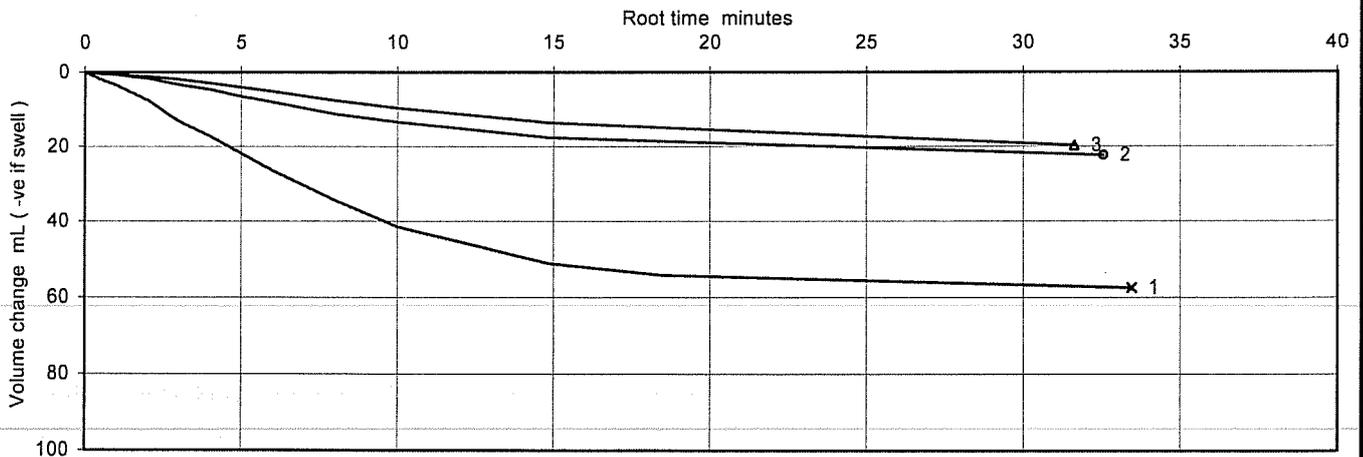
Specimen Details		
Initial		
Length	mm	205.22
Diameter	mm	103.64
Bulk Density	Mg/m <sup>3</sup>	2.11
Water Content	%	19
Dry density	Mg/m <sup>3</sup>	1.78
After test		
Bulk Density	Mg/m <sup>3</sup>	2.14
Water Content	%	17
Dry density	Mg/m <sup>3</sup>	1.84

Soil Description	Stiff brown thinly laminated CLAY with thin laminations of sand becoming slightly sandy slightly gravelly CLAY at base.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	260
Final pore water pressure	kPa	200
Final B Value		0.98



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		505	590	665	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		205	290	365	kPa
	Pore pressure at start of consolidation		491	487	496	kPa
	Pore pressure at end of consolidation		300	303	306	kPa
	Pore pressure dissipation at end of consolidation		100	99	97	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	1.37	0.97	0.63	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.17	0.07	0.06	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	7.2E-11	2.1E-11	1.2E-11	m/s



Ref

SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 11:41

Figure

**CUM 3**

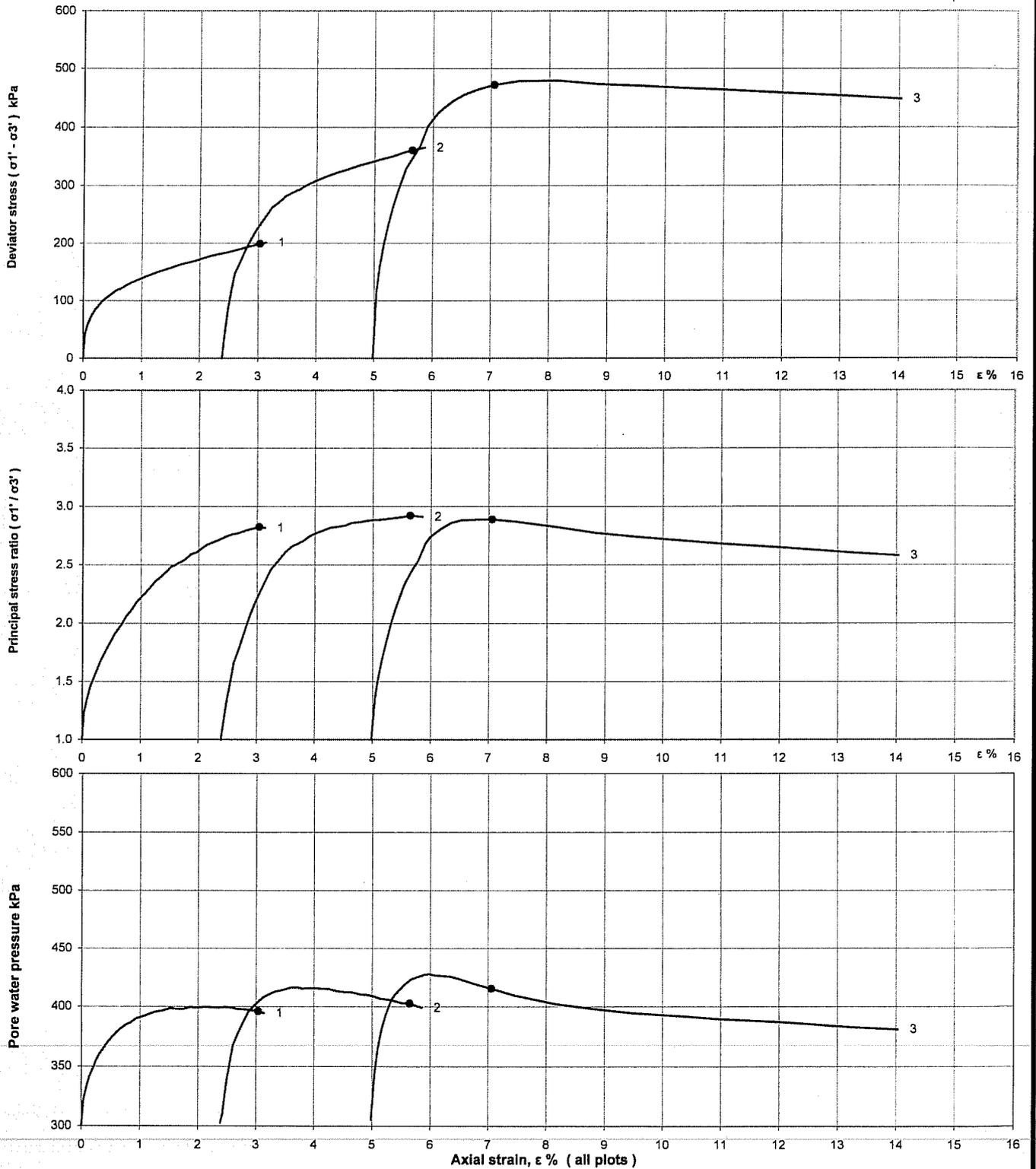
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.50-15.95	
		No	41	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



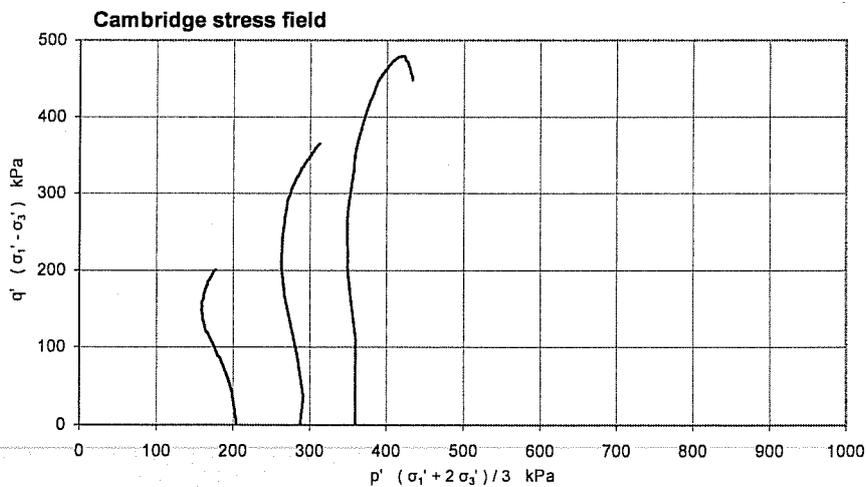
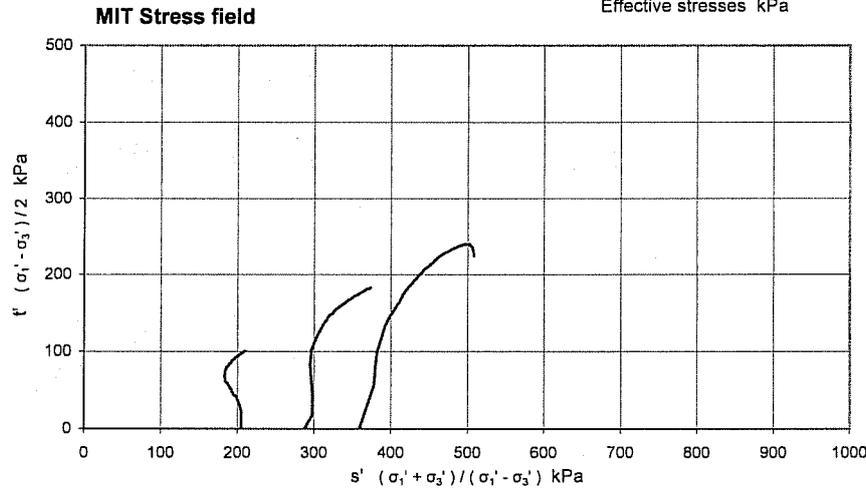
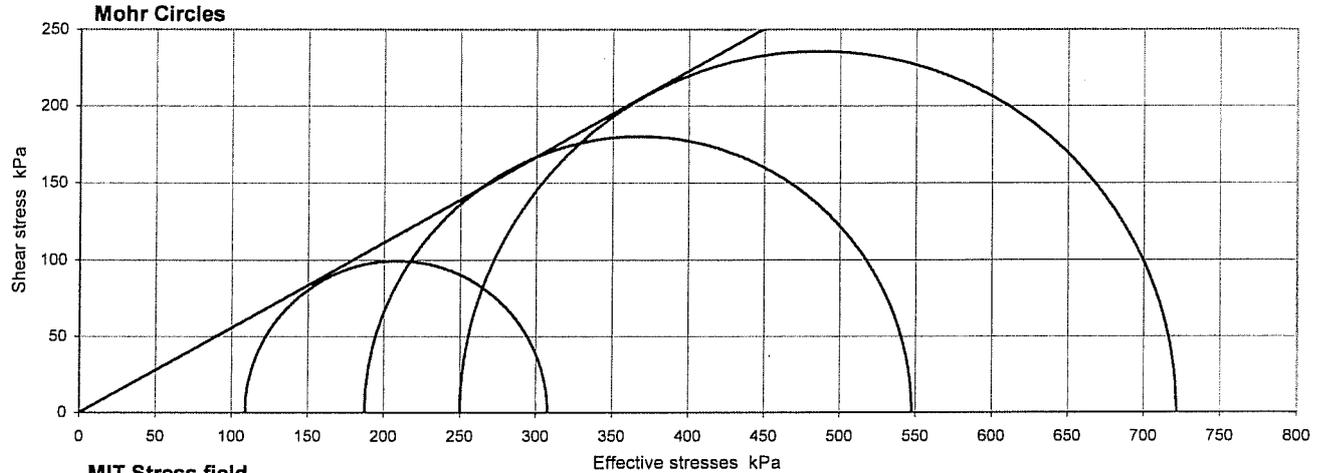
Printed: 10/11/2011 11:41

Figure

**CUM 3**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.50-15.95	
		No	41	Type	U
		ID			
		Spec Ref			



**Compression stages**

Stage	1	2	3	
Cell pressure	505	590	665	kPa
Initial pwp	300	303	306	kPa
Initial $\sigma_3'$	205	288	360	kPa
Rate of strain	1.00	1.00	1.00	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	3.04	5.65	7.06	%
$(\sigma_1' / \sigma_3')_f$	2.822	2.920	2.887	
$(\sigma_1' - \sigma_3')_f$	198.6	360.1	471.6	kPa
$u_f$	396	403	415	kPa
$\sigma_3' f$	109	188	250	kPa
$\sigma_1' f$	308	548	722	kPa
$A_f$	0.48	0.28	0.23	
Time to failure	3.0	5.6	7.1	hrs

**Shear Strength Parameters at peak stress ratio**

		Linear regression
$c'$	kPa	(-3.1)
$\phi'$	degrees	(29.5)
		Manual re-assessment
$c'$	kPa	0
$\phi'$	degrees	29.1

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



Ref

SLR8.1  
Rev 85  
May 09



Printed:10/11/2011 11:41

Figure

**CUM 3**  
sheet 3 of 3

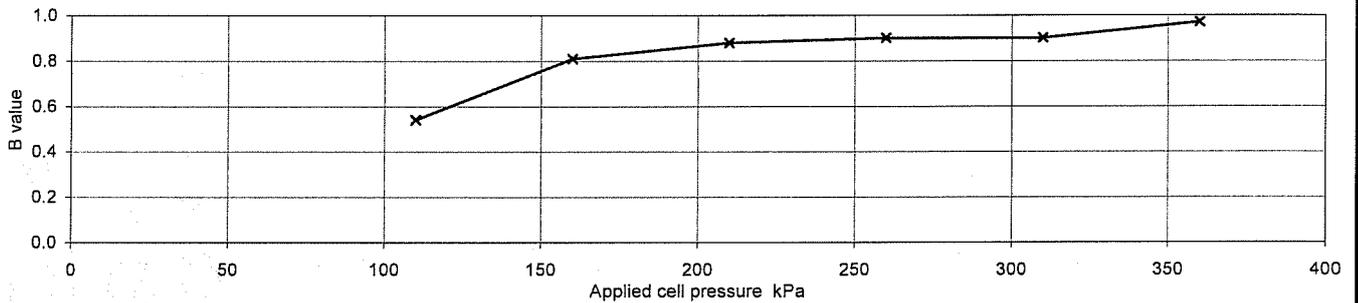
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.50-19.95
			No	52
			Type	U
			ID	
			Spec Ref	

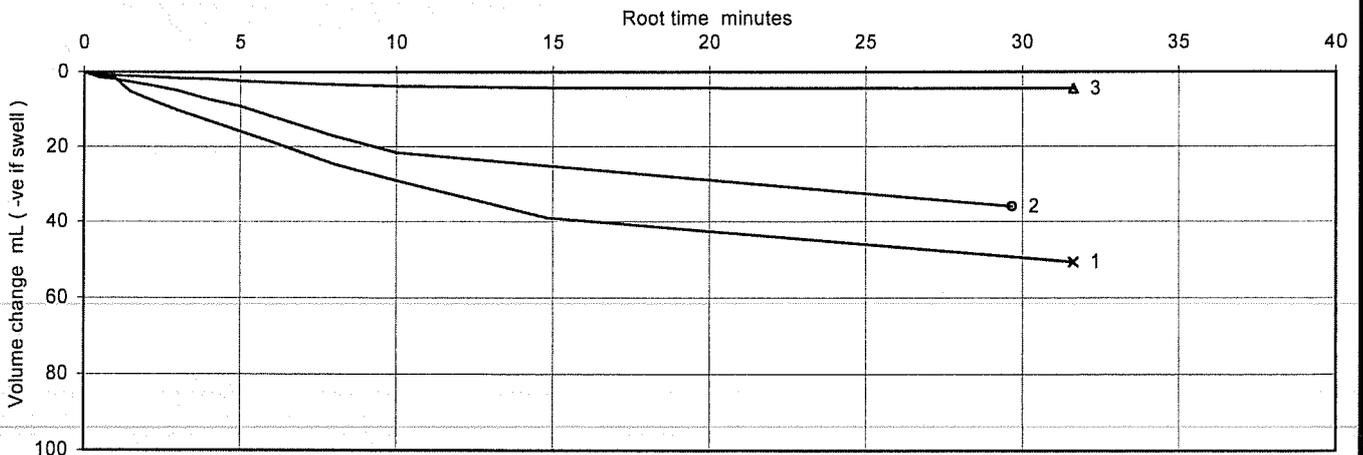
Specimen Details		
Initial		
Length	mm	204.42
Diameter	mm	101.75
Bulk Density	Mg/m <sup>3</sup>	2.24
Water Content	%	12
Dry density	Mg/m <sup>3</sup>	1.99
After test		
Bulk Density	Mg/m <sup>3</sup>	2.27
Water Content	%	11
Dry density	Mg/m <sup>3</sup>	2.04

Soil Description	Stiff greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.97



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		580	700	790	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		280	400	490	kPa
	Pore pressure at start of consolidation		563	521	536	kPa
	Pore pressure at end of consolidation		306	301	310	kPa
	Pore pressure dissipation at end of consolidation		98	100	96	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	0.74	0.83	1.99	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.12	0.10	0.01	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	2.6E-11	2.5E-11	7.5E-12	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 11:09

Figure

**CUM 4**

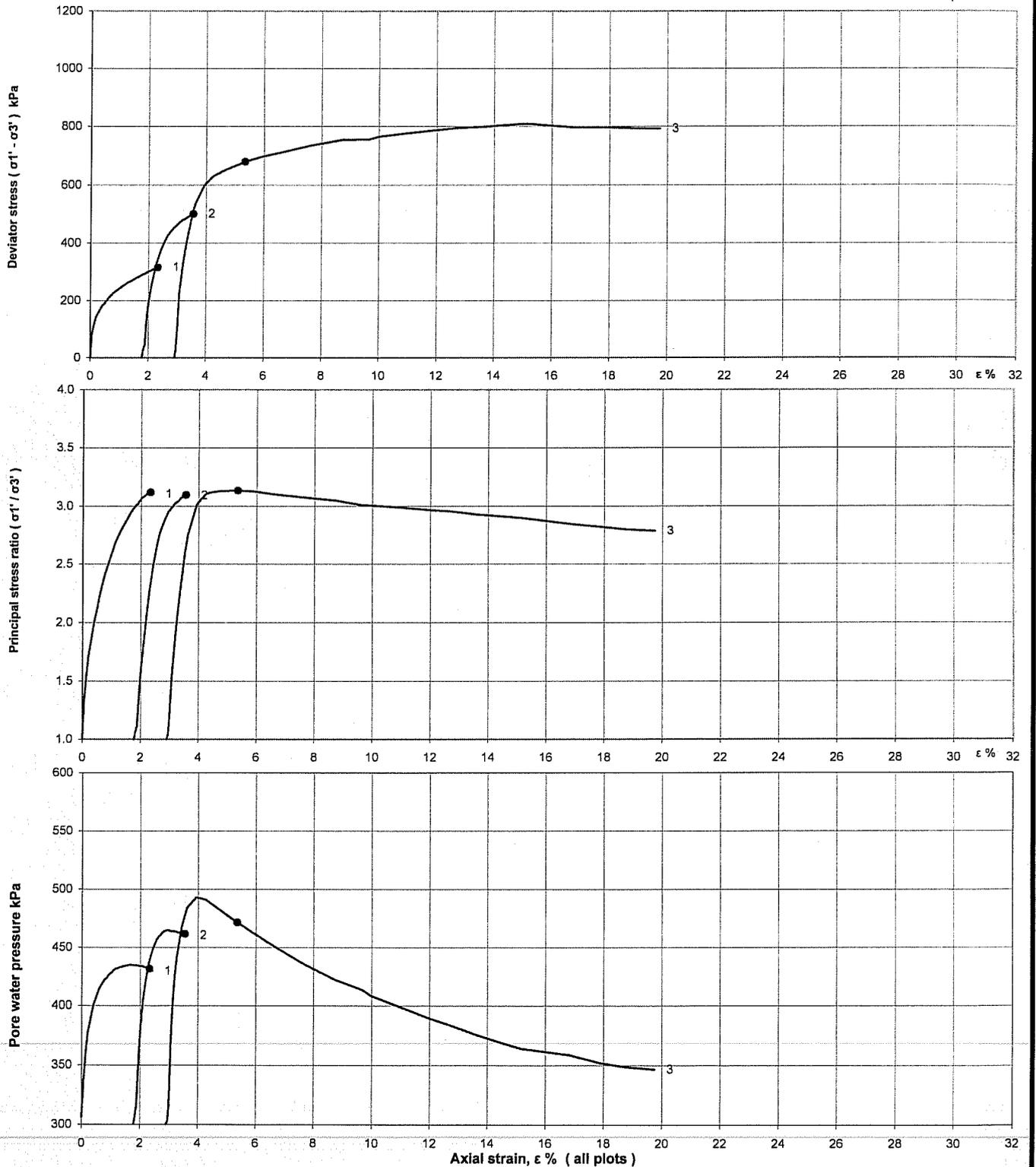
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.50-19.95	
		No	52	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



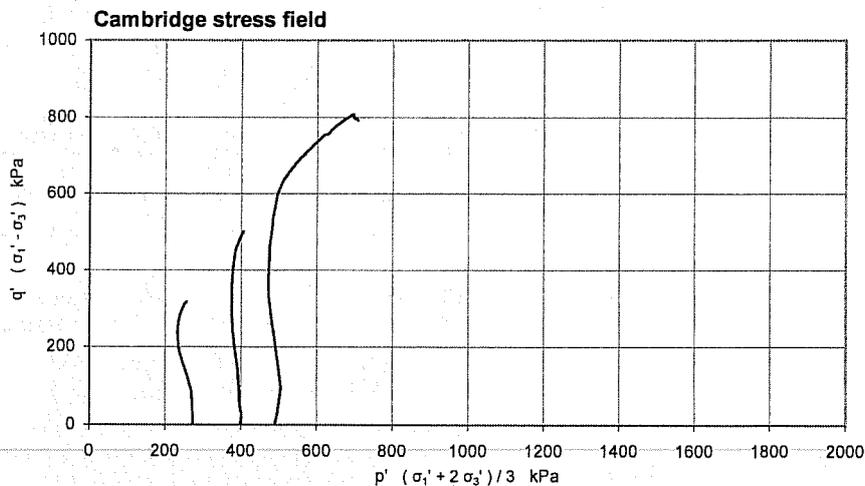
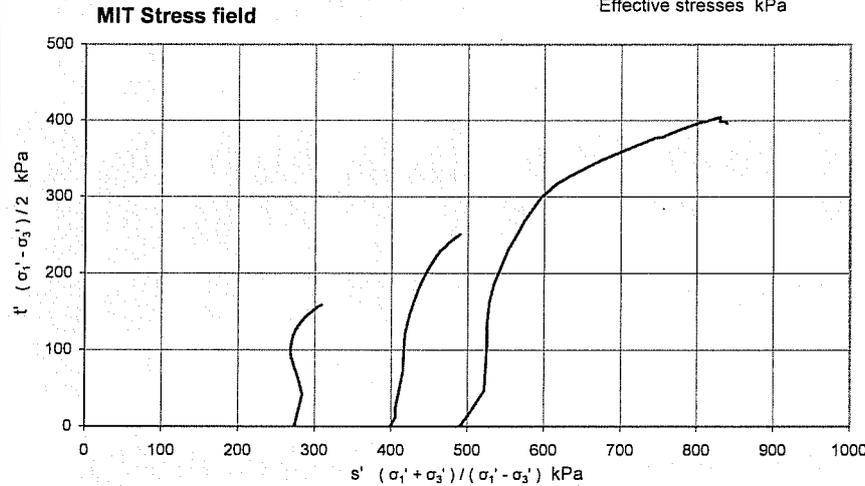
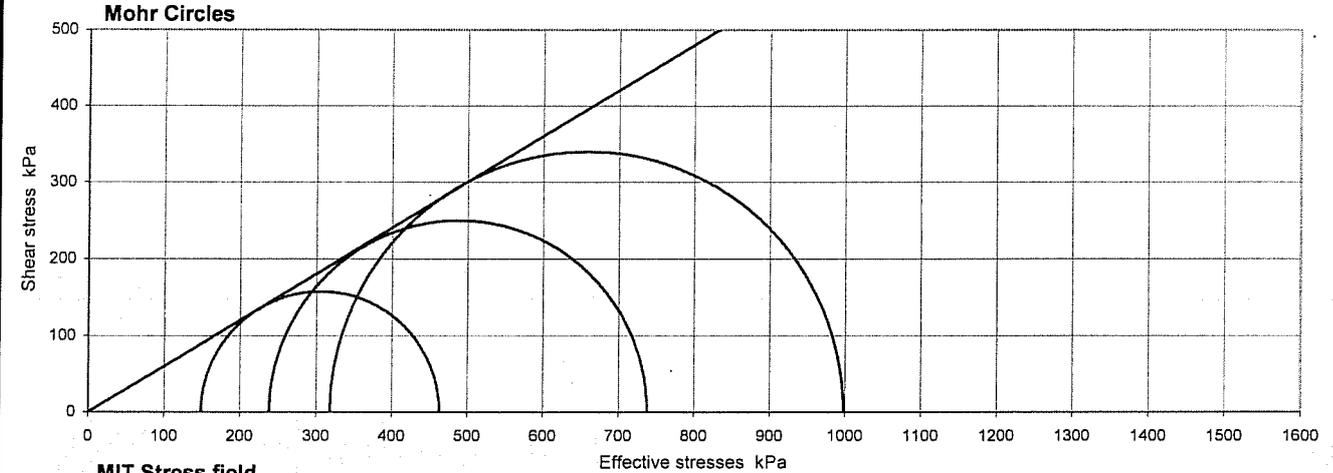
Printed: 11/11/2011 11:09

Figure

**CUM 4**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.50-19.95		
			No	52	Type	U
			ID			
		Spec Ref				



**Compression stages**

Stage	1	2	3	
Cell pressure	580	700	790	kPa
Initial pwp	306	301	300	kPa
Initial $\sigma_3'$	274	400	490	kPa
Rate of strain	0.57	0.57	0.57	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.33	3.56	5.35	%
$(\sigma_1' / \sigma_3')_f$	3.118	3.094	3.133	
$(\sigma_1' - \sigma_3')_f$	314.5	499.4	679.4	kPa
$u_f$	432	462	472	kPa
$\sigma_3'_f$	149	239	319	kPa
$\sigma_1'_f$	463	738	998	kPa
$A_f$	0.40	0.32	0.25	
Time to failure	4.1	6.2	9.4	hrs

**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	(-1.9)
$\phi'$	degrees	(31.2)
		Manual re-assessment
$c'$	kPa	0
$\phi'$	degrees	31.0

Notes : Deviator stresses corrected for area change, vertical side drains and 0.9 mm thick rubber membrane(s)

Mode of failure



Ref

SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 11:09

Figure

**CUM 4**  
sheet 3 of 3

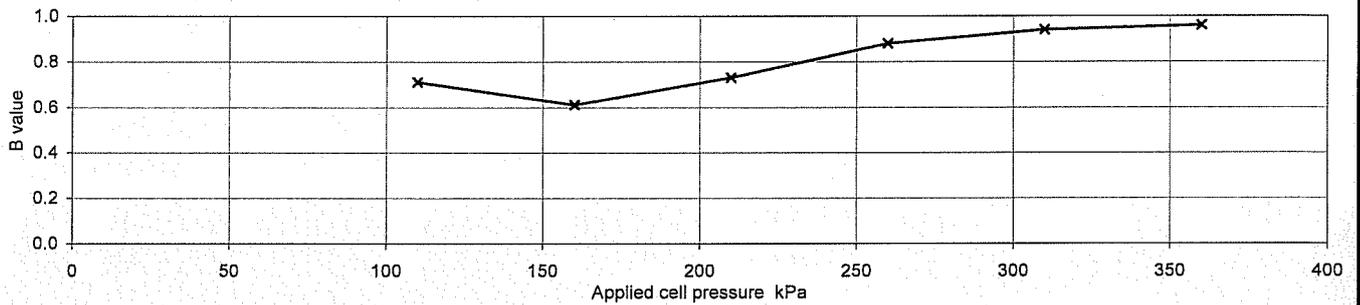
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.20-1.65		
			No	4	Type	U
			ID			
			Spec Ref			

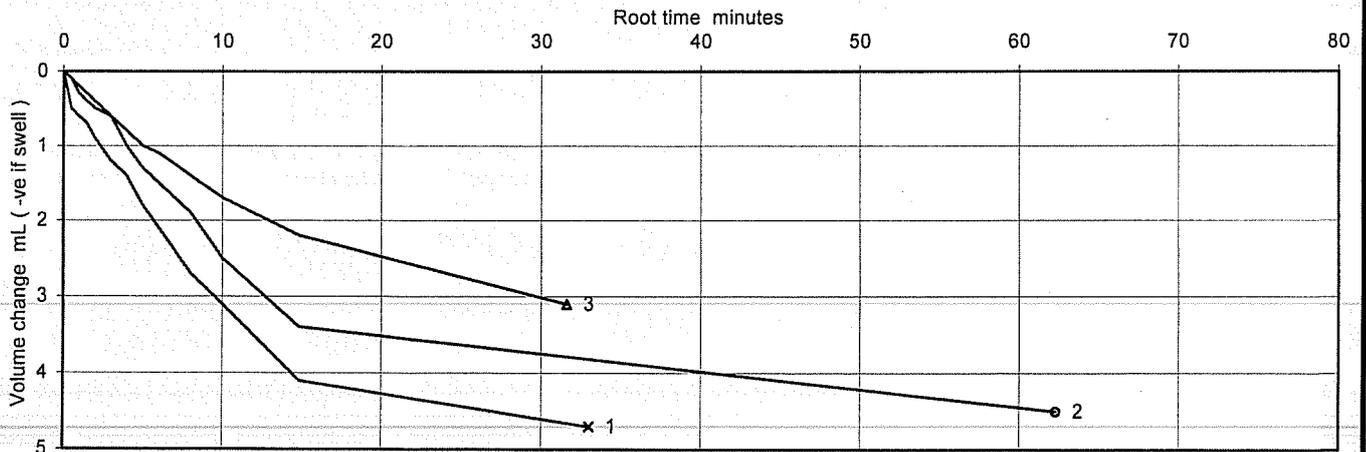
Specimen Details		
Initial		
Length	mm	204.29
Diameter	mm	101.26
Bulk Density	Mg/m <sup>3</sup>	2.07
Water Content	%	21
Dry density	Mg/m <sup>3</sup>	1.72
After test		
Bulk Density	Mg/m <sup>3</sup>	2.07
Water Content	%	22
Dry density	Mg/m <sup>3</sup>	1.69

Soil Description	Firm to stiff greyish brown and brownish grey slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.96



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		375	385	395	kPa
	Back Pressure applied		350	350	350	kPa
	Effective Pressure		25	35	45	kPa
	Pore pressure at start of consolidation		362	361	362	kPa
	Pore pressure at end of consolidation		350	350	351	kPa
	Pore pressure dissipation at end of consolidation		100	100	96	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	1.01	0.62	0.57	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.24	0.24	0.17	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	7.6E-11	4.7E-11	3.0E-11	m/s



Ref  
SLR8.1  
Rev 85  
May 09



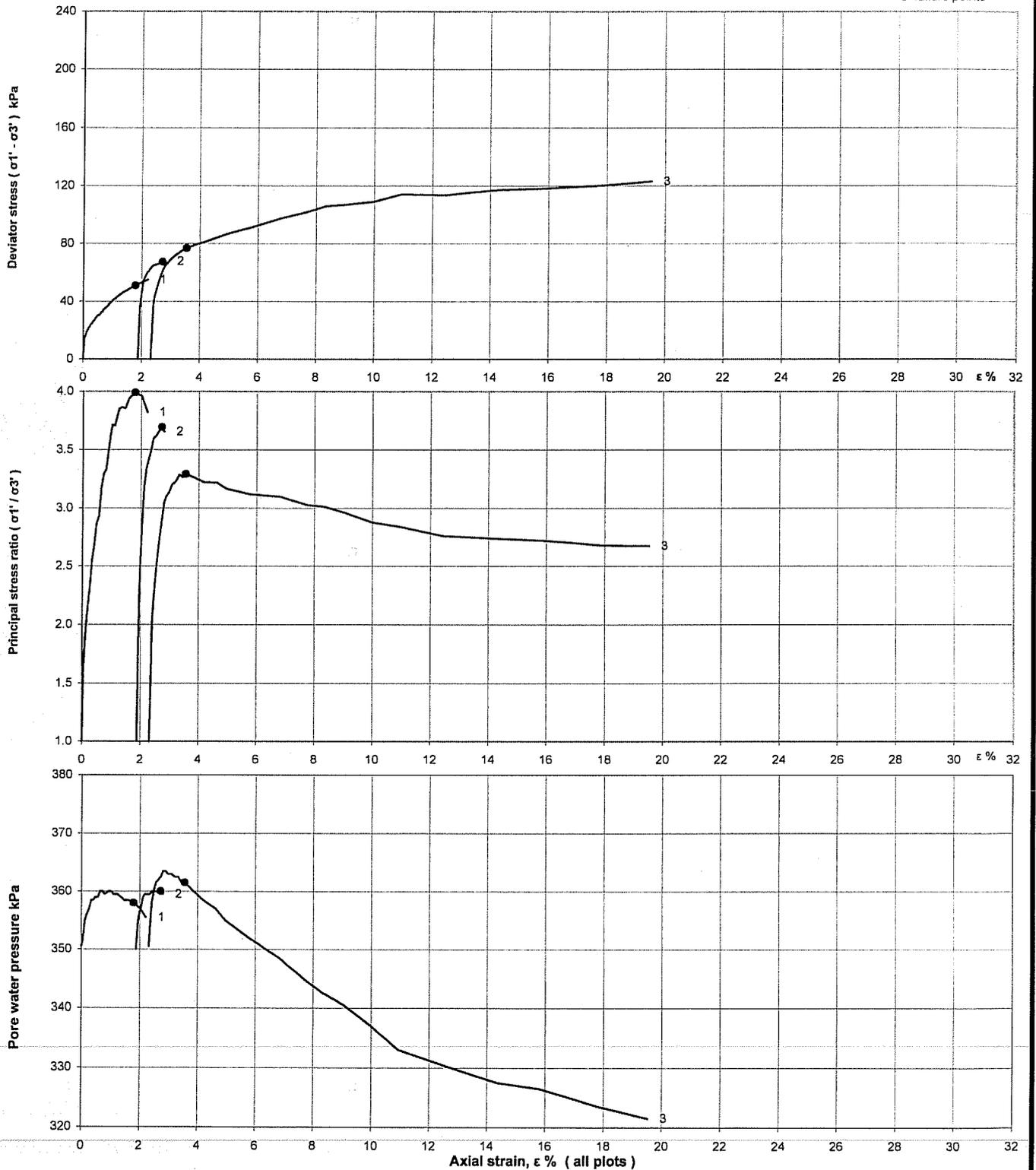
Printed: 18/11/2011 16:19

Figure  
**CUM 5**  
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.20-1.65	
		No	4	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**



Ref

SLR8.1  
Rev 85  
May 09



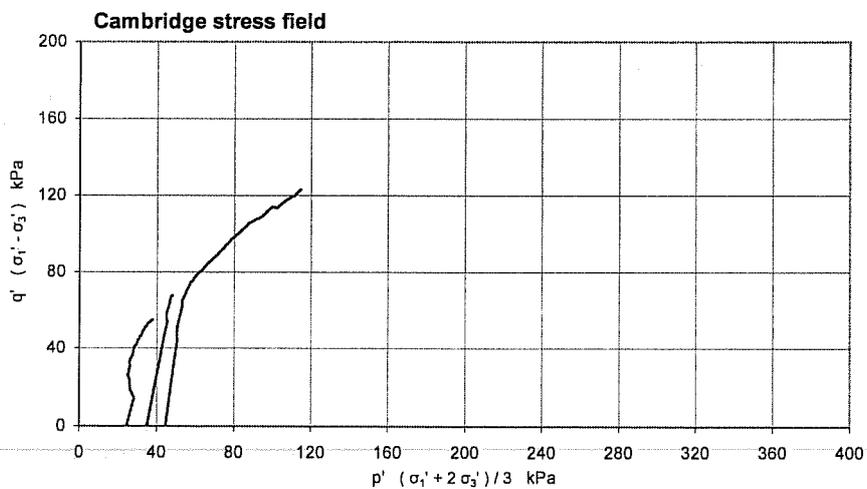
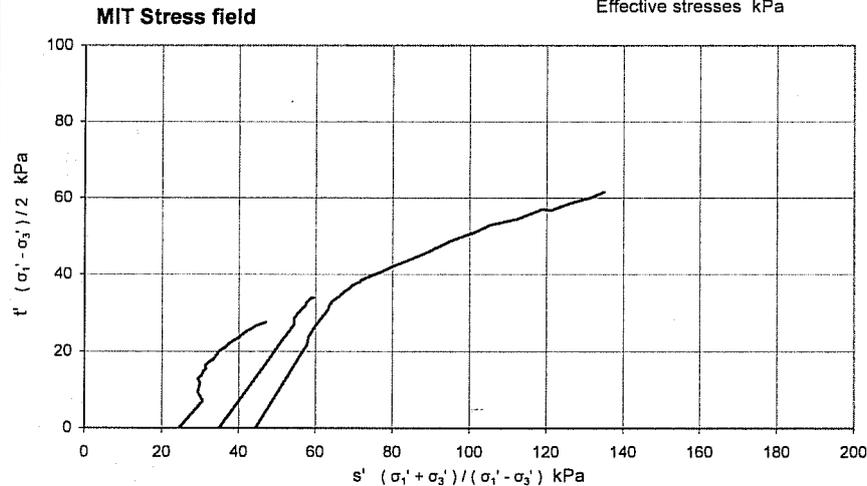
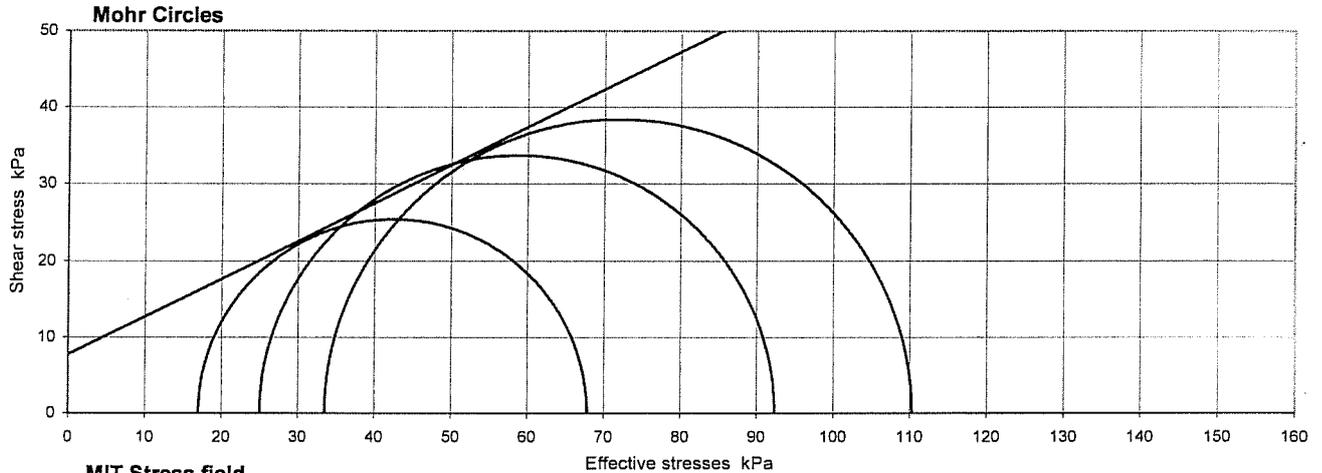
Printed: 18/11/2011 16:20

Figure

**CUM 5**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.20-1.65		
			No	4	Type	U
			ID			
			Spec Ref			



**Compression stages**

Stage	1	2	3	
Cell pressure	375	385	395	kPa
Initial pwp	351	350	351	kPa
Initial $\sigma_3'$	25	35	45	kPa
Rate of strain	0.79	0.79	0.79	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	1.79	2.73	3.56	%
$(\sigma_1' / \sigma_3')_f$	3.990	3.695	3.292	
$(\sigma_1' - \sigma_3')_f$	50.8	67.4	76.8	kPa
$u_f$	358	360	362	kPa
$\sigma_3' f$	17	25	34	kPa
$\sigma_1' f$	68	92	110	kPa
$A_f$	0.15	0.15	0.14	
Time to failure	2.3	3.5	4.5	hrs

**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	7.8
$\phi'$	degrees	26.3
		Manual re-assessment
$c'$	kPa	-
$\phi'$	degrees	-

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Ref

SLR8.1  
Rev 85  
May 09



Printed:18/11/2011 16:20

Figure

**CUM 5**  
sheet 3 of 3

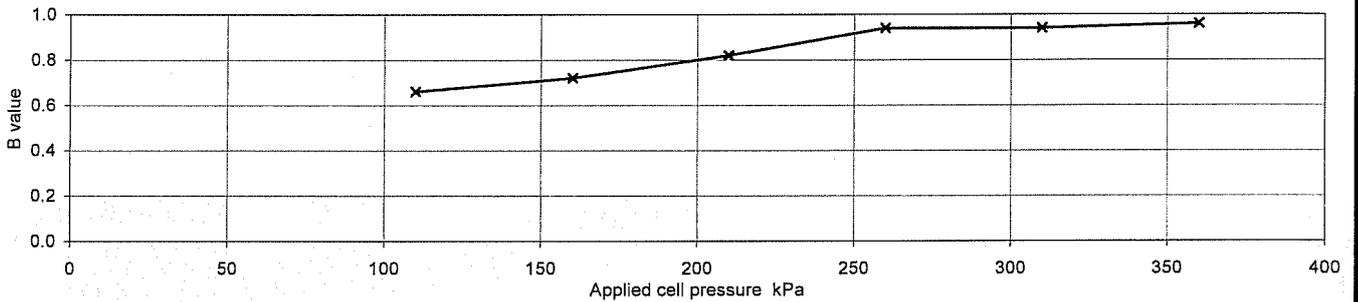
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45		
			No	22	Type	U
			ID			
			Spec Ref			

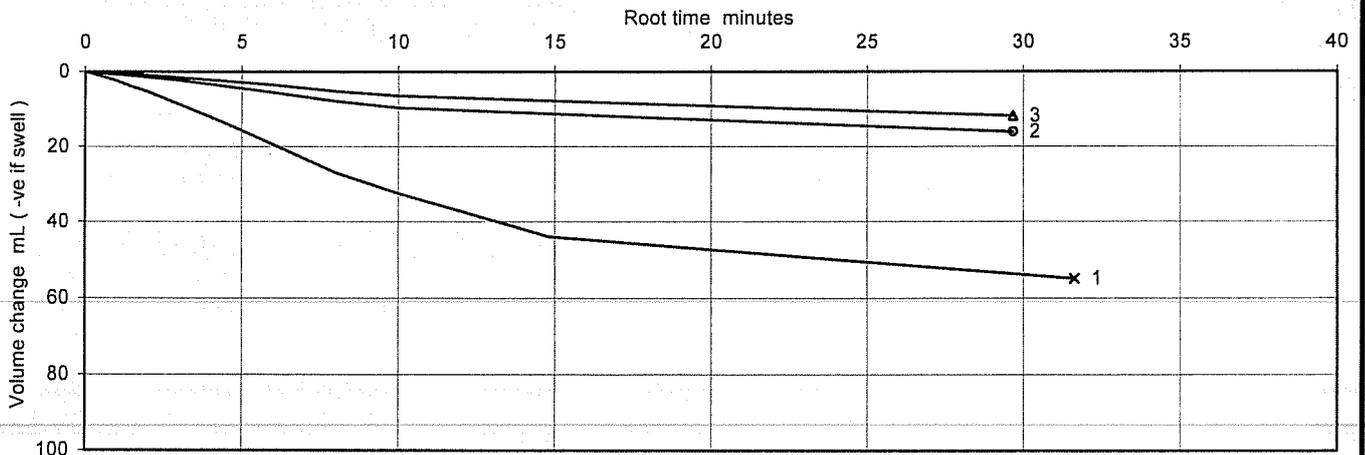
Specimen Details		
Initial		
Length	mm	203.69
Diameter	mm	103.43
Bulk Density	Mg/m <sup>3</sup>	2.18
Water Content	%	16
Dry density	Mg/m <sup>3</sup>	1.88
After test		
Bulk Density	Mg/m <sup>3</sup>	2.24
Water Content	%	14
Dry density	Mg/m <sup>3</sup>	1.97

Soil Description	Firm becoming soft to firm at base greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.96



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		450	510	565	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		150	210	265	kPa
	Pore pressure at start of consolidation		426	434	446	kPa
	Pore pressure at end of consolidation		300	302	302	kPa
	Pore pressure dissipation at end of consolidation		100	99	99	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	0.96	0.94	0.80	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.25	0.07	0.05	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	7.5E-11	2.1E-11	1.2E-11	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 11:15

Figure

**CUM 6**

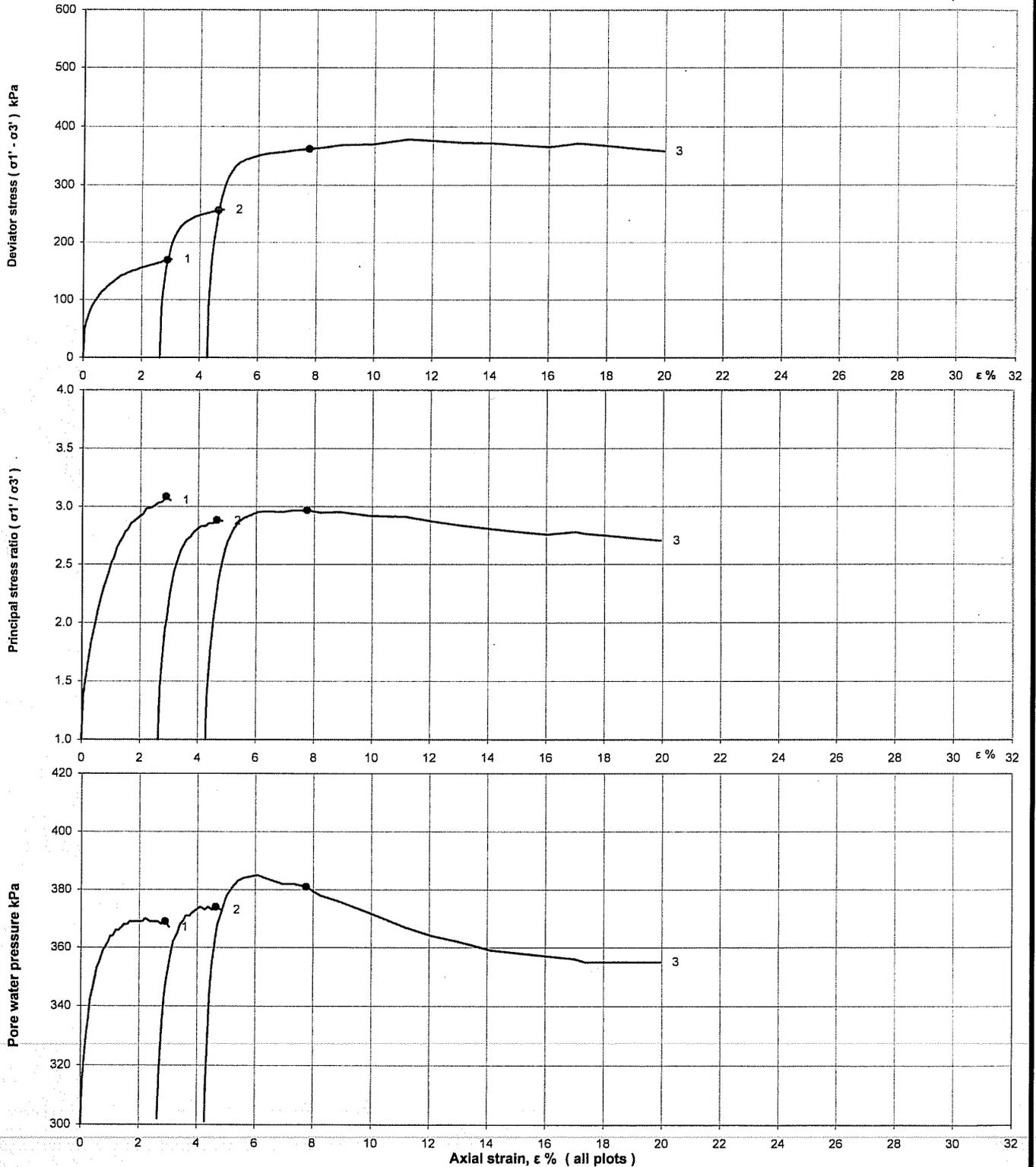
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45	
		No	22	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



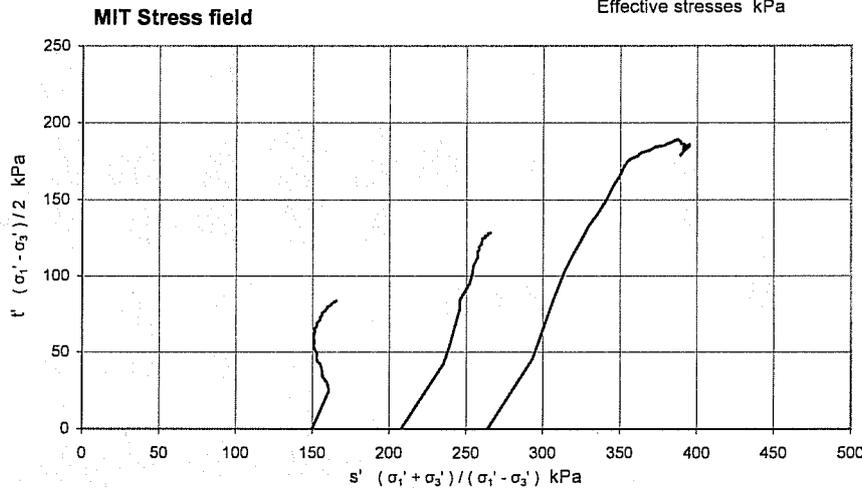
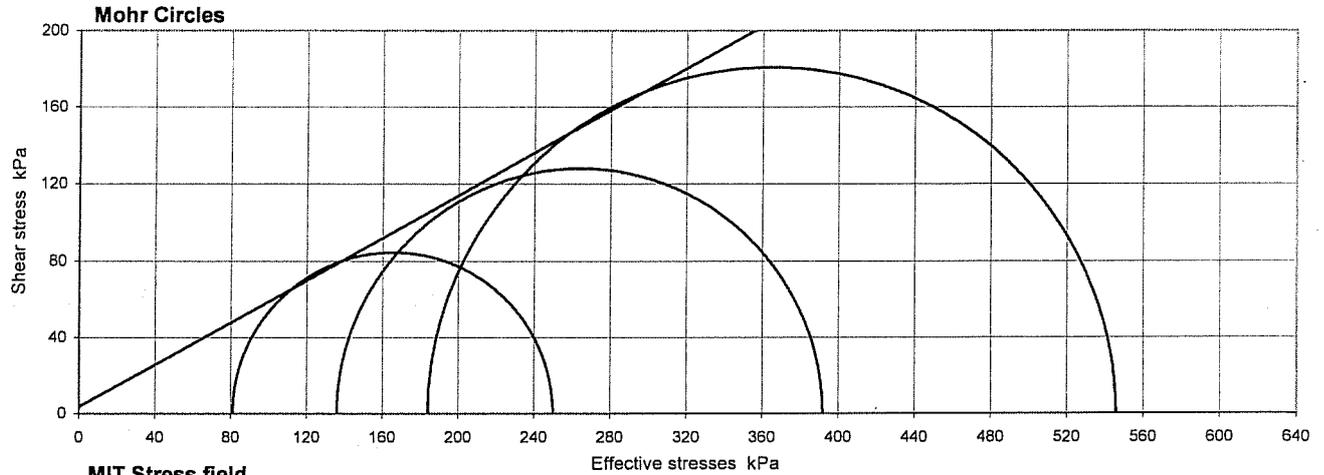
Printed:11/11/2011 11:15

Figure

**CUM 6**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45		
			No	22	Type	U
			ID			
		Spec Ref				

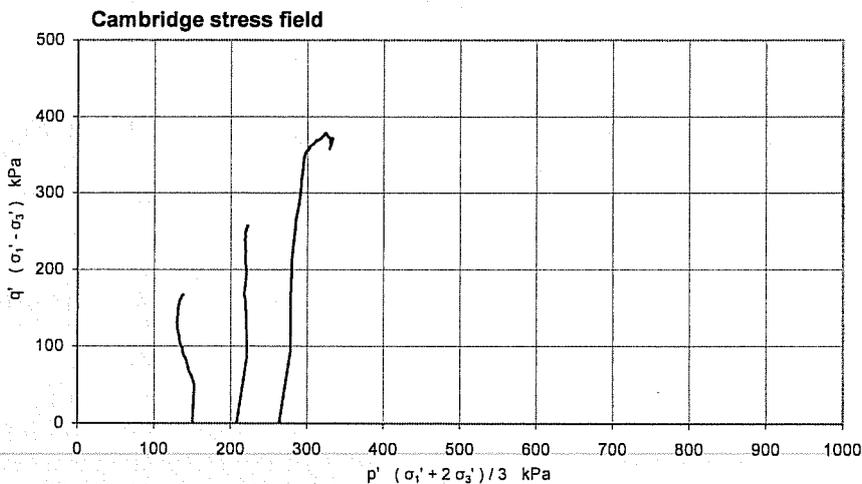


**Compression stages**

Stage	1	2	3	
Cell pressure	450	510	565	kPa
Initial pwp	300	302	301	kPa
Initial $\sigma_3'$	150	208	264	kPa
Rate of strain	0.70	0.70	0.70	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.90	4.64	7.75	%
$(\sigma_1' / \sigma_3')_f$	3.085	2.881	2.966	
$(\sigma_1' - \sigma_3')_f$	168.9	255.9	361.8	kPa
$u_f$	369	374	381	kPa
$\sigma_3'_f$	81	136	184	kPa
$\sigma_1'_f$	250	392	546	kPa
$A_f$	0.41	0.28	0.22	
Time to failure	4.1	6.6	11.1	hrs



**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	3.5
$\phi'$	degrees	28.9
		Manual re-assessment
$c'$	kPa	-
$\phi'$	degrees	-

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Ref

SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 11:15

Figure

**CUM 6**  
sheet 3 of 3

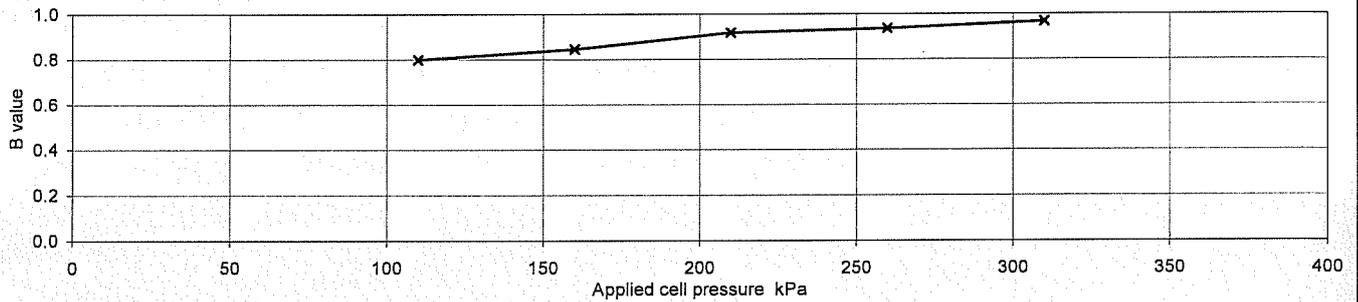
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45
			No	28
			Type	U
			ID	
		Spec Ref		

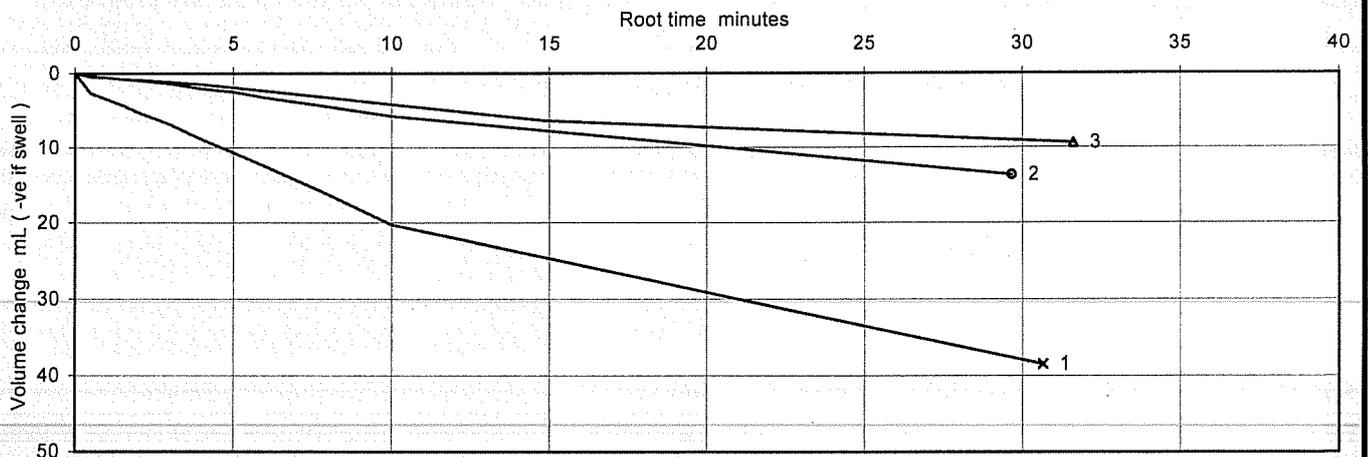
Specimen Details		
Initial		
Length	mm	204.74
Diameter	mm	102.64
Bulk Density	Mg/m <sup>3</sup>	2.21
Water Content	%	14
Dry density	Mg/m <sup>3</sup>	1.94
After test		
Bulk Density	Mg/m <sup>3</sup>	2.26
Water Content	%	13
Dry density	Mg/m <sup>3</sup>	2.01

Soil Description	Firm to stiff greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	310
Final pore water pressure	kPa	250
Final B Value		0.97



Consolidation Details	Drainage Conditions	From radial boundary and one end				
	Stage No.	1	2	3		
	Cell Pressure applied	450	510	565	kPa	
	Back Pressure applied	300	300	300	kPa	
	Effective Pressure	150	210	265	kPa	
	Pore pressure at start of consolidation	437	420	427	kPa	
	Pore pressure at end of consolidation	303	305	306	kPa	
	Pore pressure dissipation at end of consolidation	98	96	95	%	
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vt</sub>	0.56	0.41	0.46	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vt</sub>	0.17	0.07	0.05	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vt</sub>	3.0E-11	9.1E-12	6.8E-12	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:14/11/2011 16:10

Figure

**CUM 7**

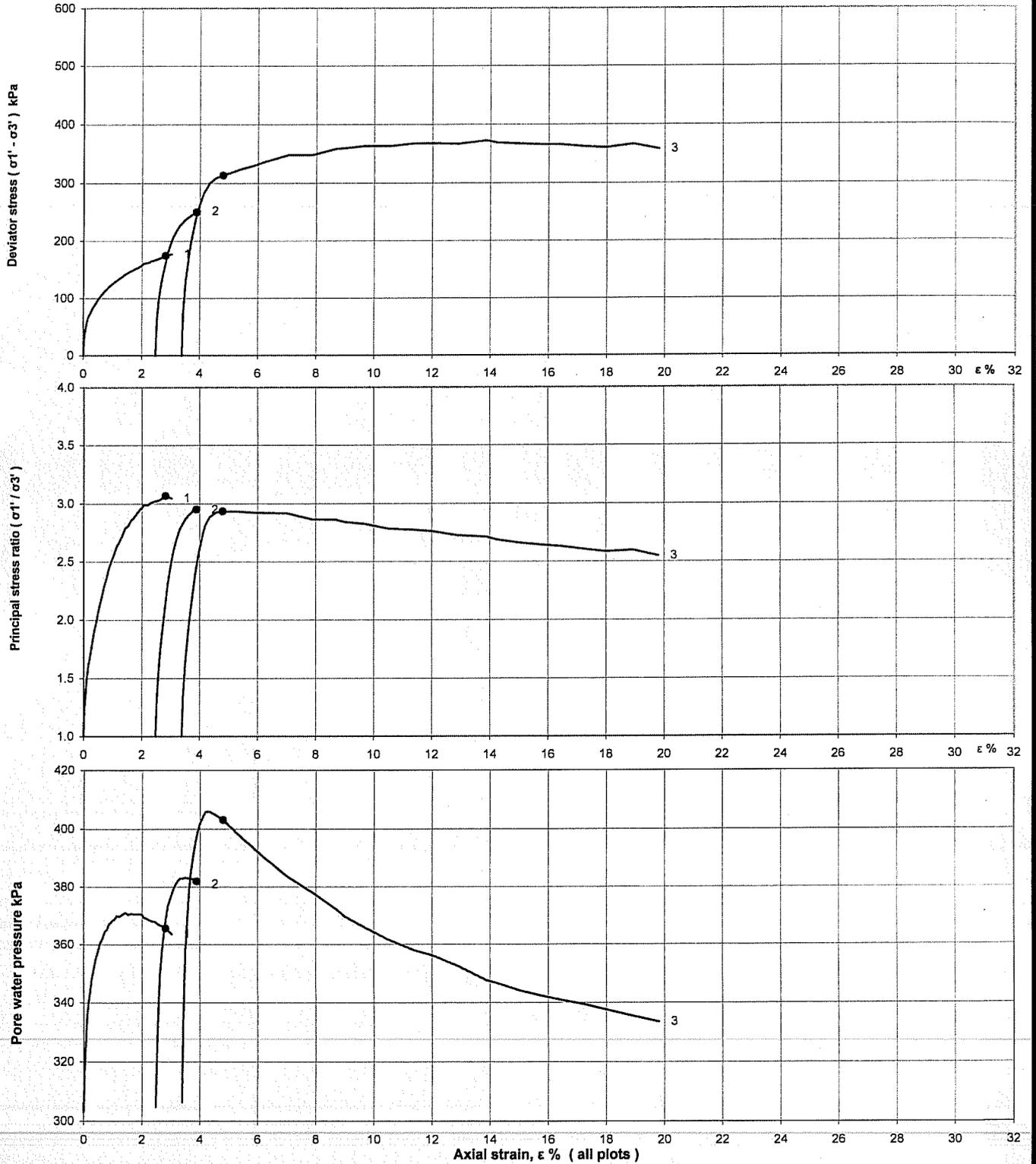
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45		
			No	28	Type	U
			ID			
			Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



Printed: 14/11/2011 16:10

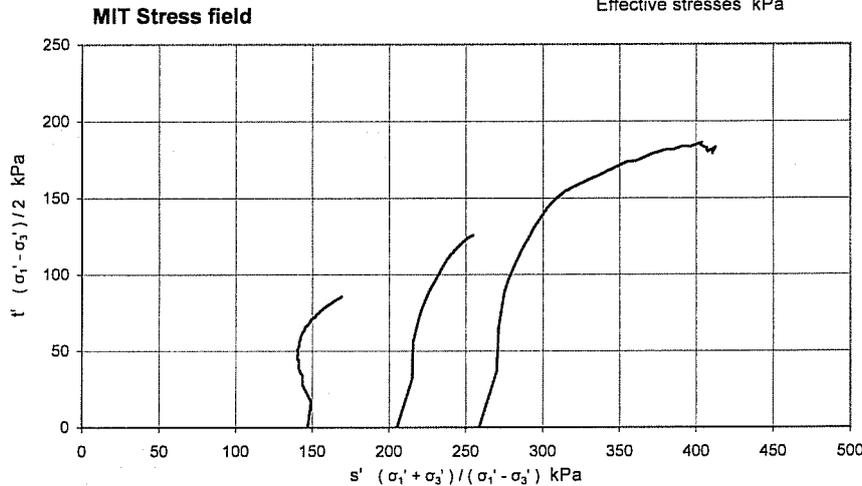
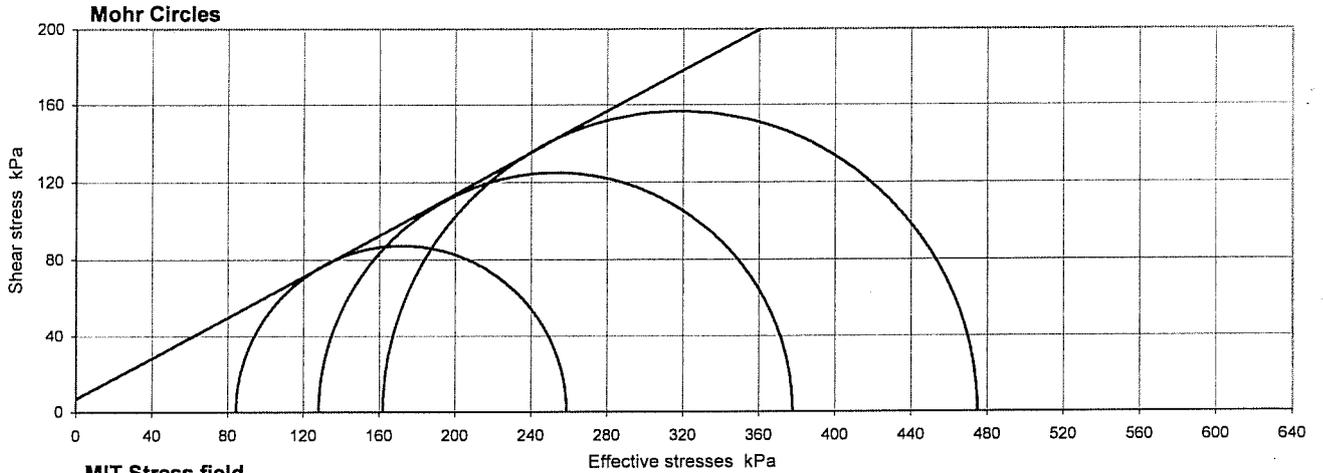
Figure

**CUM 7**

sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	10.00-10.45		
			No	28	Type	U
			ID			
			Spec Ref			

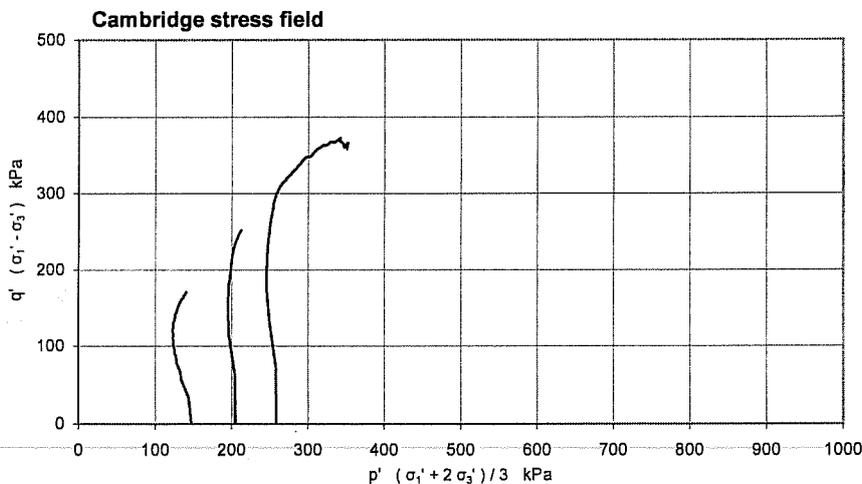


**Compression stages**

Stage	1	2	3	
Cell pressure	450	510	565	kPa
Initial pwp	303	305	306	kPa
Initial $\sigma_3'$	147	205	259	kPa
Rate of strain	0.43	0.43	0.43	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.82	3.89	4.80	%
$(\sigma_1' / \sigma_3')_f$	3.066	2.950	2.933	
$(\sigma_1' - \sigma_3')_f$	174.4	249.7	312.9	kPa
$u_f$	366	382	403	kPa
$\sigma_3'_f$	84	128	162	kPa
$\sigma_1'_f$	259	378	475	kPa
$A_f$	0.36	0.31	0.31	
Time to failure	6.6	9.1	11.2	hrs



**Shear Strength Parameters**

at peak stress ratio

Parameter	Unit	Linear regression	
		Value	Manual re-assessment
$c'$	kPa	6.9	-
$\phi'$	degrees	28.1	-

Notes : Deviator stresses corrected for area change, vertical side drains and 0.9 mm thick rubber membrane(s)

Mode of failure



Ref

SLR8.1  
Rev 85  
May 09



Printed: 14/11/2011 16:10

Figure

**CUM 7**  
sheet 3 of 3

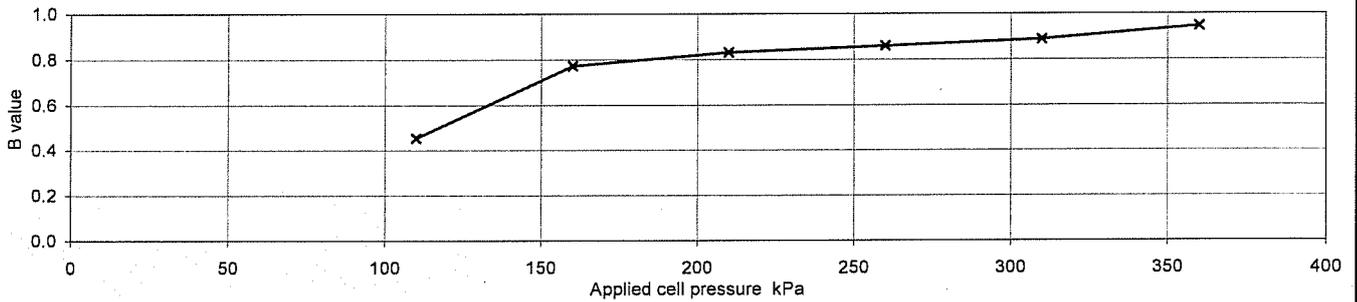
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH4
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45
			No	16
			Type	U
			ID	
			Spec Ref	

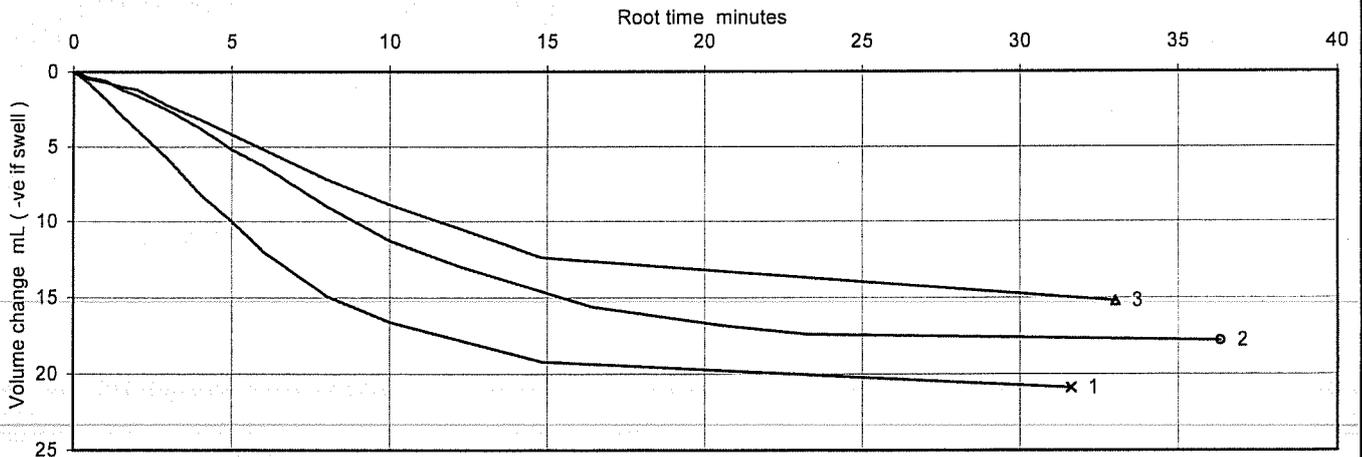
Specimen Details		
Initial		
Length	mm	204.09
Diameter	mm	102.86
Bulk Density	Mg/m <sup>3</sup>	1.96
Water Content	%	23
Dry density	Mg/m <sup>3</sup>	1.60
After test		
Bulk Density	Mg/m <sup>3</sup>	2.02
Water Content	%	23
Dry density	Mg/m <sup>3</sup>	1.65

Soil Description	Firm greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details		Method of Saturation
		Increments of cell and back pressure
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	360
Final pore water pressure	kPa	300
Final B Value		0.95



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		400	420	440	kPa
	Back Pressure applied		350	350	350	kPa
	Effective Pressure		50	70	90	kPa
	Pore pressure at start of consolidation		385	393	401	kPa
	Pore pressure at end of consolidation		351	350	352	kPa
	Pore pressure dissipation at end of consolidation		99	99	97	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	2.01	0.98	0.83	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.36	0.25	0.19	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	2.2E-10	7.5E-11	4.8E-11	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:15/11/2011 15:59

Figure

**CUM 8**

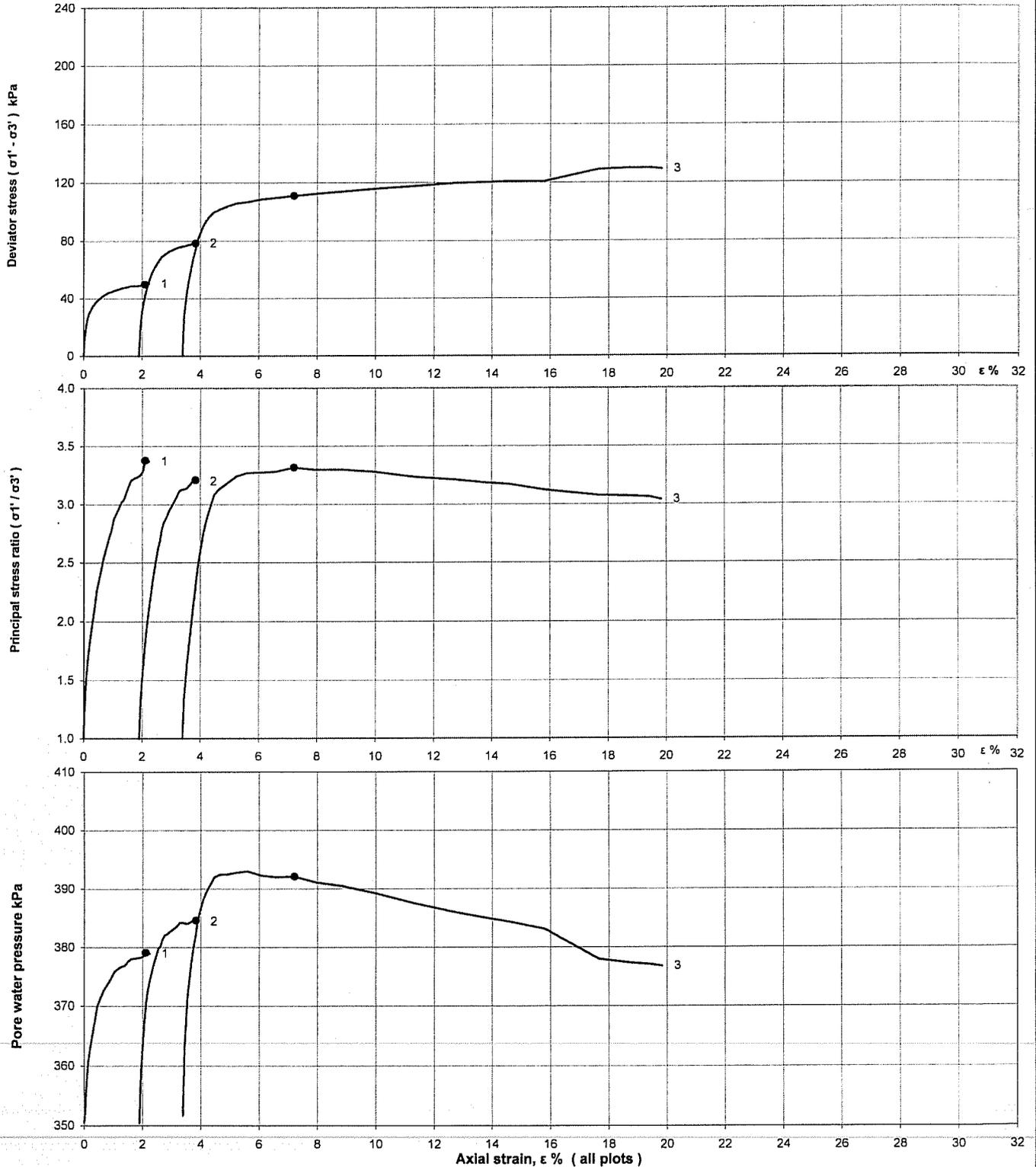
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH4		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45		
			No	16	Type	U
			ID			
			Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



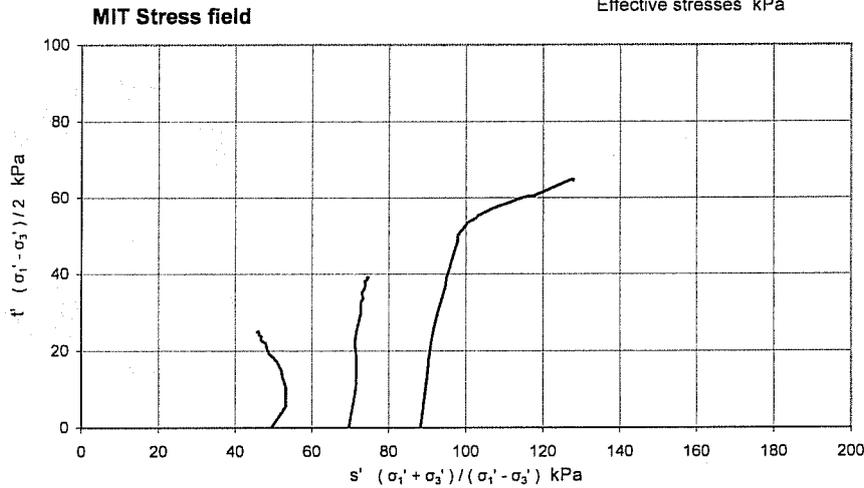
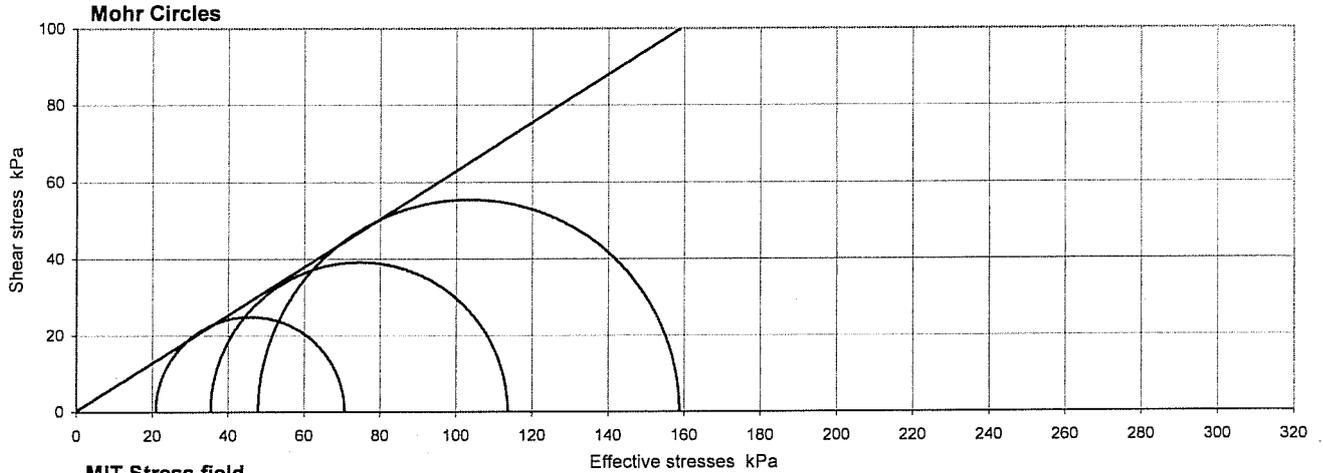
Printed: 15/11/2011 15:59

Figure

**CUM 8**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH4		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45		
			No	16	Type	U
			ID			
			Spec Ref			

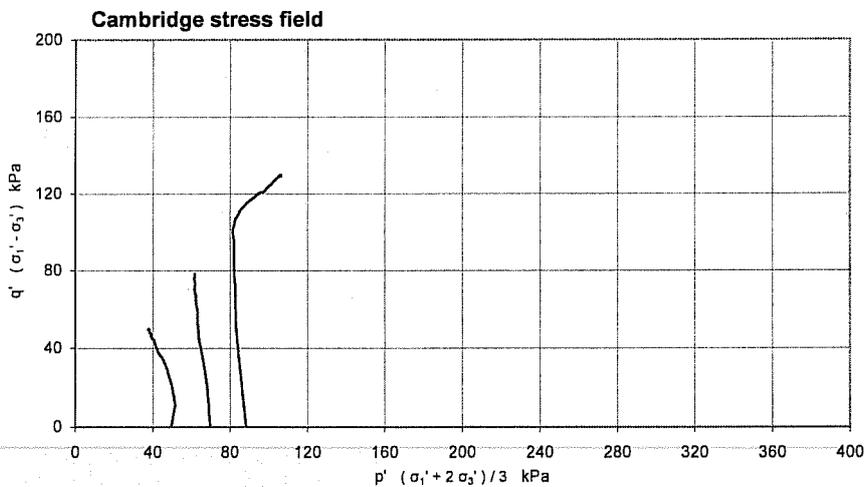


**Compression stages**

Stage	1	2	3	
Cell pressure	400	420	440	kPa
Initial pwp	351	350	352	kPa
Initial $\sigma_3'$	50	70	88	kPa
Rate of strain	1.51	1.51	1.51	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.10	3.84	7.22	%
$(\sigma_1' / \sigma_3')_f$	3.378	3.209	3.315	
$(\sigma_1' - \sigma_3')_f$	49.7	78.2	110.9	kPa
$u_f$	379	385	392	kPa
$\sigma_3' f$	21	35	48	kPa
$\sigma_1' f$	71	114	159	kPa
$A_f$	0.58	0.44	0.36	
Time to failure	1.4	2.5	4.8	hrs



**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	0.2
$\phi'$	degrees	32.1
		Manual re-assessment
$c'$	kPa	-
$\phi'$	degrees	-

Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)

Mode of failure



Ref

SLR8.1  
Rev 85  
May 09



Printed:15/11/2011 15:59

Figure

**CUM 8**  
sheet 3 of 3

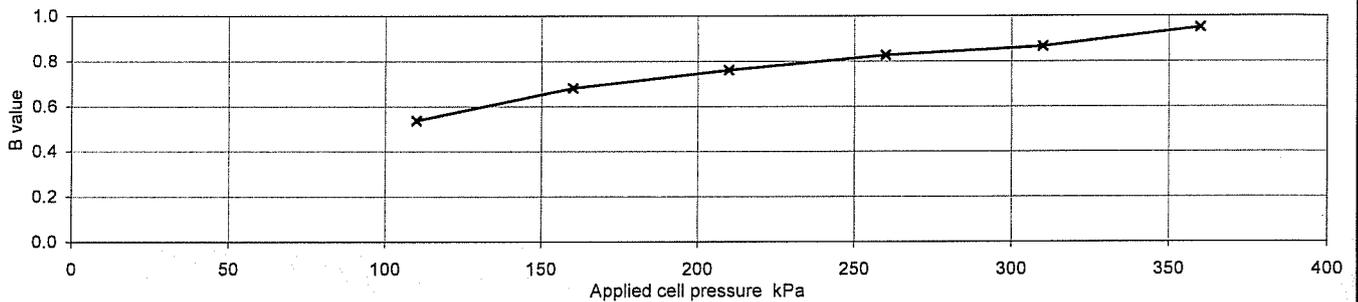
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45		
			No	14	Type	U
			ID			
			Spec Ref			

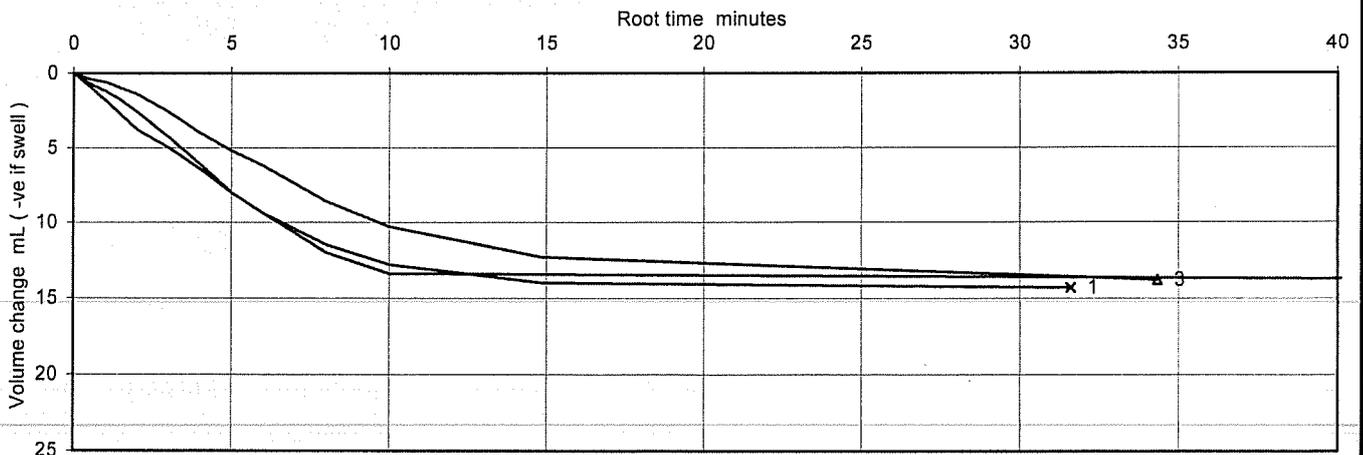
Specimen Details			
Initial			
Length	mm		174.16
Diameter	mm		103.45
Bulk Density	Mg/m <sup>3</sup>		1.87
Water Content	%		22
Dry density	Mg/m <sup>3</sup>		1.53
After test			
Bulk Density	Mg/m <sup>3</sup>		2.02
Water Content	%		23
Dry density	Mg/m <sup>3</sup>		1.63

Soil Description	Stiff greyish brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details		Method of Saturation	
		Increments of cell and back pressure	
Cell pressure increments	kPa		50
Differential Pressure	kPa		10
Final Cell Pressure	kPa		360
Final pore water pressure	kPa		300
Final B Value			0.95



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		400	420	440	kPa
	Back Pressure applied		350	350	350	kPa
	Effective Pressure		50	70	90	kPa
	Pore pressure at start of consolidation		390	399	409	kPa
	Pore pressure at end of consolidation		350	350	350	kPa
	Pore pressure dissipation at end of consolidation		100	99	99	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	3.27	3.88	1.75	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.25	0.21	0.17	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	2.5E-10	2.5E-10	9.3E-11	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 11:55

Figure

**CUM 9**

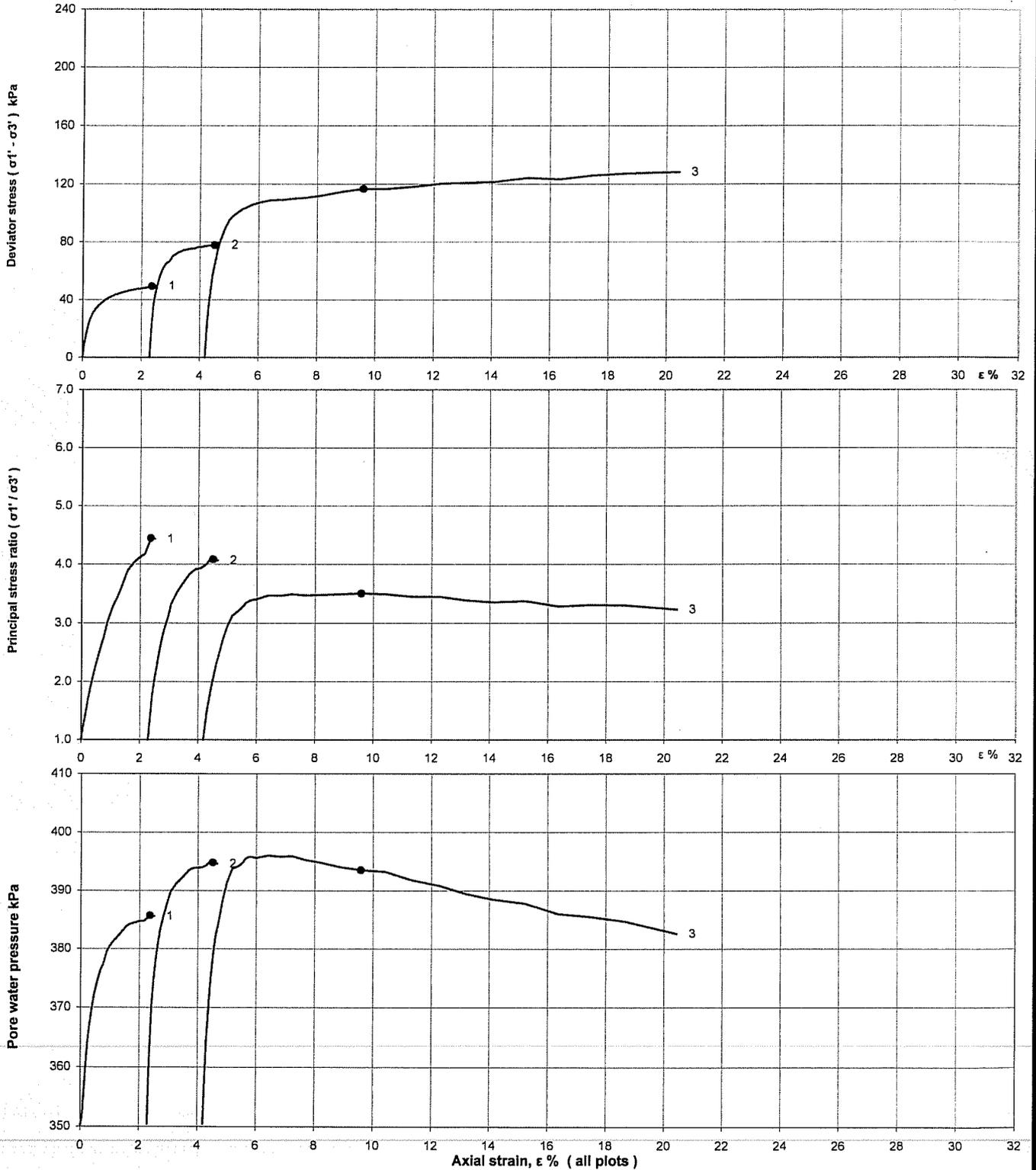
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH5	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45	
		No	14	Type	U
		ID			
		Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



Printed: 11/11/2011 11:55

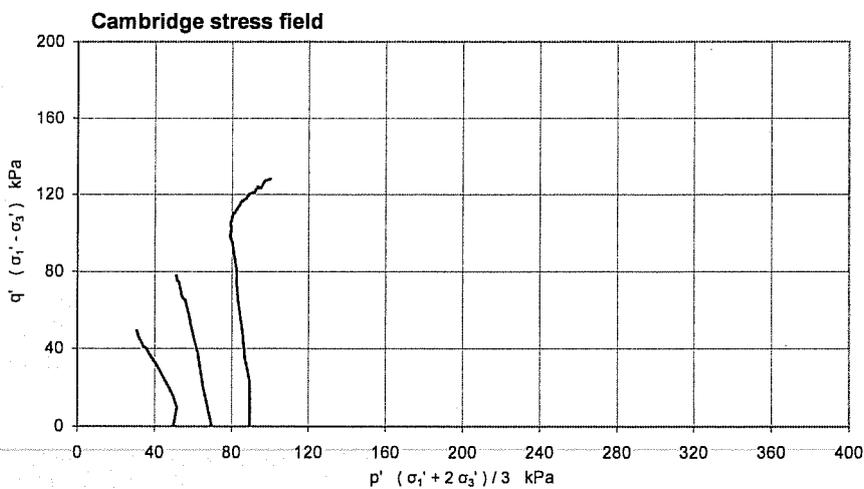
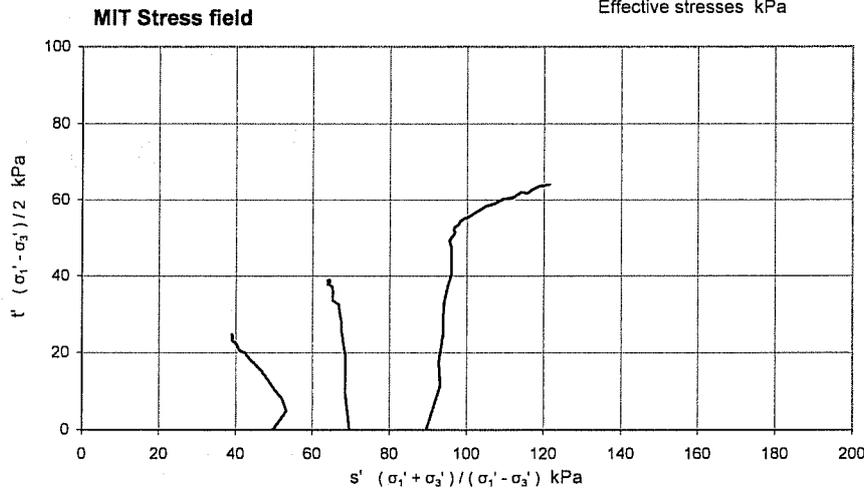
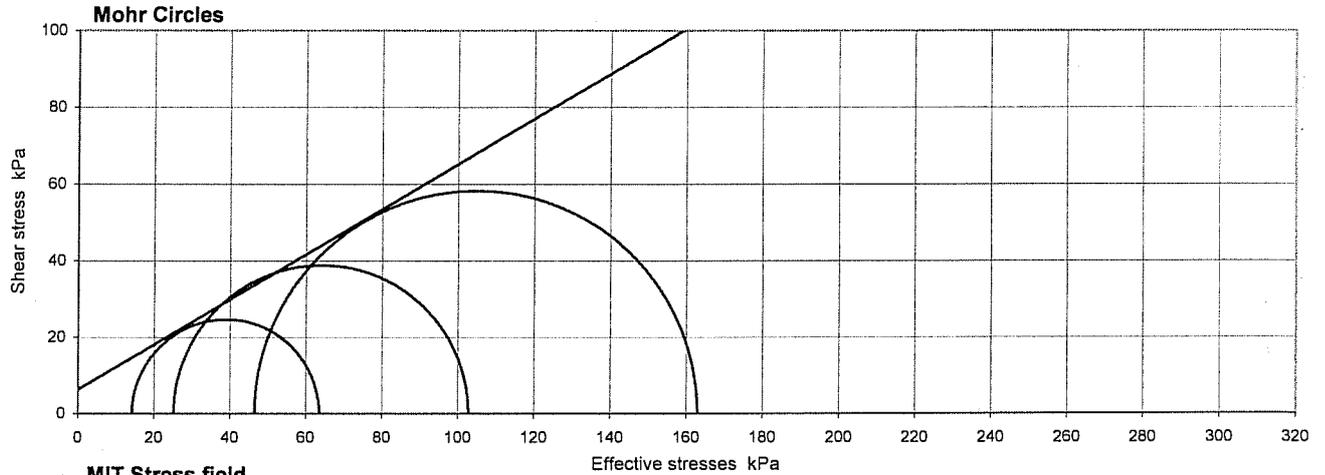
Figure

**CUM 9**

sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH5		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45		
			No	14	Type	U
			ID			
		Spec Ref				



**Compression stages**

Stage	1	2	3	
Cell pressure	400	420	440	kPa
Initial pwp	350	350	350	kPa
Initial $\sigma_3'$	50	70	90	kPa
Rate of strain	1.93	1.93	1.93	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	1	2	3	
Axial strain	2.37	4.51	9.59	%
$(\sigma_1' / \sigma_3')_f$	4.442	4.078	3.505	
$(\sigma_1' - \sigma_3')_f$	49.2	77.6	116.5	kPa
$u_f$	386	395	394	kPa
$\sigma_3' f$	14	25	47	kPa
$\sigma_1' f$	64	103	163	kPa
$A_f$	0.72	0.57	0.37	
Time to failure	1.2	2.3	5.0	hrs

**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	6.3
$\phi'$	degrees	30.5
		Manual re-assessment
$c'$	kPa	-
$\phi'$	degrees	-

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.45 mm thick rubber membrane(s)  
Test specimen contains natural break.

Ref

SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 11:55

Figure

**CUM 9**  
sheet 3 of 3

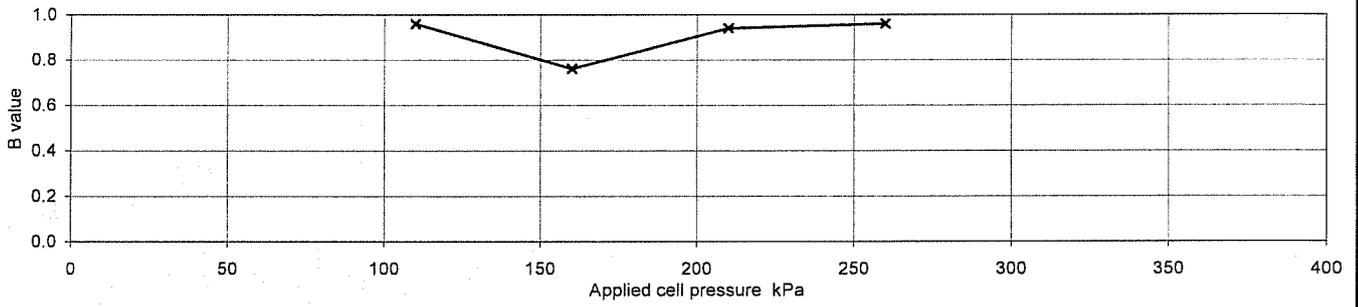
**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45		
			No	14	Type	U
			ID			
			Spec Ref			

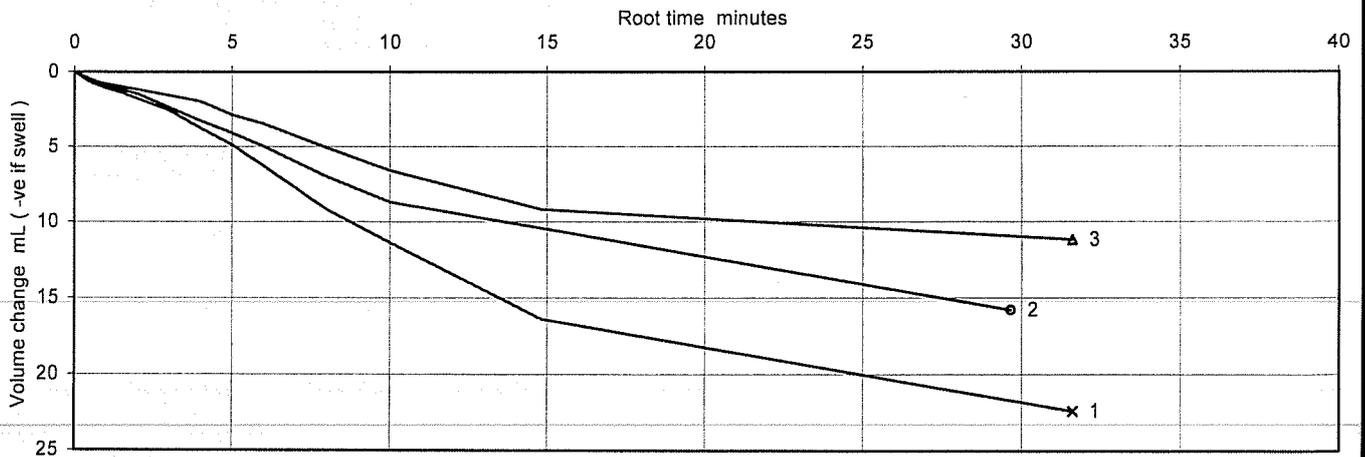
Specimen Details		
Initial		
Length	mm	202.63
Diameter	mm	103.04
Bulk Density	Mg/m <sup>3</sup>	2.08
Water Content	%	21
Dry density	Mg/m <sup>3</sup>	1.72
After test		
Bulk Density	Mg/m <sup>3</sup>	2.13
Water Content	%	20
Dry density	Mg/m <sup>3</sup>	1.78

Soil Description	Firm brownish grey slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	UNDISTURBED

Saturation Details	Method of Saturation	
	Increments of cell and back pressure	
Cell pressure increments	kPa	50
Differential Pressure	kPa	10
Final Cell Pressure	kPa	260
Final pore water pressure	kPa	200
Final B Value		0.96



Consolidation Details	Drainage Conditions		From radial boundary and one end			
	Stage No.		1	2	3	
	Cell Pressure applied		350	370	390	kPa
	Back Pressure applied		300	300	300	kPa
	Effective Pressure		50	70	90	kPa
	Pore pressure at start of consolidation		334	348	350	kPa
	Pore pressure at end of consolidation		300	300	300	kPa
	Pore pressure dissipation at end of consolidation		100	100	100	%
Consolidation parameters ( see note to BS1377 : pt 8, clause 6.3.4 )	Coefficient of Consolidation	C <sub>vi</sub>	0.70	0.72	0.86	m <sup>2</sup> /year
	Coefficient of Compressibility	M <sub>vi</sub>	0.39	0.20	0.14	m <sup>2</sup> /MN
	Coefficient of Permeability ( calculated )	k <sub>vi</sub>	8.5E-11	4.4E-11	3.6E-11	m/s



Ref  
SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 12:02

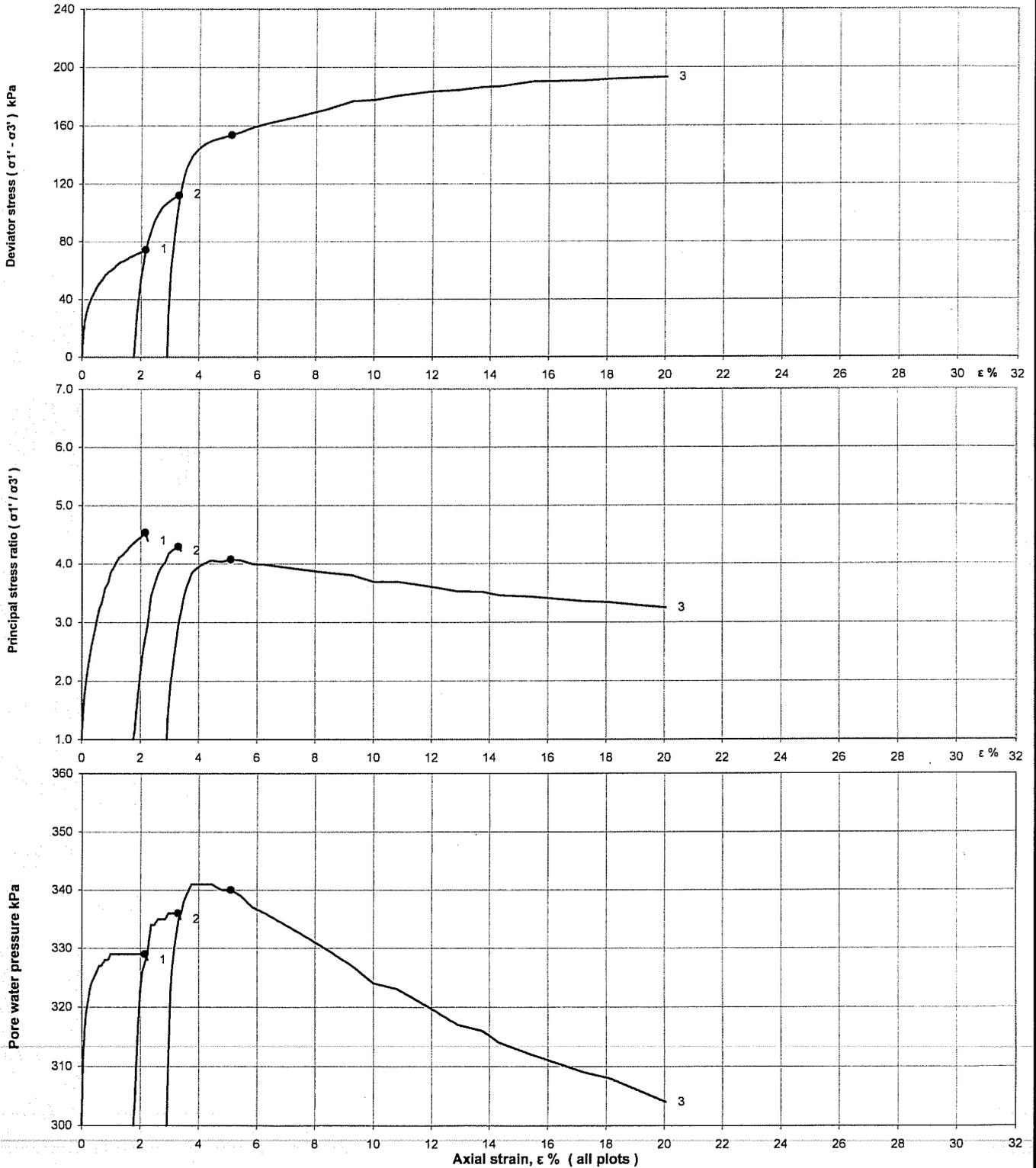
Figure  
**CUM 10**  
sheet 1 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH6		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45		
			No	14	Type	U
			ID			
			Spec Ref			

**Shearing stages - graphical data**

o failure points



Ref

SLR8.1  
Rev 85  
May 09



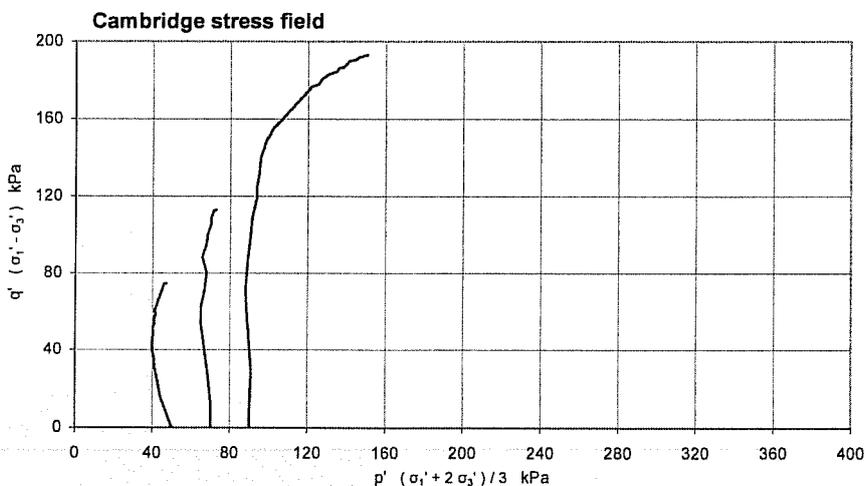
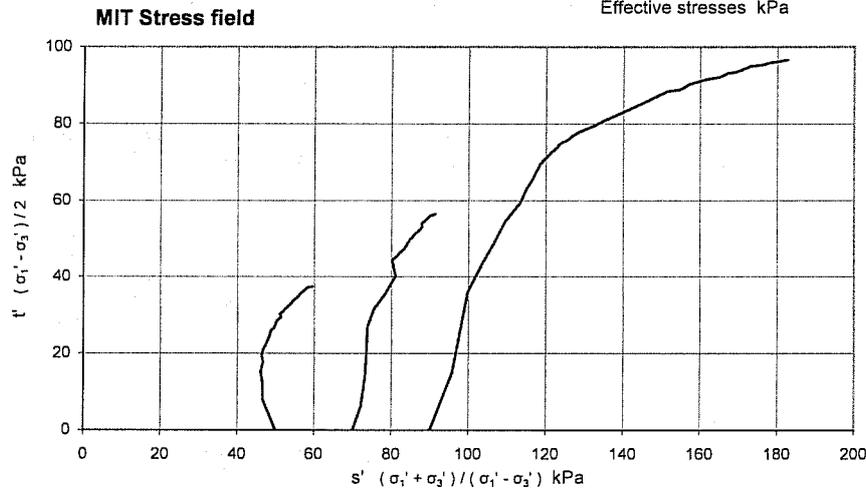
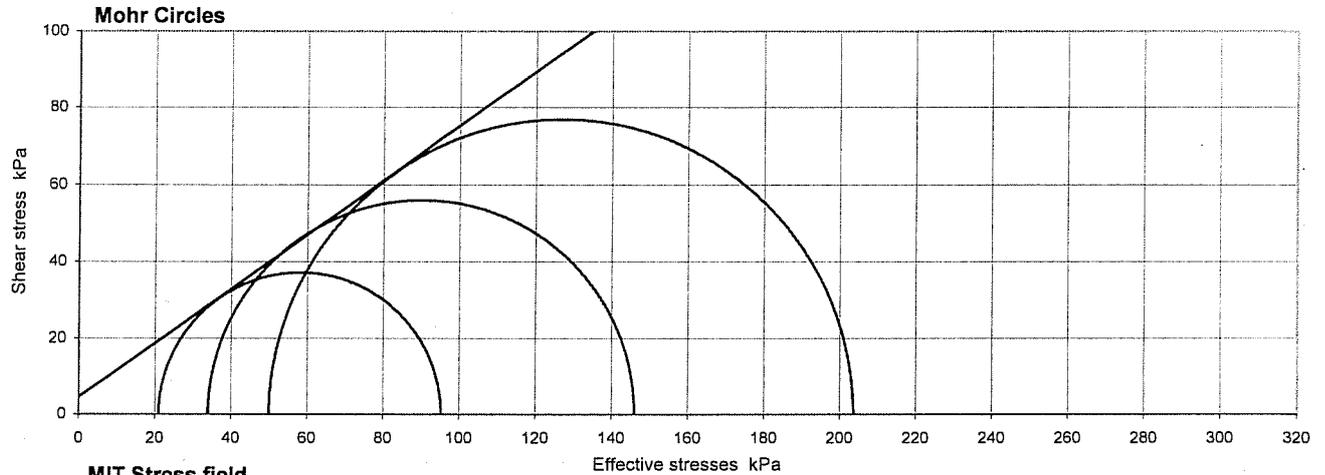
Printed: 11/11/2011 12:02

Figure

**CUM 10**  
sheet 2 of 3

**Consolidated Undrained Triaxial Compression test with Measurement of Pore Water Pressure  
( BS1377 : Part 8 : 1990 ) - Multistage test on a single specimen**

Project No	A1077-11	Sample Details:	Hole No	BH6	
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.00-3.45	
		No	14	Type	U
		ID			
		Spec Ref			



**Compression stages**

Stage	1	2	3	
Cell pressure	350	370	390	kPa
Initial pwp	300	300	300	kPa
Initial $\sigma_3'$	50	70	90	kPa
Rate of strain	0.52	0.52	0.52	%/hr

**Failure conditions**

Criterion	Maximum effective principal stress ratio			
	2.16	3.30	5.10	
Axial strain	2.16	3.30	5.10	%
$(\sigma_1' / \sigma_3')_f$	4.535	4.295	4.073	
$(\sigma_1' - \sigma_3')_f$	74.2	112.0	153.6	kPa
$u_f$	329	336	340	kPa
$\sigma_3'_{f1}$	21	34	50	kPa
$\sigma_1'_{f1}$	95	146	204	kPa
$A_f$	0.39	0.32	0.26	
Time to failure	4.1	6.3	9.8	hrs

**Shear Strength Parameters**

at peak stress ratio

		Linear regression
$c'$	kPa	4.6
$\phi'$	degrees	35.3
		Manual re-assessment
$c'$	kPa	-
$\phi'$	degrees	-

Mode of failure



Notes : Deviator stresses corrected for area change, vertical side drains and 0.9 mm thick rubber membrane(s)

Ref

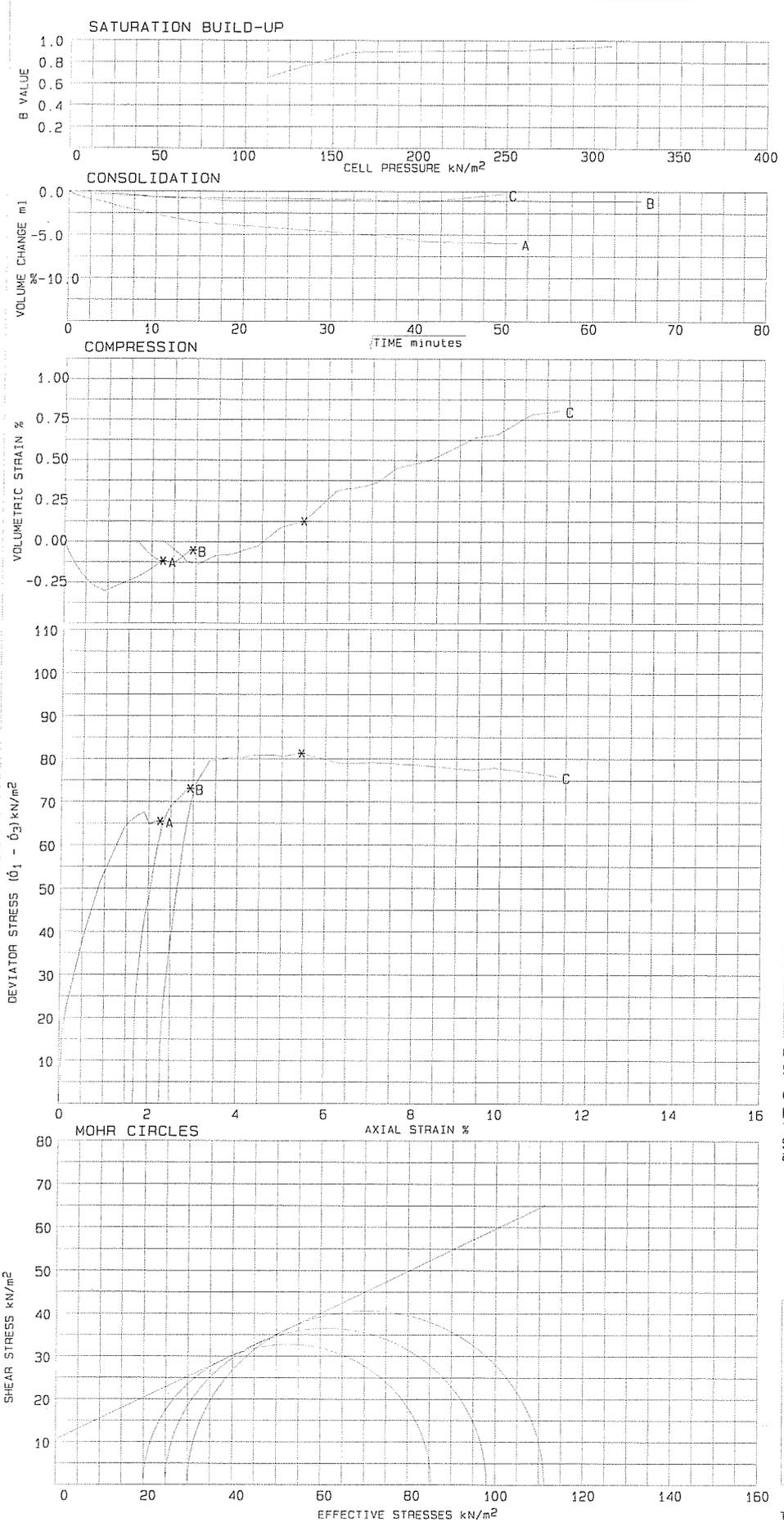
SLR8.1  
Rev 85  
May 09



Printed:11/11/2011 12:02

Figure

**CUM 10**  
sheet 3 of 3



		SPECIMEN		
		A	B	C
INITIAL	Density Mg/m <sup>3</sup>	2.11	2.10	2.10
	Moisture %	19	18	18
	Dry Density Mg/m <sup>3</sup>	1.78	1.77	1.77
AFTER CONSOLIDATION	Density Mg/m <sup>3</sup>	2.10	2.10	2.09
	Moisture %	18	18	18
	Dry Density Mg/m <sup>3</sup>	1.77	1.77	1.77
AFTER TEST	Density Mg/m <sup>3</sup>	2.10	2.10	2.08
	Moisture %	18	18	19
	Dry Density Mg/m <sup>3</sup>	1.77	1.77	1.75
SATURATION	Initial pwp	0	0	0
	Saturated pwp	249	249	249
	Final pwp pressure	260	260	260
	B value	0.95	0.95	0.95
CONSOLIDATION STAGE	Cell Pressure	320	325	330
	Back Pressure	300	300	300
	Initial pwp	309	303	305
	Final pwp	300	300	300
CONSOLIDATION PARAMETERS	Cv <sub>3</sub> m <sup>2</sup> /year	0.43	0.64	0.67
	mv <sub>3</sub> m <sup>2</sup> /MN	0.42	0.21	0.03
COMPRESSION STAGE	Cell Pressure	320	325	330
	Back pressure	300	300	300
	σ <sub>3</sub> '	20	25	30
	Rate of % per strain hour	0.13	0.13	0.13
FAILURE CONDITIONS	Axial Strain %	2.3	2.9	5.5
	Volumetric Strain %	-0.1	-0.1	0.1
	(σ <sub>1</sub> - σ <sub>3</sub> ) <sub>f</sub>	65	73	81
	σ <sub>1</sub> ' <sub>f</sub>	85	98	111
	Time to failure hours	17.4	22.5	41.9
MODE OF FAILURE				

SHEAR STRENGTH PARAMETERS	C <sub>D</sub>	φ <sub>D</sub>
BY LINEAR REGRESSION	11 kN/m <sup>2</sup>	26 °

**NOTES**  
 Saturation by application of cell/back pressure increments of 50 kN/m<sup>2</sup> with a differential of 10 kN/m<sup>2</sup>.  
 Drainage during consolidation and shearing to top with Vertical side drains fitted  
 Stress/strain curves corrected for area change, side drains and .45 mm thick membrane

<b>SOIL DESCRIPTION</b> Firm brown slightly sandy, slightly gravelly CLAY.	
TYPE OF SPECIMEN	UNDISTURBED
SPECIMEN DIMENSIONS	102.5 mm dia x 203.9 mm long

Test carried out to BS 1377 : Part 8 : 1990 : Test 8

Consolidated Drained Triaxial Compression Test with measurement of volume change		Borehole No.	BH1	Depth	1.20-1.65m
		sample No.	U5		
SLR	Soil Mechanics	Location	SANDSEND BOREHOLES, NORTH YORKSHIRE		Loc. No.
8.8				A1077-11	Fig.

**Determination of shear strength by direct shear ( Small shearbox apparatus )  
( BS1377 : Part 7 : clause 4 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH1		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	12.50-12.95		
			Sample No	33	Type	U
			ID			
			Spec Ref			

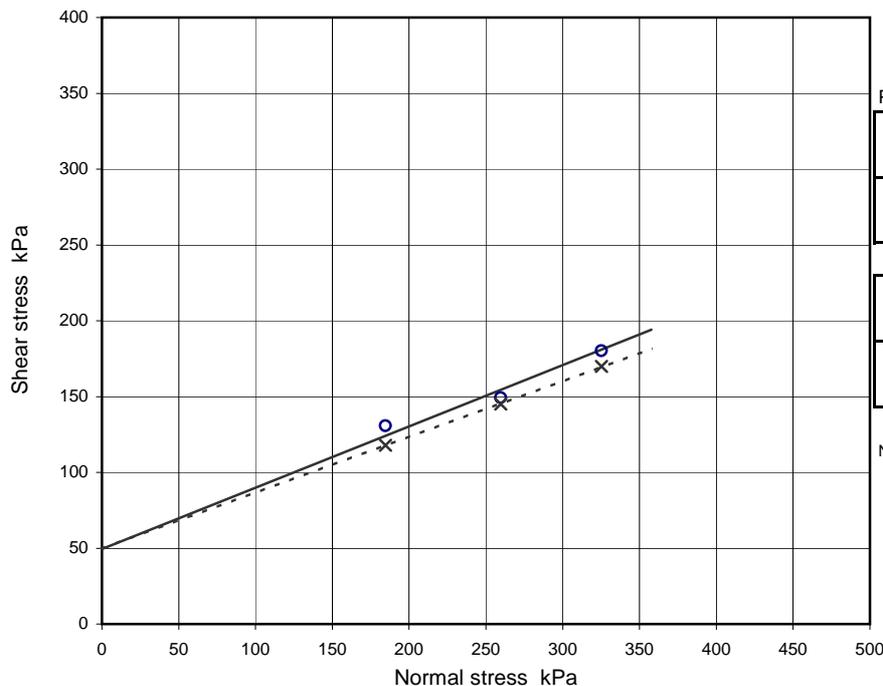
Soil Description	Firm to stiff brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square  
 Test(s) carried out in submerged condition  
 Particle density, assumed 2.65 Mg/m<sup>3</sup>

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m <sup>3</sup>	2.20	2.18	2.20			
	Water Content	%	14.5	14.6	14.9			
	Dry density	Mg/m <sup>3</sup>	1.92	1.90	1.92			
	Voids ratio		0.378	0.395	0.383			
	Degree of Saturation	%	102	98	103			
Consol <sup>1</sup>	Consolidation / Normal Stress applied	kPa	185	260	325			
	Change in height during consolidation	mm	-0.872	-1.200	-1.334			
	Voids ratio after consolidation		0.326	0.324	0.304			
Shear see note 1	Voids ratio at end of test		0.246	0.252	0.220			
	Moisture content at end of test	%	9.3	9.5	8.3			
	Saturation at end of test	%	100	100	100			

**Shearing stage**

Rate of displacement	Peak	mm/min	0.005	0.005	0.005			
	Residual	mm/min	0.023	0.023	0.023			
Peak values, (o)	Relative displacement	mm	6.04	9.50	6.30			
	Shear stress	kPa	130.7	149.2	180.2			
Residual values, (x)	No. of reversals		2	2	2			
	Relative displacement	mm	20.00	30.00	10.00			
	Shear stress	kPa	118.0	145.2	169.8			



**Shear Strength Parameters**

Peak strength, (o)		Regression	Manual
c'	kPa	( 64 )	50
Ø'	degrees	( 19½ )	22

Residual strength, (x)		Regression	Manual
c' <sub>R</sub>	kPa	50	-
Ø' <sub>R</sub>	degrees	20	-

Notes :

1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6 using δH calculated from consolidation and shear stages

Ref

SLR7.4  
Rev 85  
Oct 10



Printed:18/11/2011 16:29

Figure

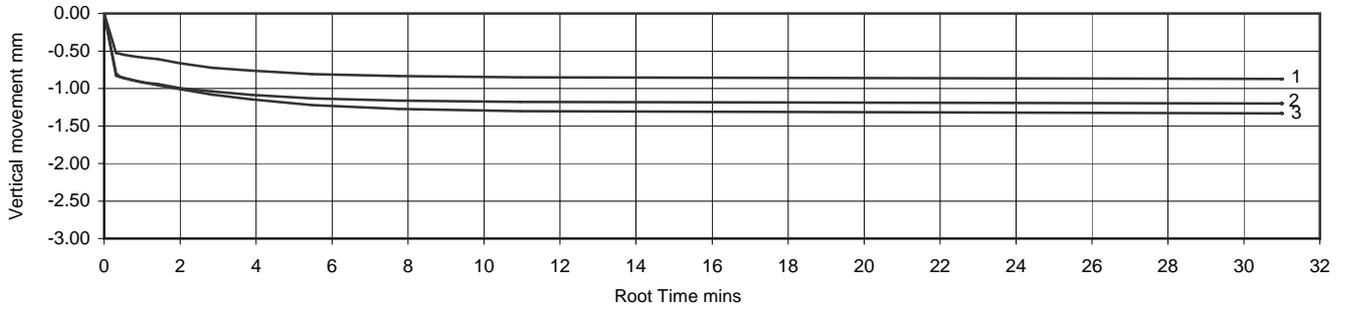
**SSB 1**

sheet 1 of 2

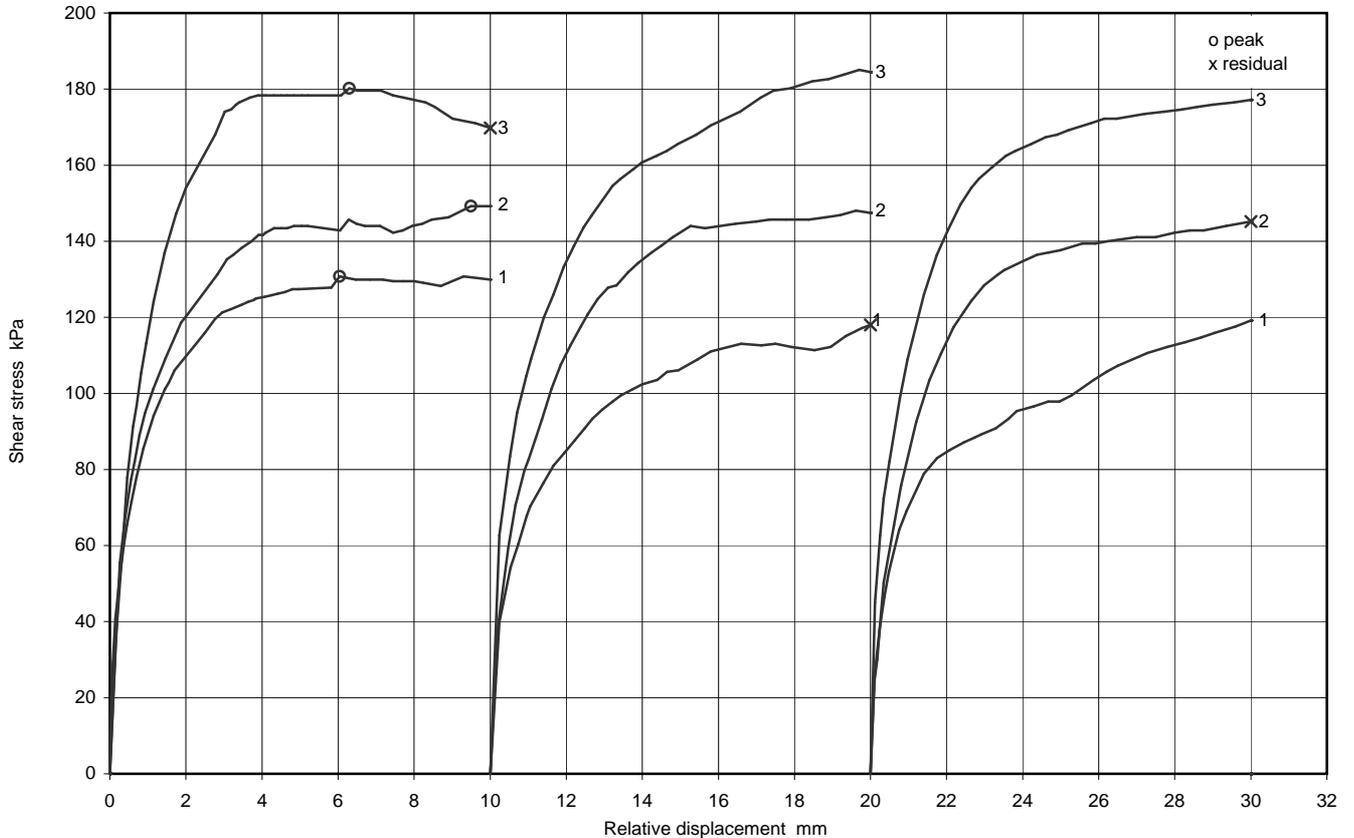
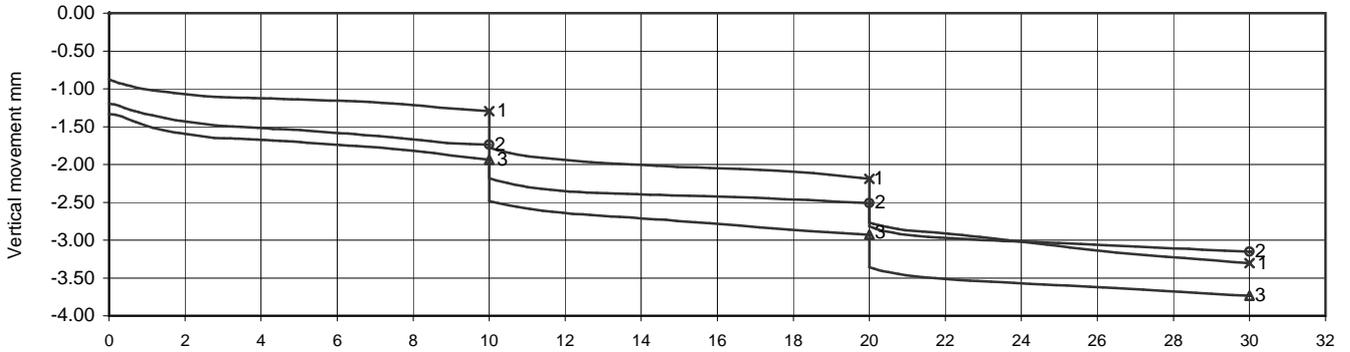
**Determination of shear strength by direct shear ( Small shearbox apparatus )  
( BS1377 : Part 7 : clause 4 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH1			
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	12.50-12.95			
			Sample No	33	Type	U	
			ID				
			Spec Ref				

**Consolidation stage(s)**



**Shearing stage(s)**



Ref

SLR7.4  
Rev 85  
Oct 10



Printed:18/11/2011 16:29

Figure

**SSB 1**

sheet 2 of 2

**Determination of shear strength by direct shear ( Small shearbox apparatus )  
( BS1377 : Part 7 : clause 4 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00-5.45		
			Sample No	12	Type	U
			ID			
			Spec Ref			

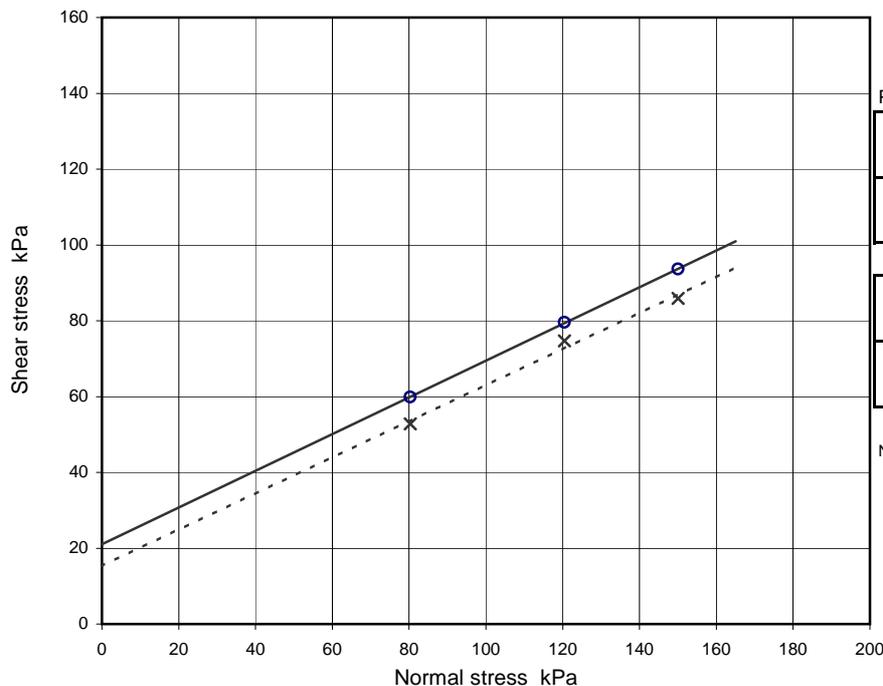
Soil Description	Firm brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square  
 Test(s) carried out in submerged condition  
 Particle density, assumed 2.65 Mg/m<sup>3</sup>

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m <sup>3</sup>	2.17	2.17	2.19			
	Water Content	%	14.2	14.2	13.9			
	Dry density	Mg/m <sup>3</sup>	1.90	1.90	1.92			
	Voids ratio		0.395	0.396	0.377			
	Degree of Saturation	%	95	95	98			
Consol <sup>1</sup>	Consolidation / Normal Stress applied	kPa	80	120	150			
	Change in height during consolidation	mm	-0.436	-0.470	-0.582			
	Voids ratio after consolidation		0.369	0.368	0.343			
Shear see note 1	Voids ratio at end of test		0.303	0.354	0.309			
	Moisture content at end of test	%	11.4	13.4	11.7			
	Saturation at end of test	%	100	100	100			

**Shearing stage**

Rate of displacement	Peak	mm/min	0.009	0.009	0.009			
	Residual	mm/min	0.046	0.046	0.046			
Peak values, (o)	Relative displacement	mm	10.00	6.05	5.85			
	Shear stress	kPa	59.9	79.6	93.6			
Residual values, (x)	No. of reversals		2	2	2			
	Relative displacement	mm	30.00	28.43	28.75			
	Shear stress	kPa	52.8	74.7	85.9			



**Shear Strength Parameters**

Peak strength, (o)		Regression	Manual
c'	kPa	21	-
Ø'	degrees	26	-

Residual strength, (x)		Regression	Manual
c' <sub>R</sub>	kPa	15	-
Ø' <sub>R</sub>	degrees	25½	-

Notes :

1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6 using δH calculated from consolidation and shear stages

Ref

SLR7.4  
Rev 85  
Oct 10



Printed:18/11/2011 16:30

Figure

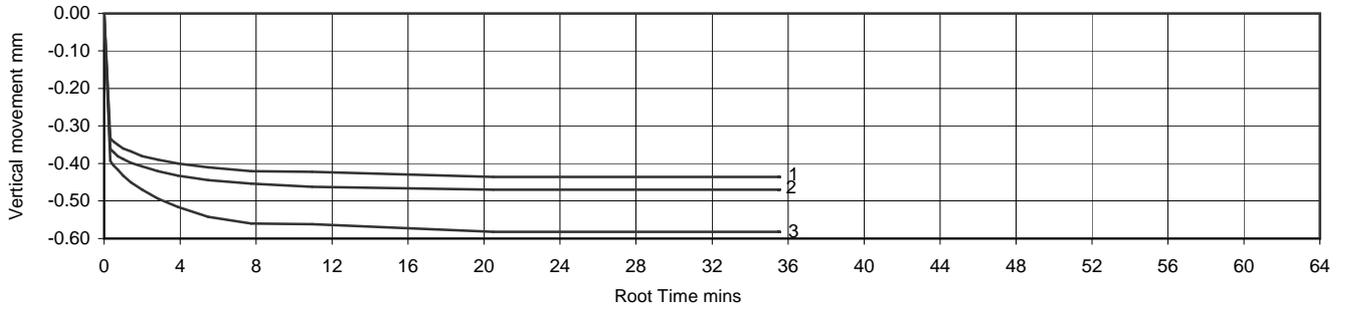
**SSB 2**

sheet 1 of 2

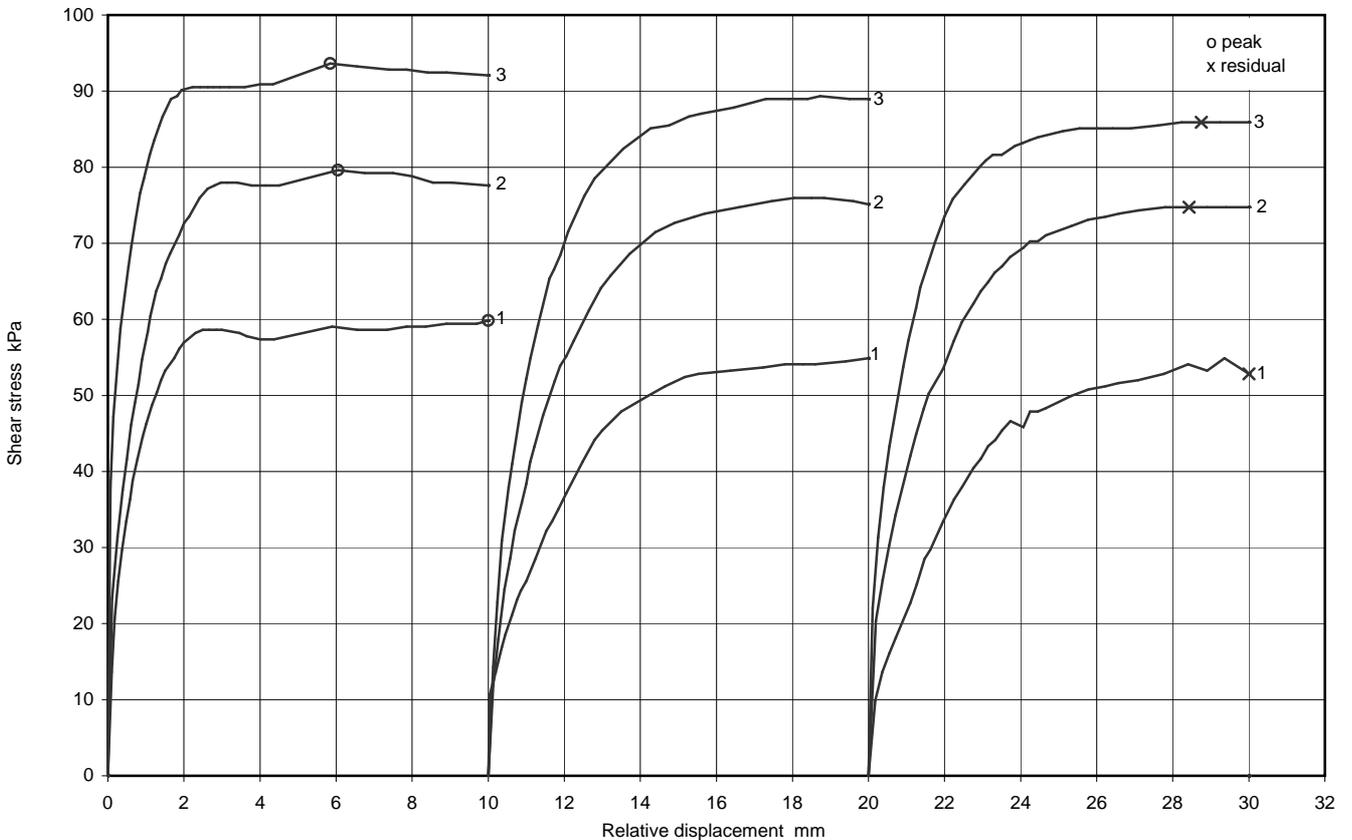
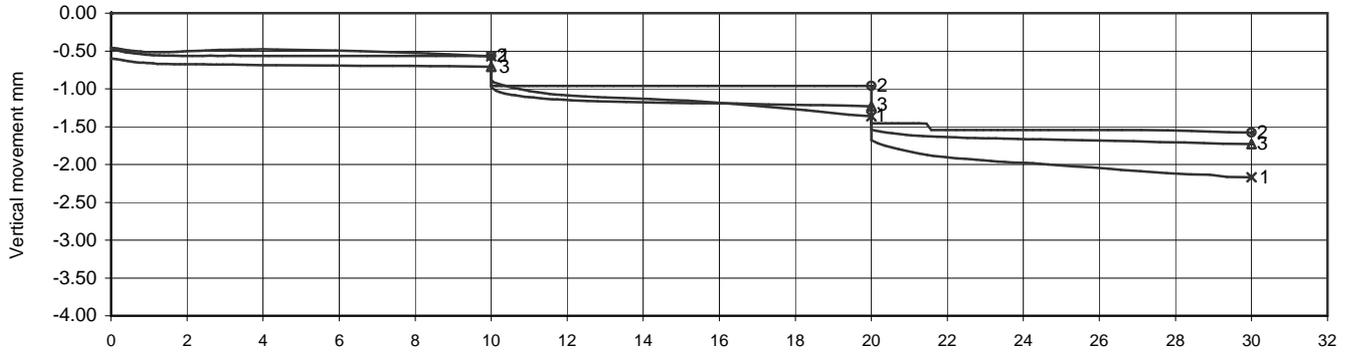
## Determination of shear strength by direct shear ( Small shearbox apparatus ) ( BS1377 : Part 7 : clause 4 : 1990 )

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.00-5.45		
			Sample No	12	Type	U
			ID			
			Spec Ref			

### Consolidation stage(s)



### Shearing stage(s)



Ref

SLR7.4  
Rev 85  
Oct 10



Soil Mechanics



Printed: 18/11/2011 16:30

Figure

**SSB 2**

sheet 2 of 2

**Determination of shear strength by direct shear ( Small shearbox apparatus )  
( BS1377 : Part 7 : clause 4 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.00-19.45		
			Sample No	43	Type	U
			ID			
			Spec Ref			

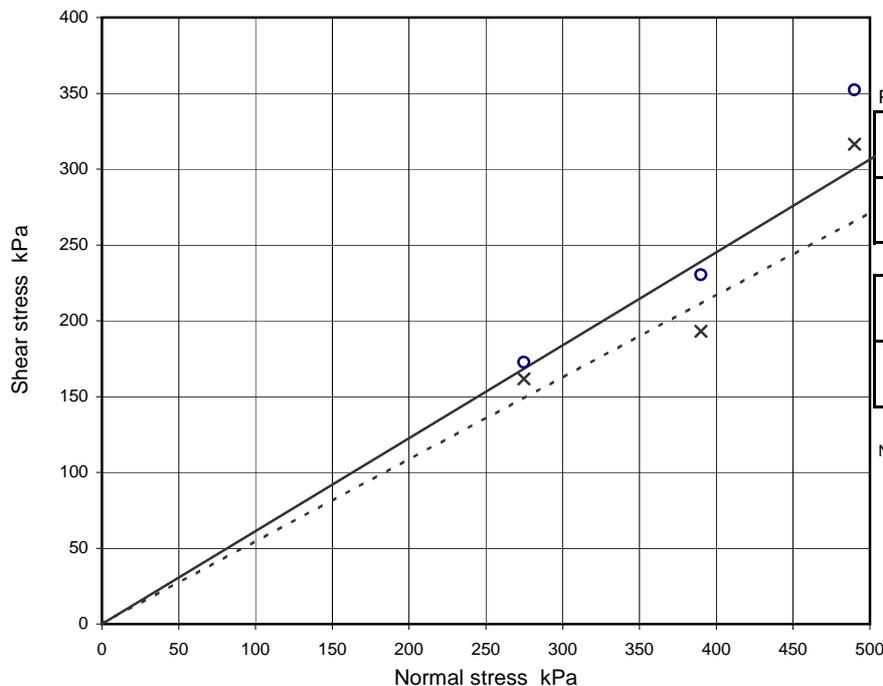
Soil Description	Firm brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square  
 Test(s) carried out in submerged condition  
 Particle density, assumed 2.65 Mg/m<sup>3</sup>

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m <sup>3</sup>	2.18	2.20	2.18			
	Water Content	%	14.5	15.4	16.1			
	Dry density	Mg/m <sup>3</sup>	1.91	1.90	1.88			
	Voids ratio		0.389	0.391	0.411			
	Degree of Saturation	%	99	104	104			
Consol <sup>n</sup>	Consolidation / Normal Stress applied	kPa	275	390	490			
	Change in height during consolidation	mm	-1.028	-1.748	-1.808			
	Voids ratio after consolidation		0.328	0.287	0.302			
Shear see note 1	Voids ratio at end of test		0.272	0.183	0.175			
	Moisture content at end of test	%	10.3	6.9	6.6			
	Saturation at end of test	%	100	100	100			

**Shearing stage**

Rate of displacement	Peak	mm/min	0.003	0.003	0.003			
	Residual	mm/min	0.013	0.013	0.013			
Peak values, (o)	Relative displacement	mm	5.93	5.68	6.30			
	Shear stress	kPa	172.7	230.4	352.3			
Residual values, (x)	No. of reversals		2	2	2			
	Relative displacement	mm	30.60	20.00	10.00			
	Shear stress	kPa	161.6	193.1	316.4			



**Shear Strength Parameters**

Peak strength, (o)		Regression	Manual
c'	kPa	( -66.1 )	0.0
Ø'	degrees	( 39½ )	31½

Residual strength, (x)		Regression	Manual
c' <sub>R</sub>	kPa	( -48.7 )	0.0
Ø' <sub>R</sub>	degrees	( 35½ )	28½

Notes :

- 1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6 using δH calculated from consolidation and shear stages
- 2 2 pieces of medium gravel found in specimen 3 after test.
- 3 Manual interpretation carried out for results for specimens 1 and 2 only, assuming c' and c<sub>R</sub> = 0

Ref

SLR7.4  
Rev 85  
Oct 10



Printed:08/12/2011 12:47

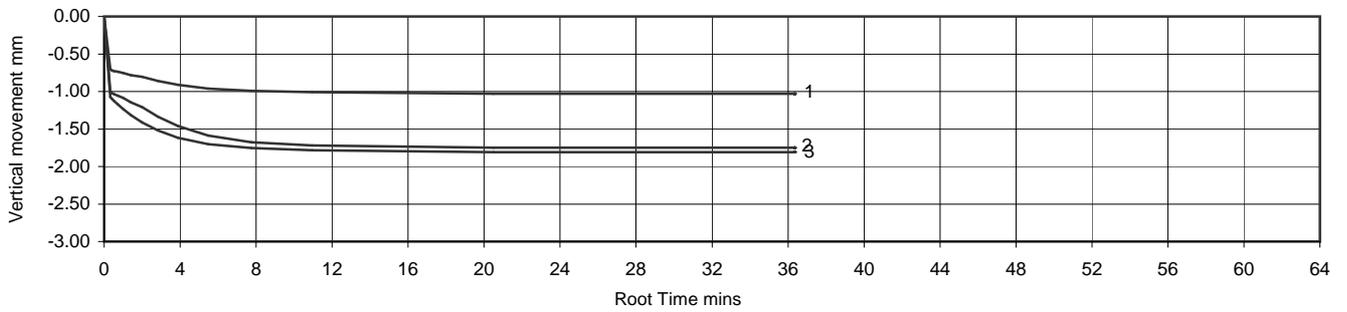
Figure

**SSB 3**

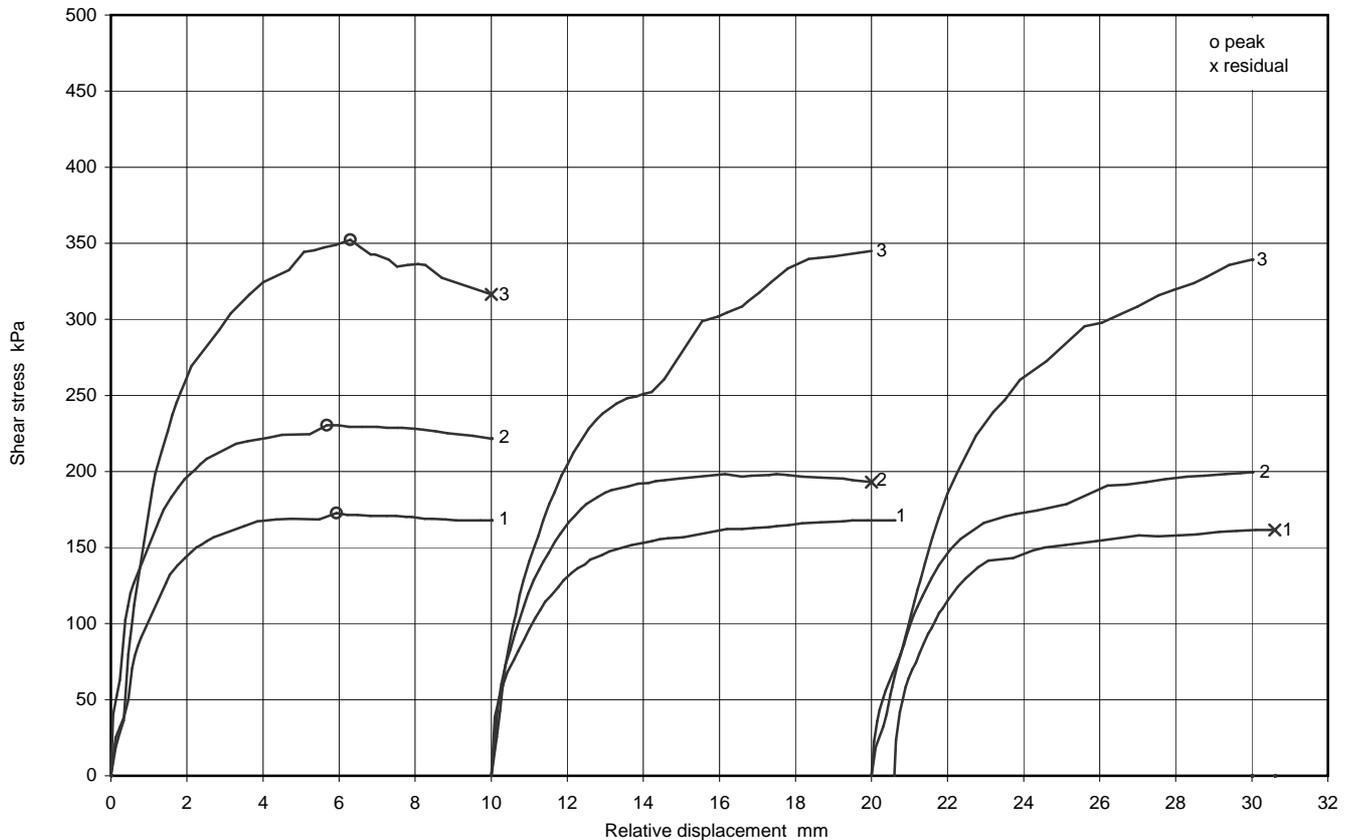
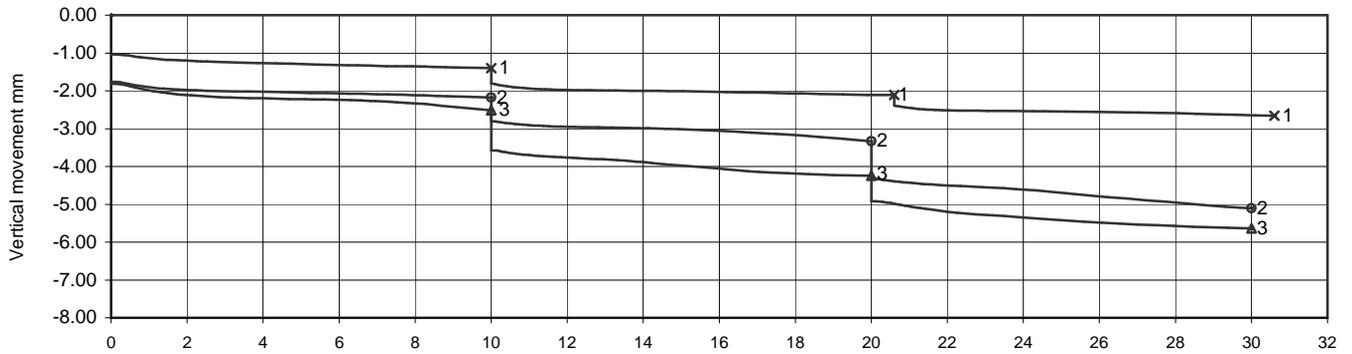
## Determination of shear strength by direct shear ( Small shearbox apparatus ) ( BS1377 : Part 7 : clause 4 : 1990 )

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	19.00-19.45		
		Sample No	43	Type	U	
		ID				
		Spec Ref				

### Consolidation stage(s)



### Shearing stage(s)



Ref

SLR7.4  
Rev 85  
Oct 10



Soil Mechanics



Printed:08/12/2011 12:47

Figure

**SSB 3**

sheet 2 of 2

**Determination of shear strength by direct shear ( Small shearbox apparatus )  
( BS1377 : Part 7 : clause 4 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	23.00-23.45		
			Sample No	51	Type	U
			ID			
			Spec Ref			

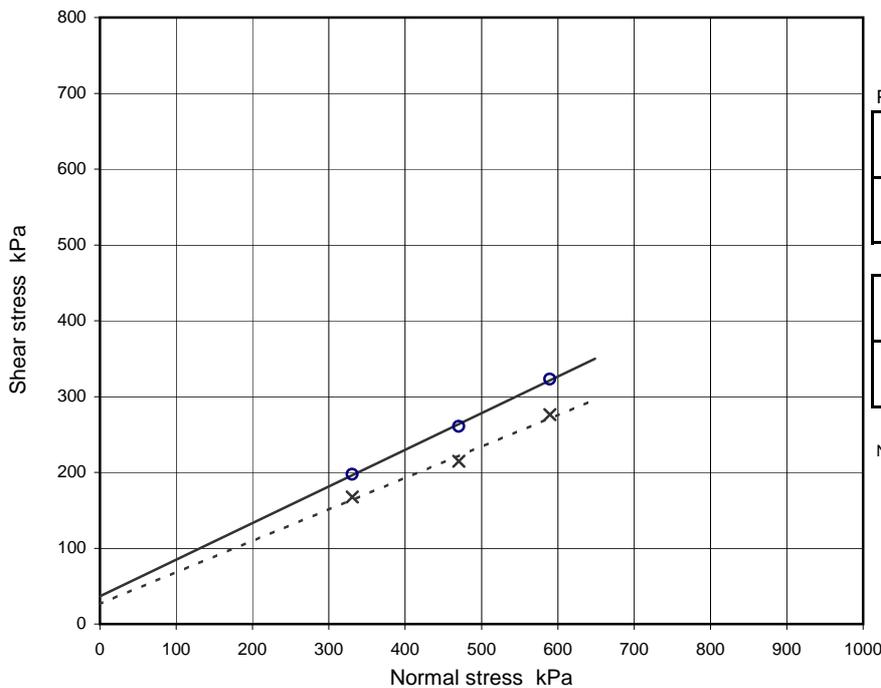
Soil Description	Firm to stiff brown slightly sandy slightly gravelly CLAY.
Specimen Type /Preparation	Undisturbed

Specimen(s) nominally 60mm x 60mm square  
 Test(s) carried out in submerged condition  
 Particle density, assumed 2.65 Mg/m<sup>3</sup>

Specimen Details		No.	1	2	3	4	5	6
Initial	Height	mm	23.4	23.4	23.4			
	Bulk Density	Mg/m <sup>3</sup>	2.19	2.20	2.21			
	Water Content	%	13.6	13.6	13.6			
	Dry density	Mg/m <sup>3</sup>	1.93	1.94	1.95			
	Voids ratio		0.372	0.365	0.360			
	Degree of Saturation	%	96	98	100			
Consol <sup>1</sup>	Consolidation / Normal Stress applied	kPa	<b>330</b>	<b>470</b>	<b>590</b>			
	Change in height during consolidation	mm	-0.906	-0.972	-1.280			
	Voids ratio after consolidation		0.319	0.308	0.286			
Shear see note 1	Voids ratio at end of test		0.282	0.211	0.126			
	Moisture content at end of test	%	10.6	8.0	4.7			
	Saturation at end of test	%	100	100	100			

**Shearing stage**

Rate of displacement	Peak	mm/min	0.003	0.003	0.003			
	Residual	mm/min	0.016	0.016	0.016			
Peak values, (o)	Relative displacement	mm	4.35	4.15	3.94			
	Shear stress	kPa	197.5	260.5	322.9			
Residual values, (x)	No. of reversals		2	2	3			
	Relative displacement	mm	28.95	30.00	40.00			
	Shear stress	kPa	167.5	214.5	276.1			



**Shear Strength Parameters**

Peak strength, (o)		Regression	Manual
c'	kPa	36	-
Ø'	degrees	26	-

Residual strength, (x)		Regression	Manual
c' <sub>R</sub>	kPa	26	-
Ø' <sub>R</sub>	degrees	22½	-

Notes :

1 After shear values based on BS1377. Pt 7 cl. 4.6.1.6 using δH calculated from consolidation and shear stages

Ref

SLR7.4  
Rev 85  
Oct 10



Printed:18/11/2011 16:32

Figure

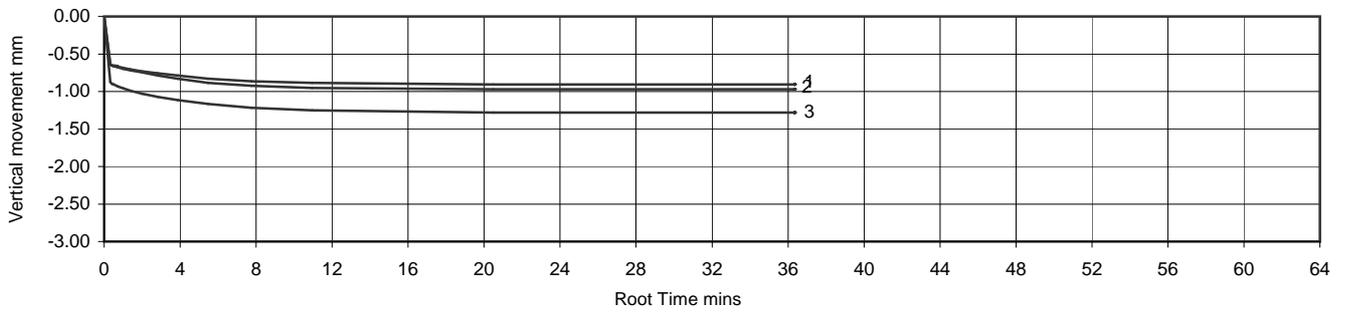
**SSB 4**

sheet 1 of 2

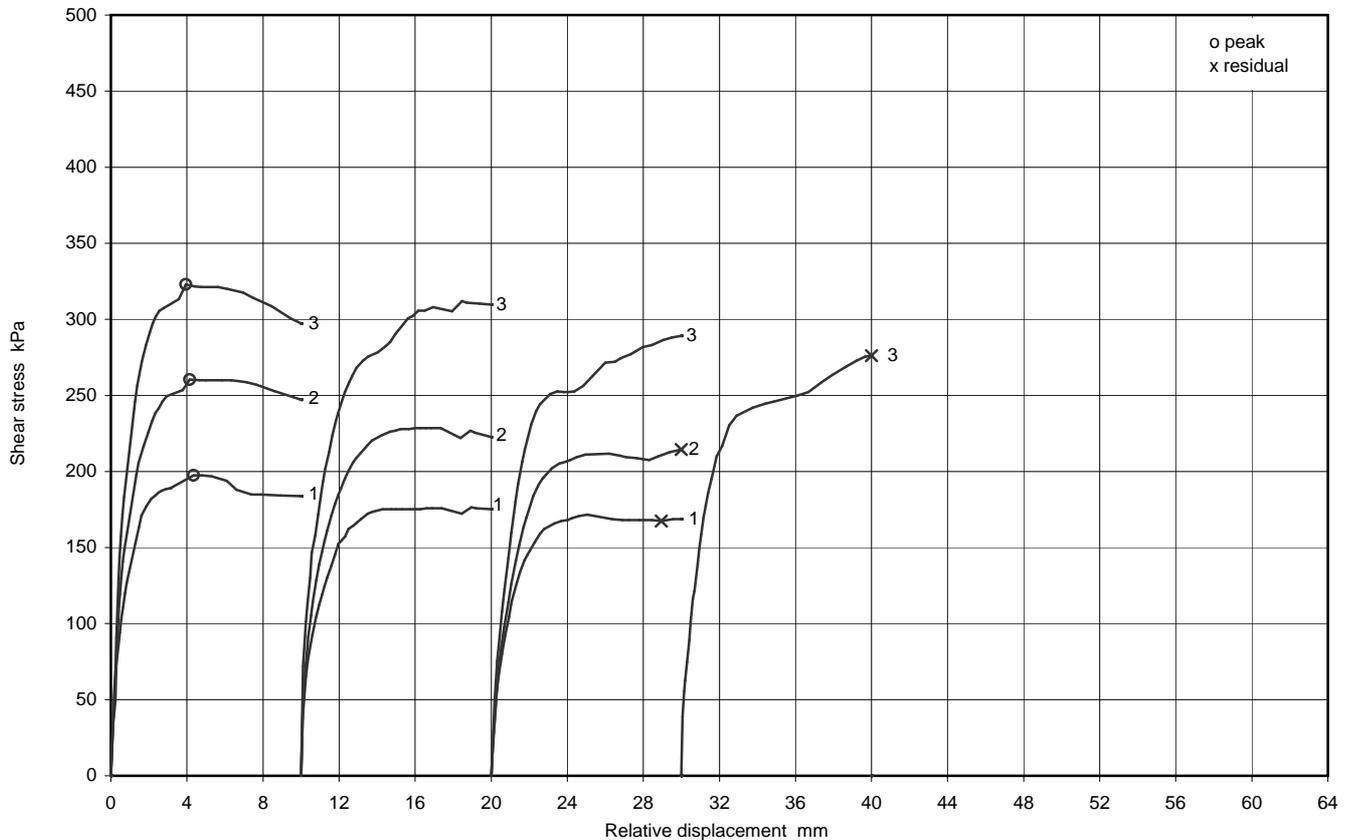
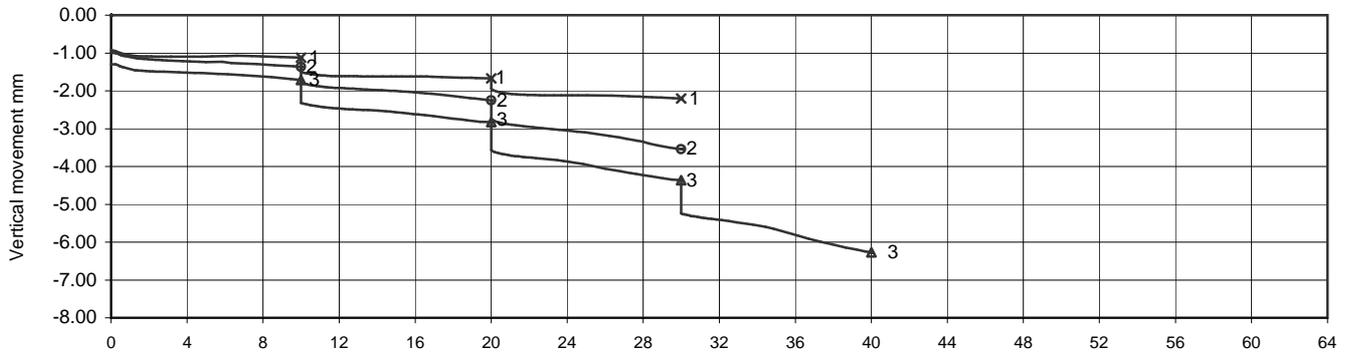
## Determination of shear strength by direct shear ( Small shearbox apparatus ) ( BS1377 : Part 7 : clause 4 : 1990 )

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	23.00-23.45		
		Sample No	51	Type	U	
		ID				
		Spec Ref				

### Consolidation stage(s)



### Shearing stage(s)



Ref

SLR7.4  
Rev 85  
Oct 10



**Soil Mechanics**



Printed: 18/11/2011 16:32

Figure

**SSB 4**

sheet 2 of 2

**Determination of residual shear strength using the small ring shear apparatus  
( BS1377 : Part 7 : clause 6 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH2		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.00-15.45		
			Sample No	31	Type	U
			ID			
			Spec Ref			

Soil Description	Firm to stiff brown slightly sandy slightly gravelly CLAY.	
Specimen Type /Preparation	-1.18mm material. Recompacted at as received moisture content.	

**Specimen Details**

Initial height	mm	5.0
Initial Bulk Density	Mg/m <sup>3</sup>	2.38
Initial Moisture Content ( trimmings )	%	13.1
Initial Dry Density	Mg/m <sup>3</sup>	2.10
Moisture content at end of test	%	16.3

Nominal dimensions  
Outer diameter 100 mm  
Inner diameter 70 mm

Test carried out in submerged condition

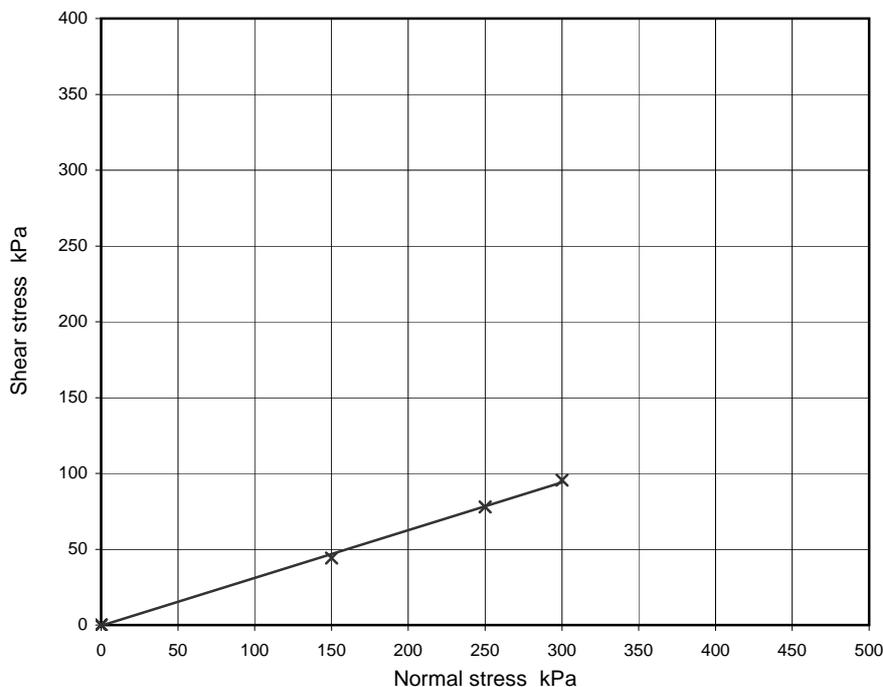
<b>STAGE No.</b>	<b>1</b>	<b>2</b>	<b>3</b>			
------------------	----------	----------	----------	--	--	--

**Consolidation stage(s)**

Consolidation / Normal Stress applied	kPa	<b>150</b>	<b>250</b>	<b>300</b>		
Change in height during stage	mm	-0.178	-0.138	-0.060		

**Shearing stage(s)**

Torque arm diameter	mm	150	150	150		
Rate of displacement	Angular	°/min	0.120	0.120	0.120	
	Average linear	mm/min	0.089	0.089	0.089	
At residual state	Normal stress	kPa	150	250	300	
	Mean linear displacement	mm	14.5	32.5	65.9	
	Mean Shear stress	kPa	44.2	77.9	95.6	



**Residual Shear Strength Parameters**

$c'_R$  is assumed to be zero, BS 1377

$\phi'_R$  17.5 degrees

Notes :

1 Bulk and dry density values are based on nominal dimensions and are therefore indicative only.

**Ref**

SLR7.6  
Rev 85  
Jan 11



Printed:18/11/2011 16:36

**Figure**

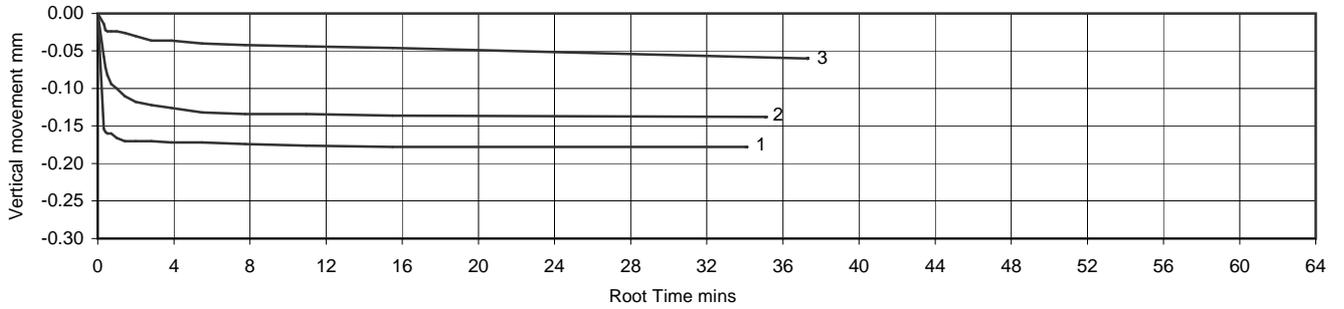
**RS 1**

sheet 1 of 2

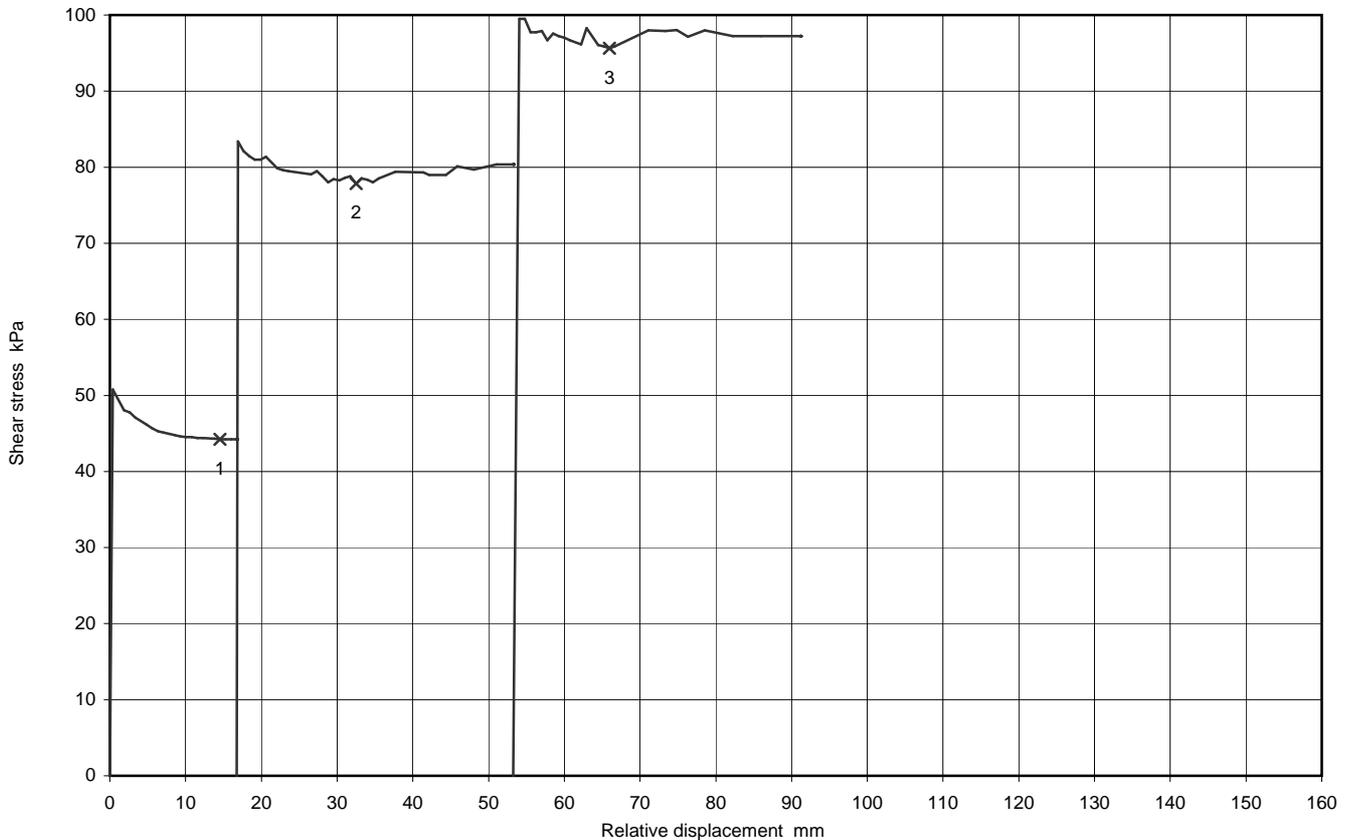
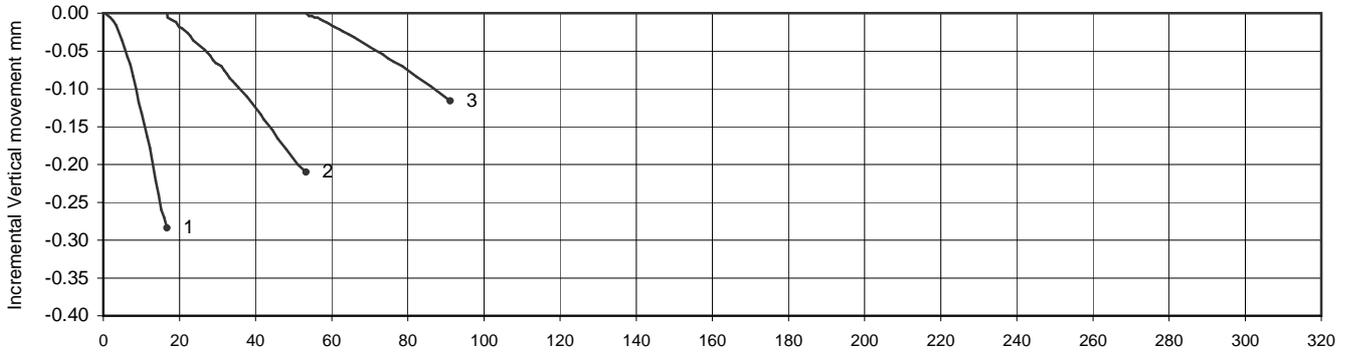
**Determination of residual shear strength using the small ring shear apparatus  
( BS1377 : Part 7 : clause 6 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH2			
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	15.00-15.45			
			Sample No	31	Type	U	
			ID				
			Spec Ref				

**Consolidation stage(s)**



**Shearing stage(s)**



Ref

SLR7.6  
Rev 85  
Jan 11



Printed:18/11/2011 16:36

Figure

**RS 1**

sheet 2 of 2

**Determination of residual shear strength using the small ring shear apparatus  
( BS1377 : Part 7 : clause 6 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH3		
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	7.00-7.45		
			Sample No	20	Type	U
			ID			
			Spec Ref			

Soil Description	Firm brown slightly sandy slightly gravelly CLAY.	
Specimen Type /Preparation	-1.18mm material. Recompacted at as received moisture content.	

**Specimen Details**

Initial height	mm	5.0
Initial Bulk Density	Mg/m <sup>3</sup>	2.33
Initial Moisture Content ( trimmings )	%	16.2
Initial Dry Density	Mg/m <sup>3</sup>	2.01
Moisture content at end of test	%	15.4

Nominal dimensions  
Outer diameter 100 mm  
Inner diameter 70 mm

Test carried out in submerged condition

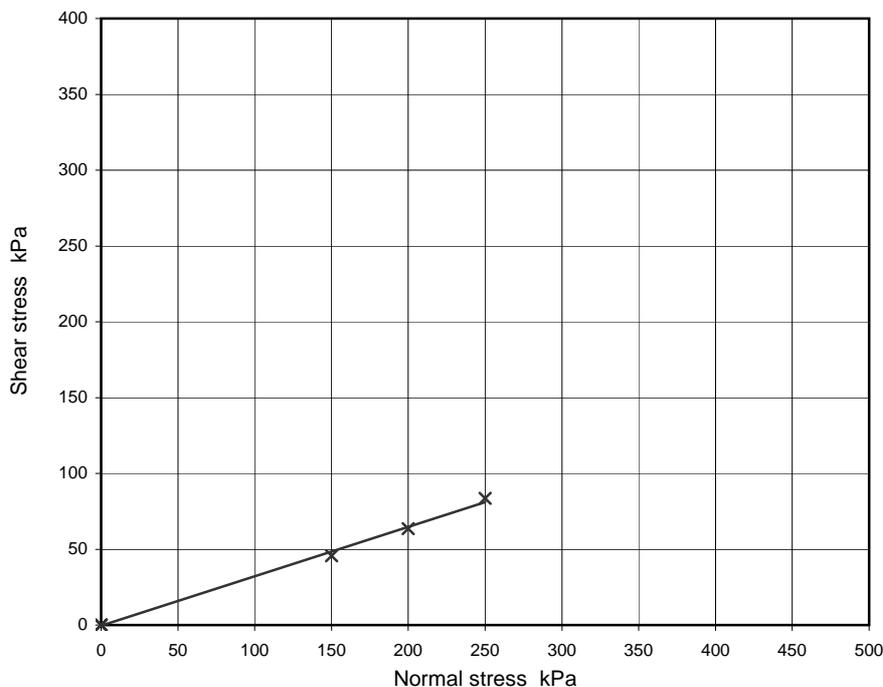
<b>STAGE No.</b>	<b>1</b>	<b>2</b>	<b>3</b>			
------------------	----------	----------	----------	--	--	--

**Consolidation stage(s)**

Consolidation / Normal Stress applied	kPa	<b>150</b>	<b>200</b>	<b>250</b>			
Change in height during stage	mm	-0.204	-0.050	-0.036			

**Shearing stage(s)**

Torque arm diameter	mm	150	150	150			
Rate of displacement	Angular	°/min	0.120	0.120	0.120		
	Average linear	mm/min	0.089	0.089	0.089		
At residual state	Normal stress	kPa	150	200	250		
	Mean linear displacement	mm	8.0	39.6	89.0		
	Mean Shear stress	kPa	45.8	63.6	83.5		



**Residual Shear Strength Parameters**

$c'_R$  is assumed to be zero, BS 1377

$\phi'_R$  **18.0** degrees

Notes :

1 Bulk and dry density values are based on nominal dimensions and are therefore indicative only.

**Ref**

SLR7.6  
Rev 85  
Jan 11



Printed:18/11/2011 16:42

**Figure**

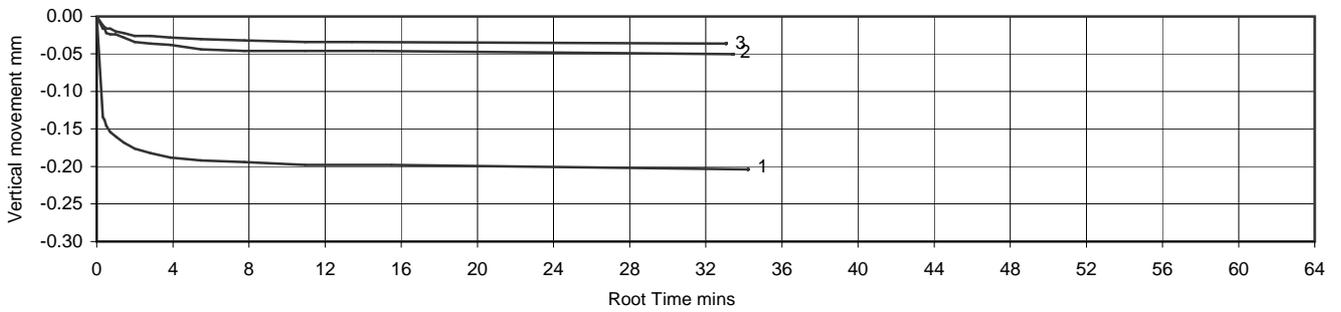
**RS 2**

sheet 1 of 2

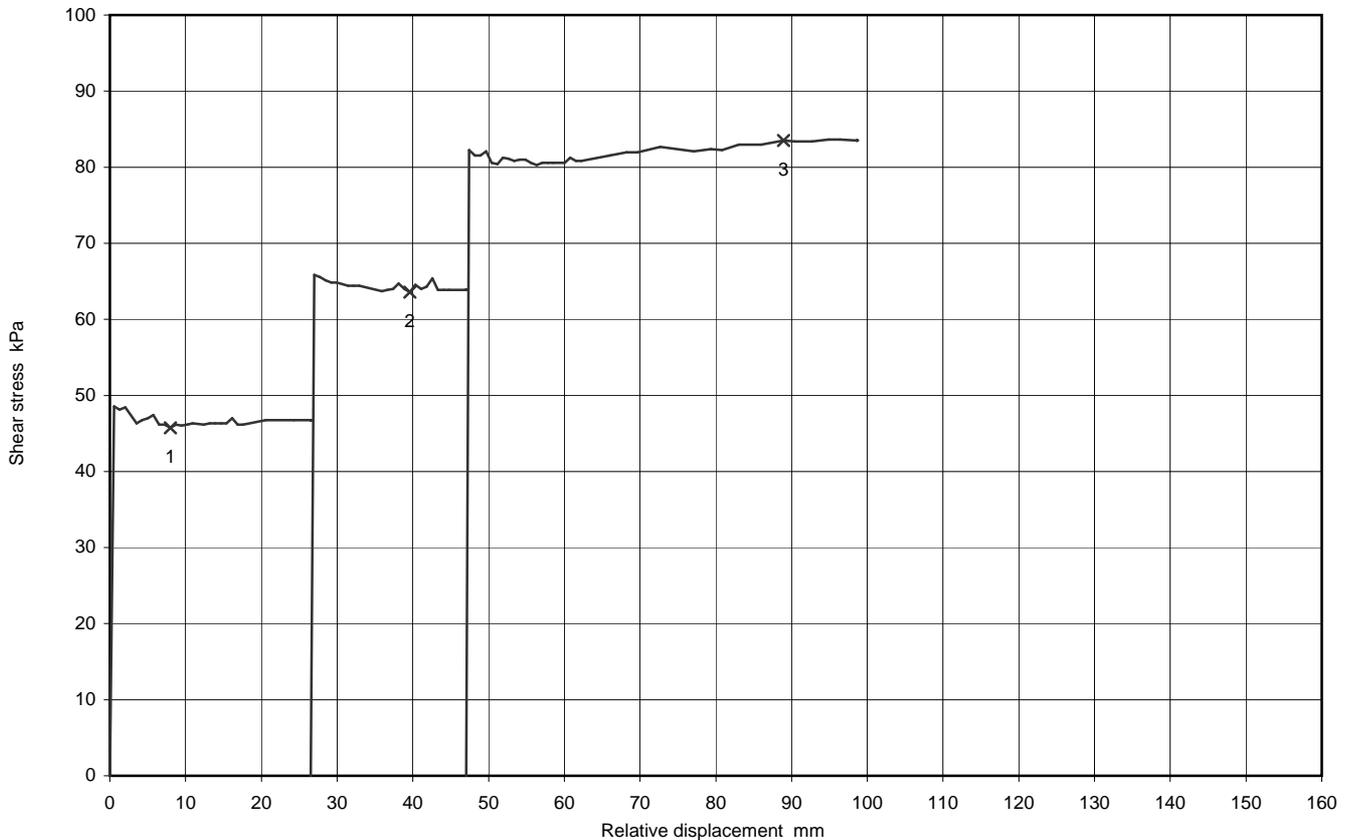
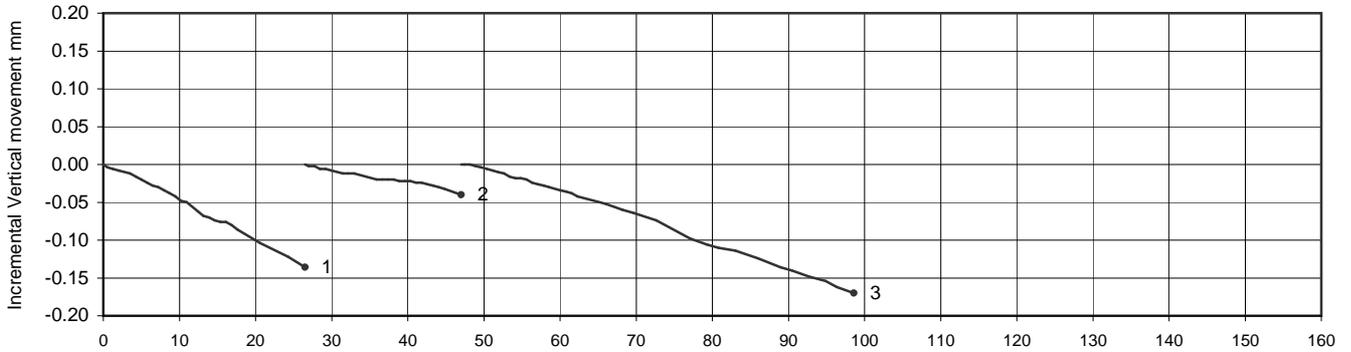
**Determination of residual shear strength using the small ring shear apparatus  
( BS1377 : Part 7 : clause 6 : 1990 )**

Project No	A1077-11	Sample Details:	Hole No.	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	7.00-7.45		
		Sample No	20	Type	U	
		ID				
		Spec Ref				

**Consolidation stage(s)**



**Shearing stage(s)**



Ref

SLR7.6  
Rev 85  
Jan 11



Printed:18/11/2011 16:42

Figure

**RS 2**

sheet 2 of 2











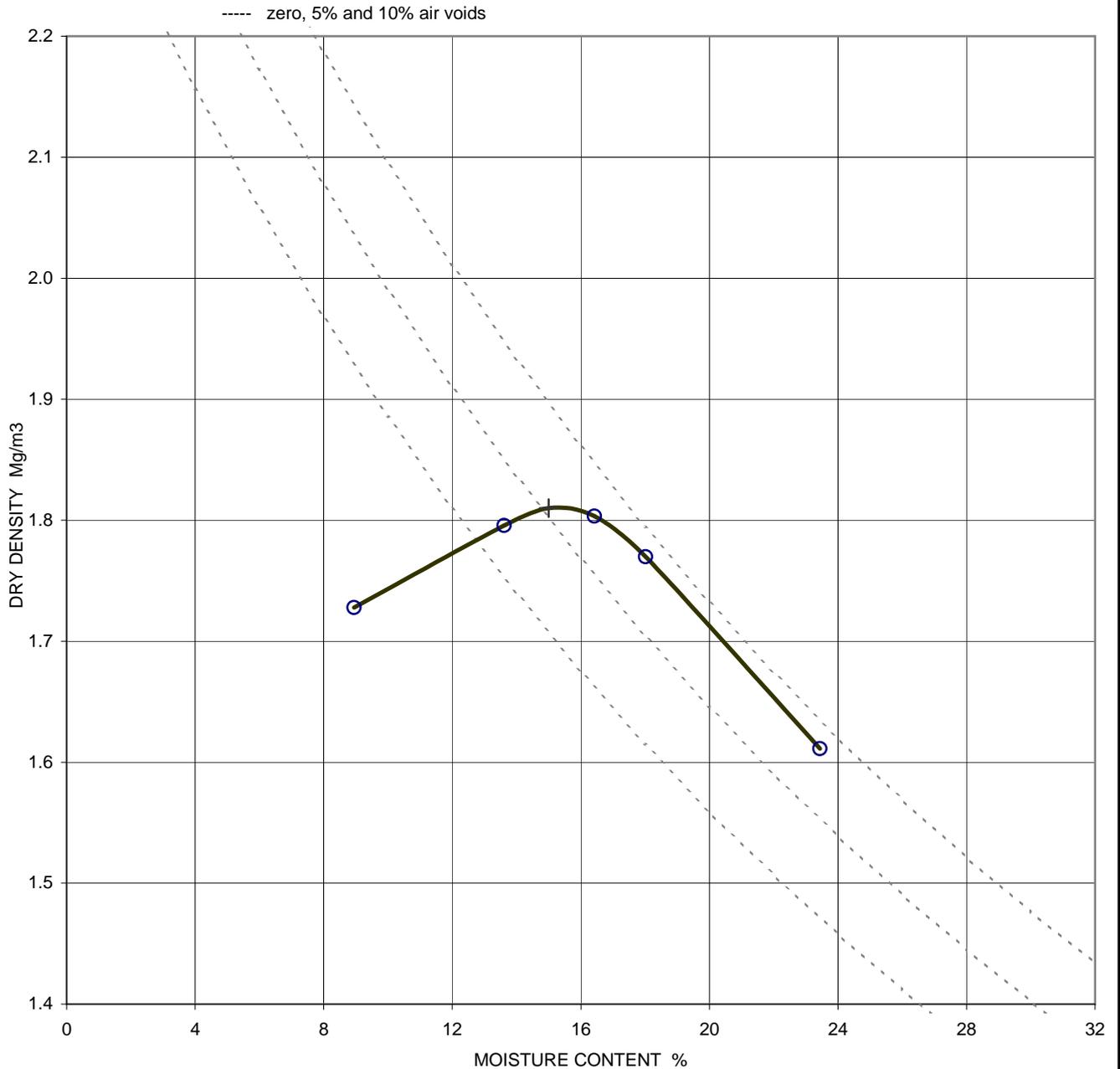






**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH1			
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50			
			Samp No	4	Type	B	
			ID	ESGA1077-11201110100000000004			
			Spec Ref				



Soil description	Dark brown slightly gravelly sandy CLAY with occasional rootlets.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>1.81</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	0 %	<b>15</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11



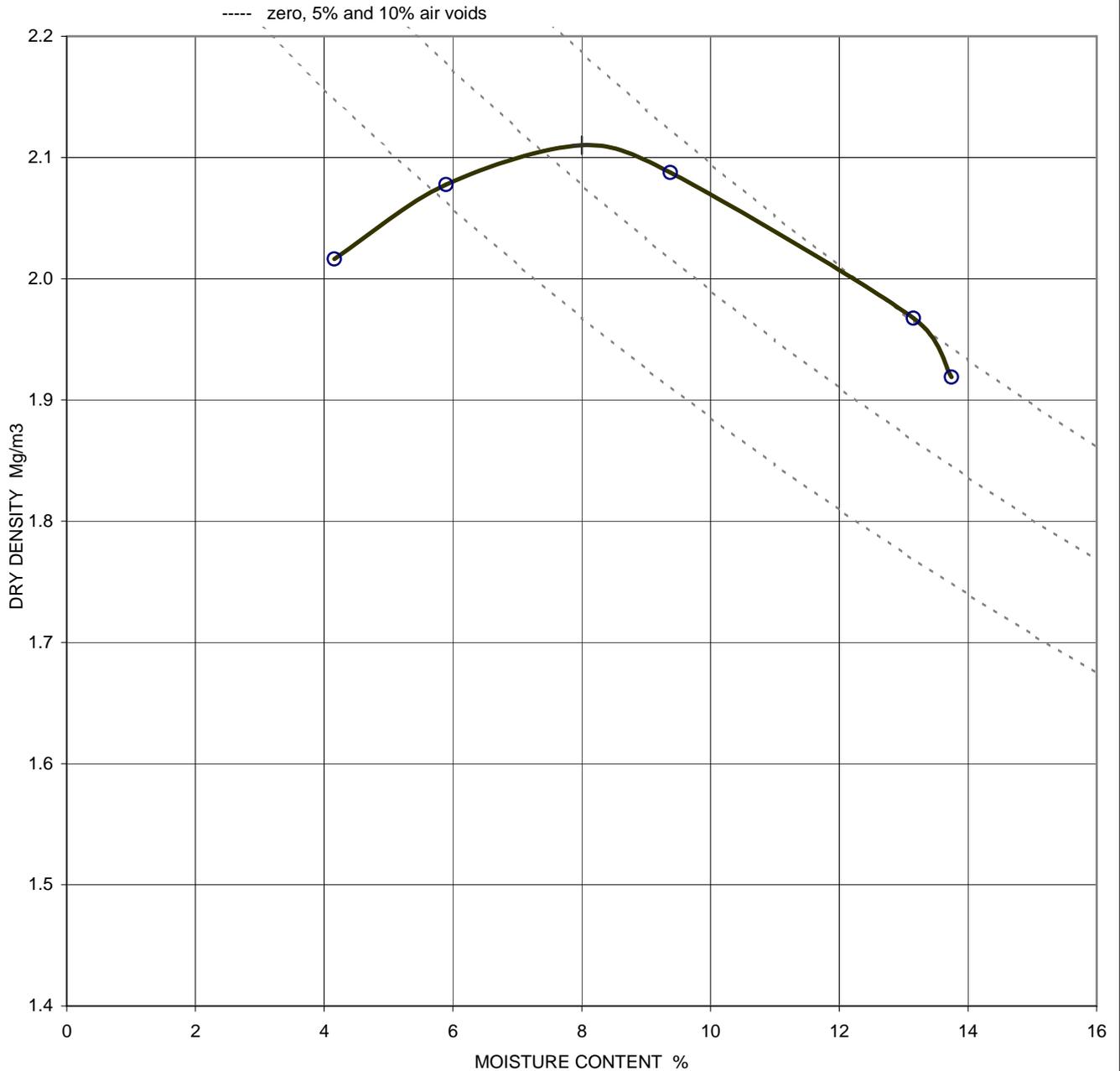
Printed:18/11/2011 17:33

**Figure**  
**COMPH 1**



**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDBSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	12	Type	B
			ID	ESGA1077-11201110100000000013		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.6, 4.5 kg rammer in a CBR mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, composite specimens tested	<b>2.11</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	3 %	<b>8.0</b>
Particle density	2.65 assumed	
Remarks		

QA Ref  
SLD 4, 3.5/6  
Rev 66  
Aug 11

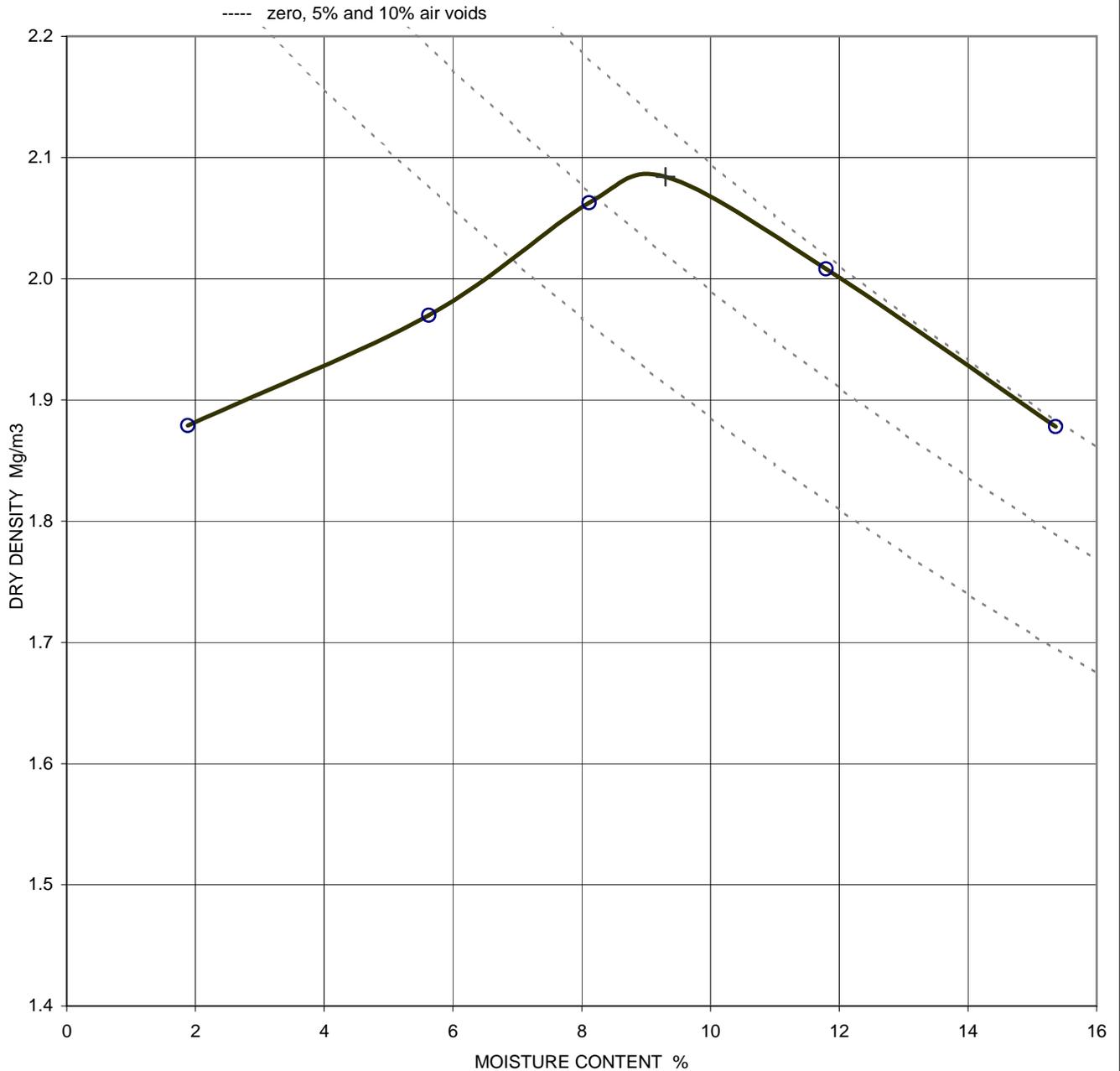


Printed:18/11/2011 17:33

Figure  
**COMPH 3**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDBSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.65		
			Samp No	16	Type	B
			ID	ESGA1077-11201110100000000017		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>2.08</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	4 %	<b>9.3</b>
Particle density	2.65 assumed	
Remarks		

QA Ref  
SLD 4, 3.5/6  
Rev 66  
Aug 11

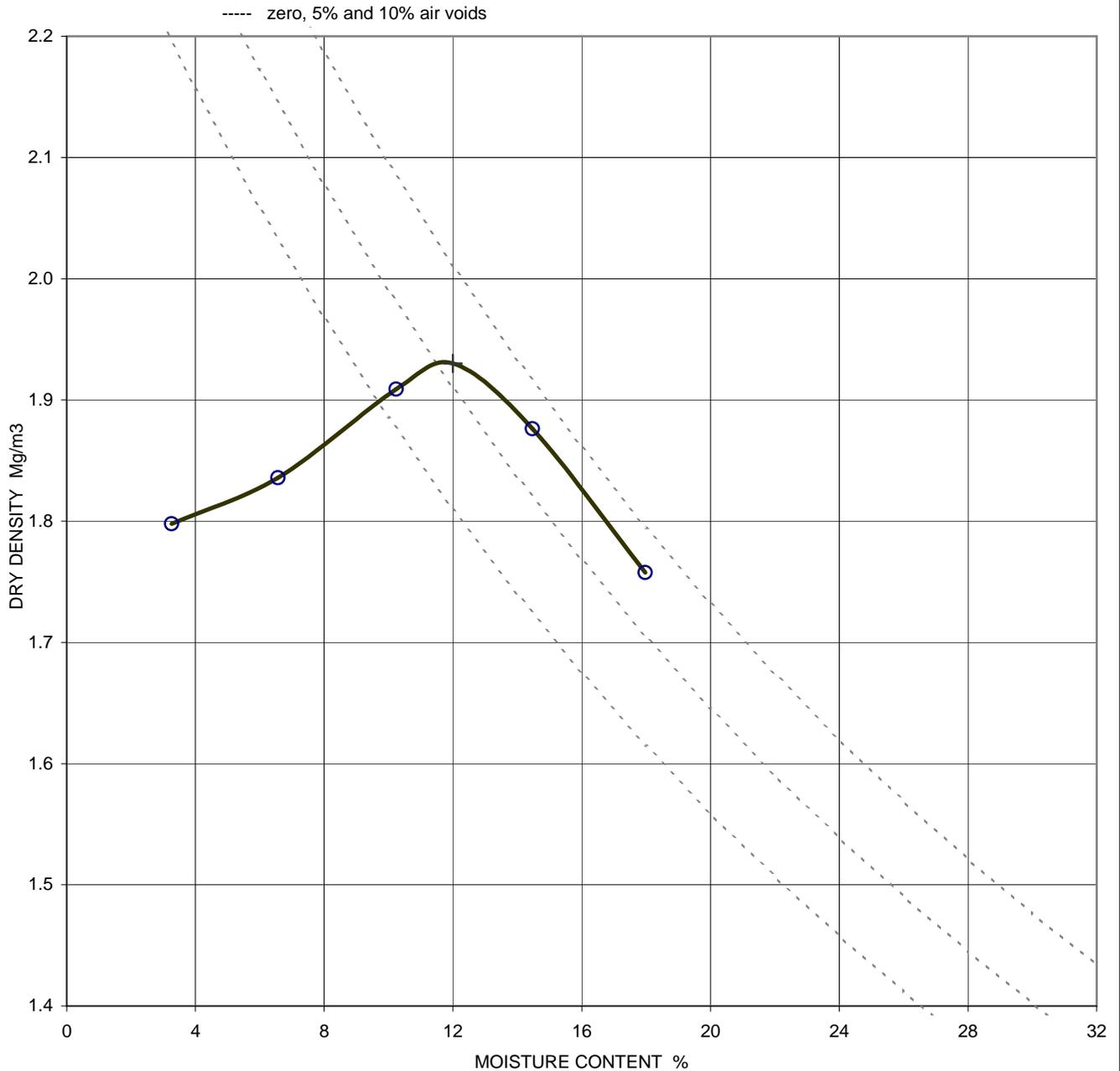


Printed:18/11/2011 17:33

Figure  
**COMPH 4**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110100000000069		
			Spec Ref			



Soil description	Brown slightly gravelly sandy silty CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>1.93</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	0 %	<b>12</b>
Particle density	2.65 assumed	
Remarks		

QA Ref  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

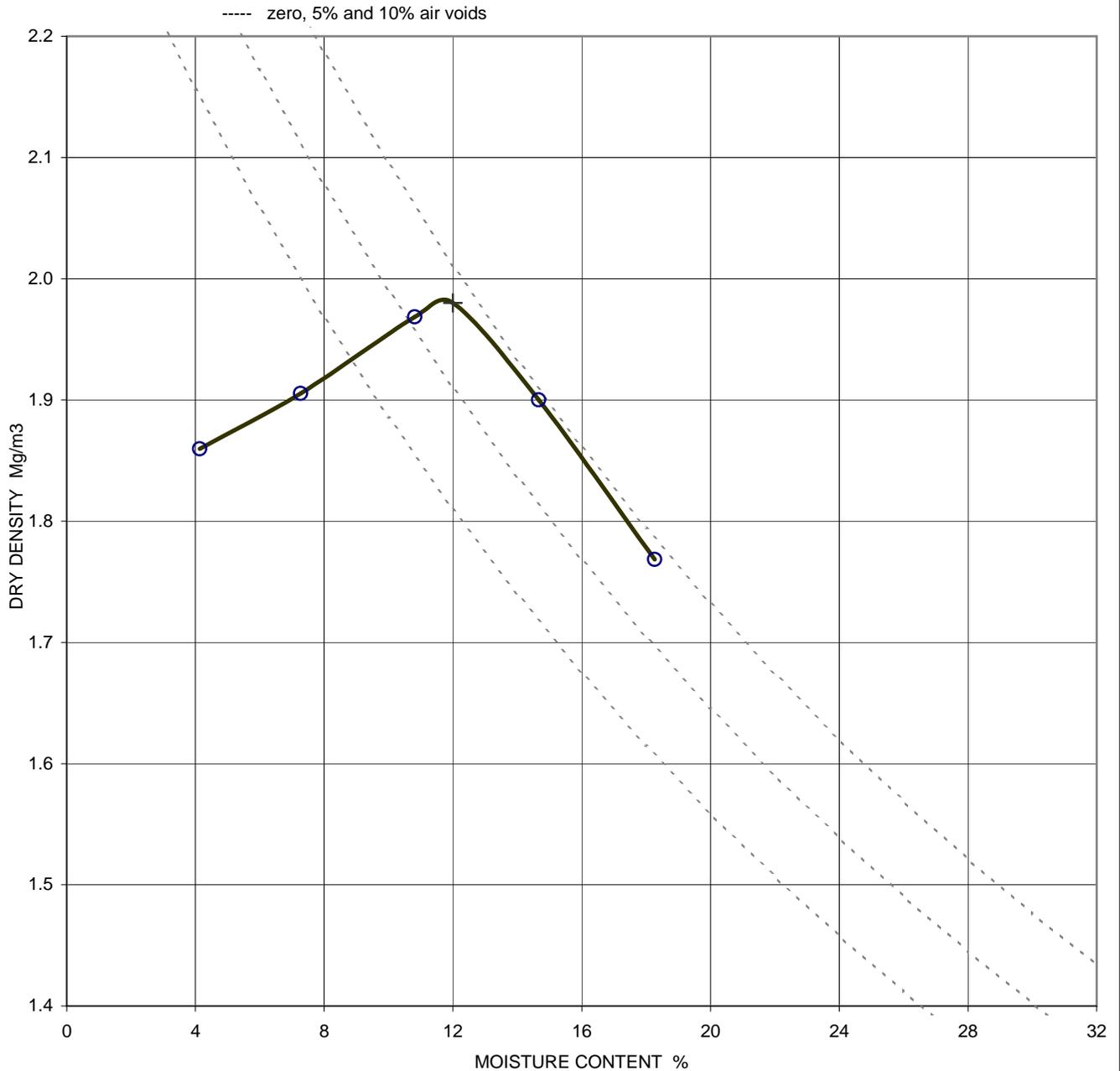


Printed:18/11/2011 17:33

Figure  
**COMPH 5**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	7	Type	B
			ID	ESGA1077-11201110100000000073		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m3
Preparation	Original material was natural, separate specimens tested	<b>1.98</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	2 %	<b>12</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

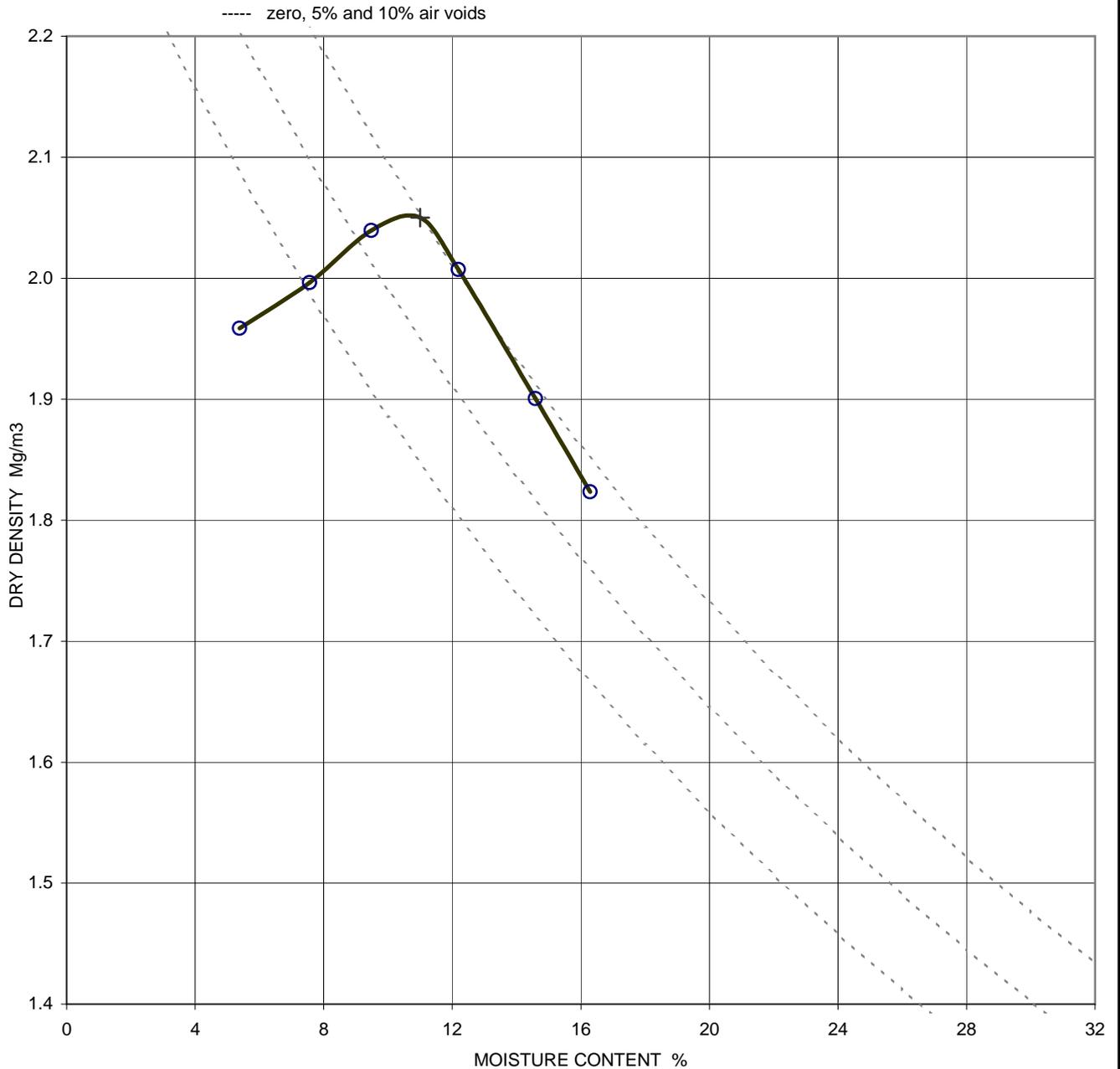


Printed:18/11/2011 17:33

**Figure**  
**COMPH 6**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	11	Type	B
			ID	ESGA1077-11201110100000000077		
			Spec Ref			



Soil description	Gryeish brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, composite specimens tested	<b>2.05</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	2 %	<b>11</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

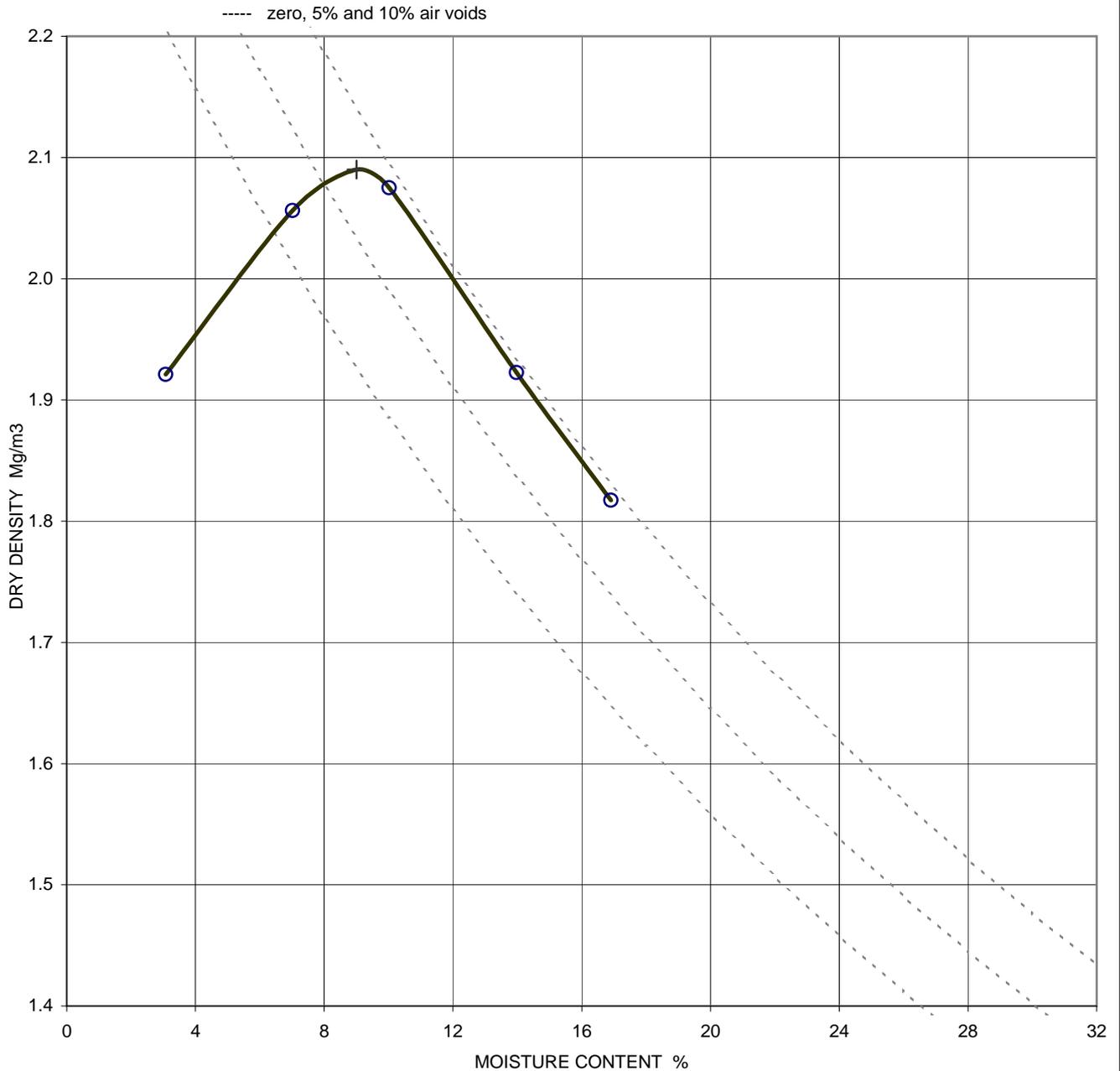


Printed:18/11/2011 17:33

**Figure**  
**COMPH 7**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.00		
			Samp No	15	Type	B
			ID	ESGA1077-11201110100000000081		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>2.09</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	1 %	<b>9.0</b>
Particle density	2.65 assumed	
Remarks		

QA Ref  
SLD 4, 3.5/6  
Rev 66  
Aug 11

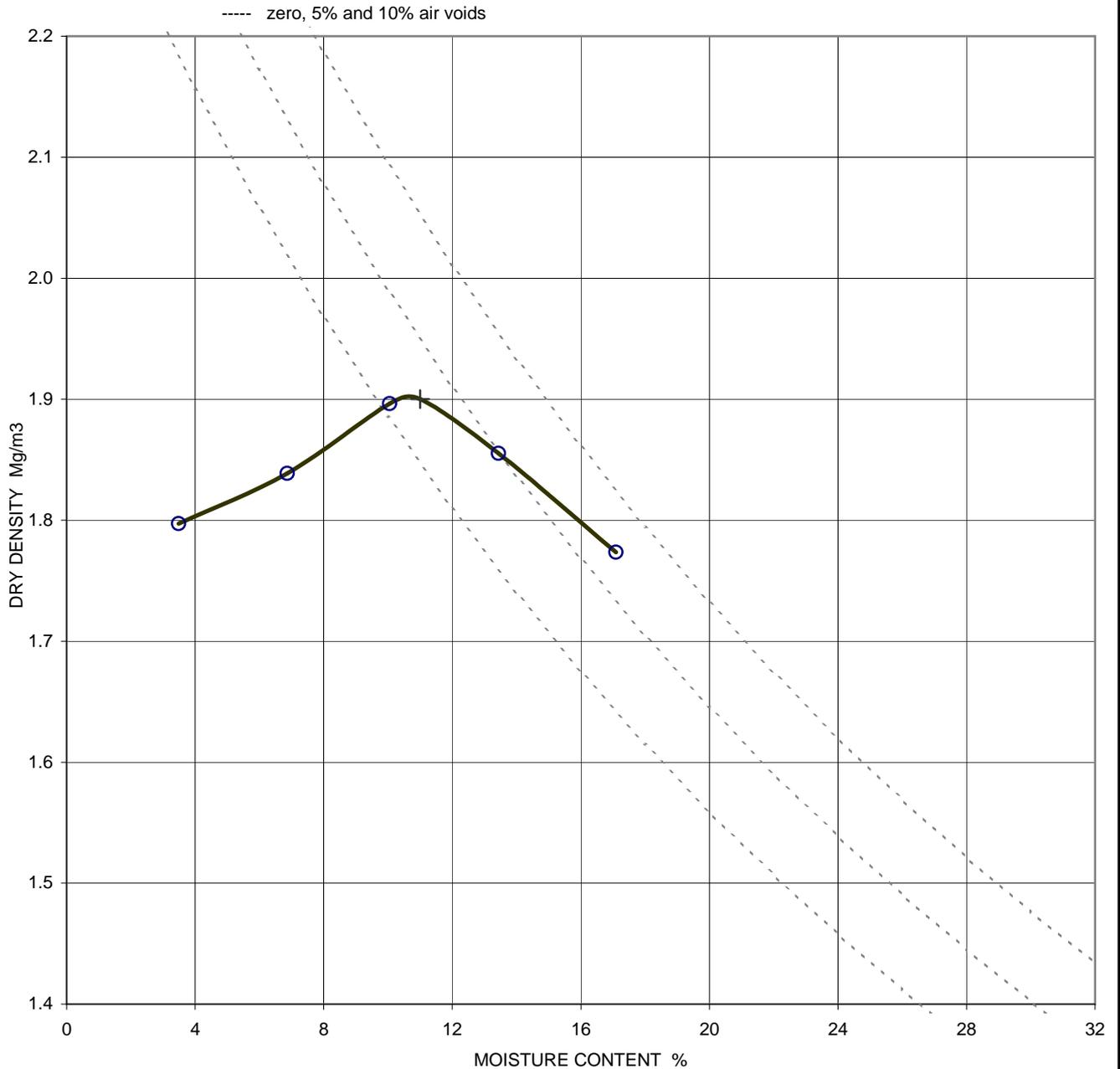


Printed:18/11/2011 17:33

Figure  
**COMPH 8**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDBSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110110000000128		
			Spec Ref			



Soil description	Brown slightly gravelly sandy CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>1.90</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	0 %	<b>11</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

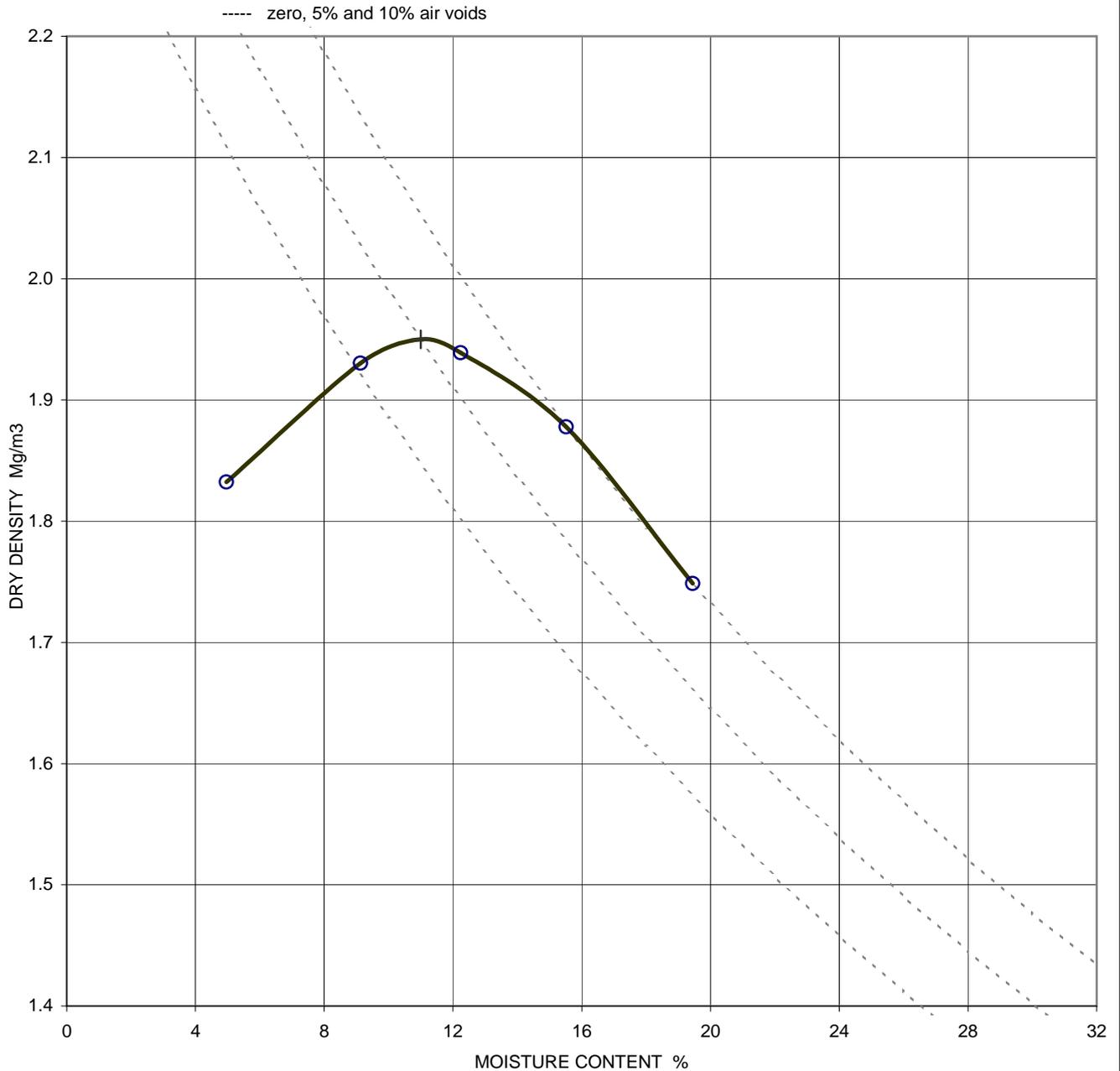


Printed:18/11/2011 17:33

**Figure**  
**COMPH 9**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDBSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.85		
			Samp No	7	Type	B
			ID	ESGA1077-11201110110000000132		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>1.95</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	1 %	<b>11</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

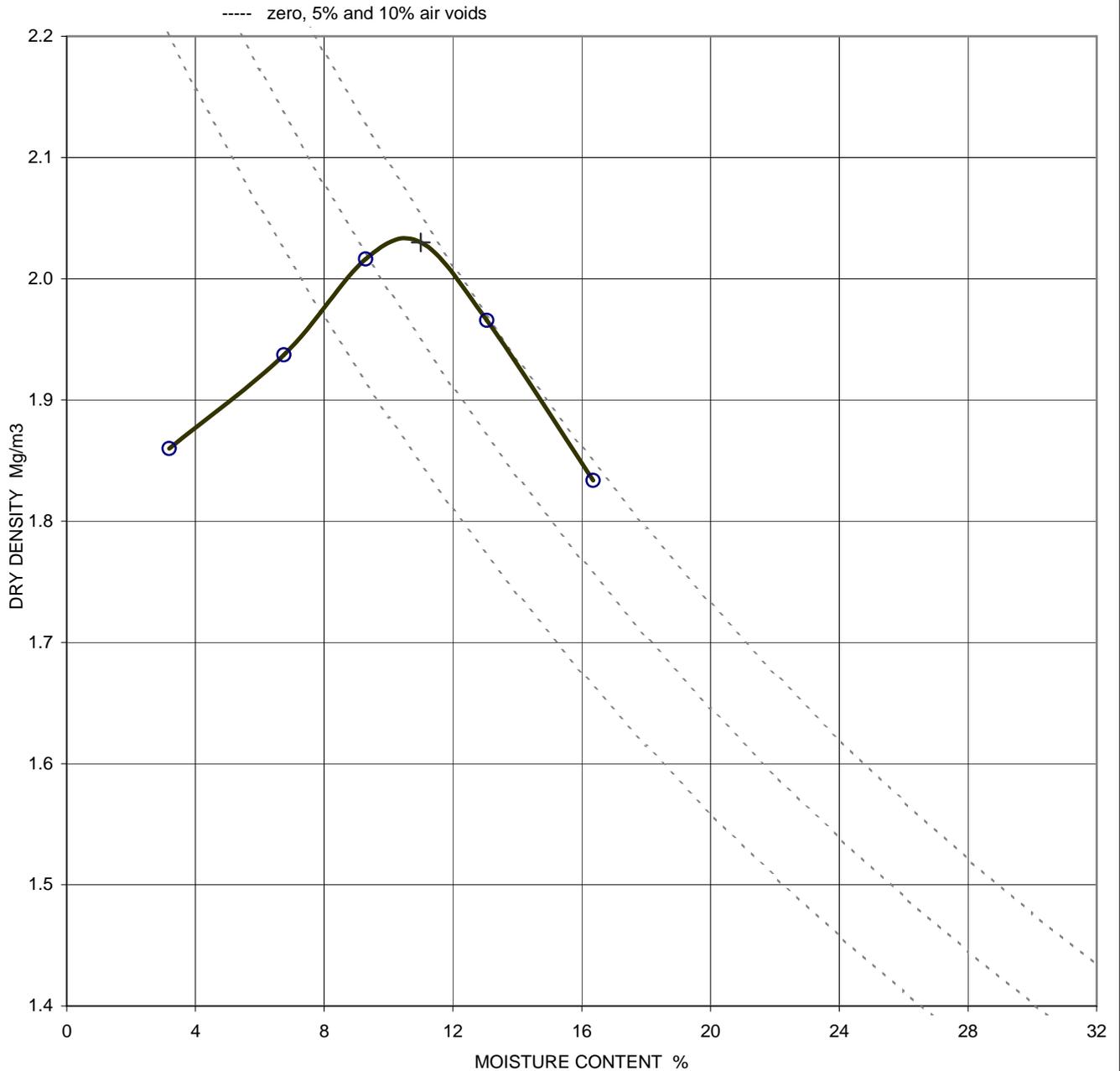


Printed:18/11/2011 17:33

**Figure**  
**COMPH 10**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDBSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.15		
			Samp No	11	Type	B
			ID	ESGA1077-11201110110000000136		
			Spec Ref			



Soil description	Reddish brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>2.03</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	0 %	<b>11</b>
Particle density	2.65 assumed	
Remarks		

QA Ref  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

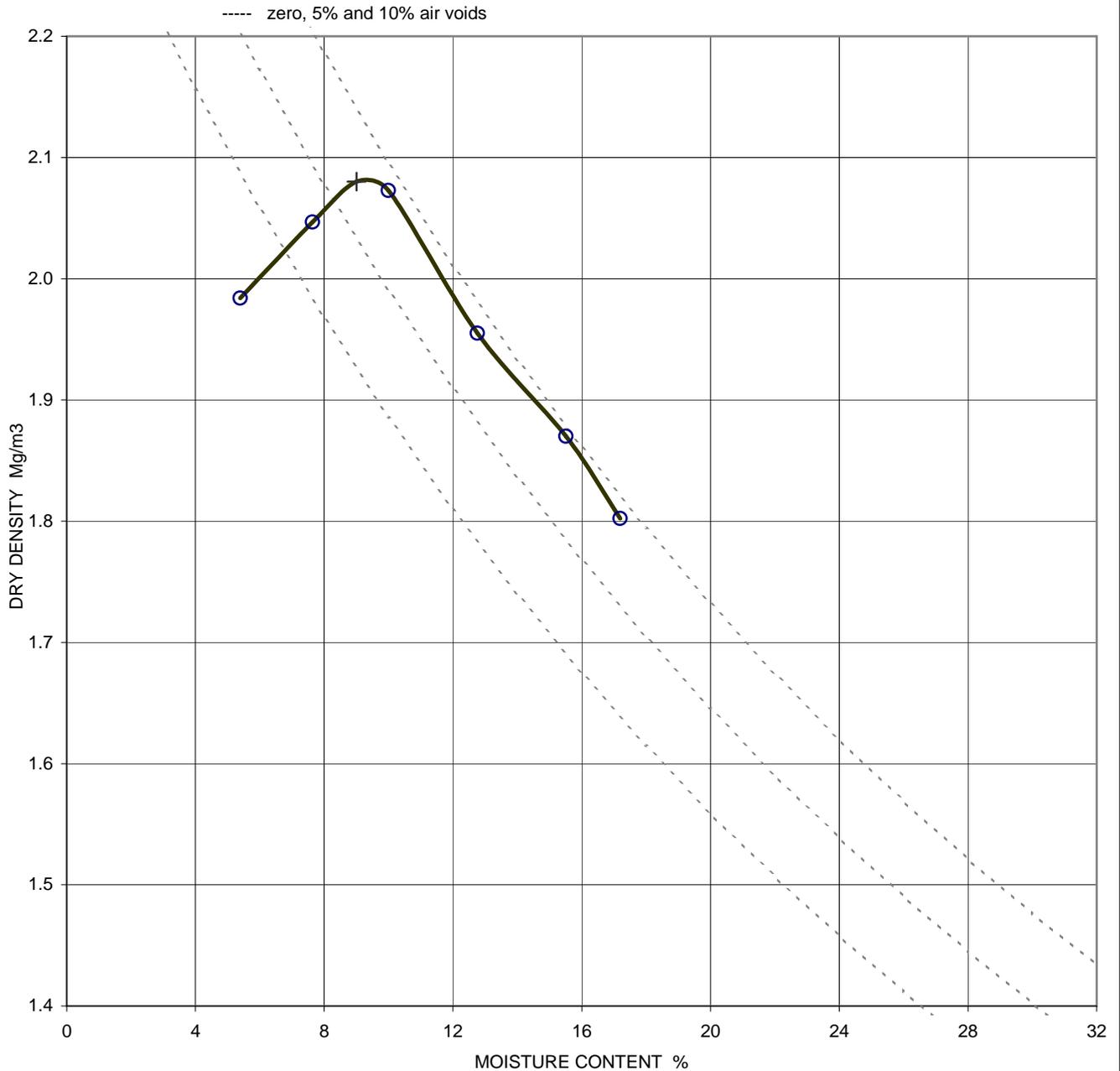


Printed:18/11/2011 17:33

Figure  
**COMPH 11**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDBSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65		
			Samp No	15	Type	B
			ID	ESGA1077-11201110110000000140		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, composite specimens tested	<b>2.08</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	1 %	<b>9.0</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11

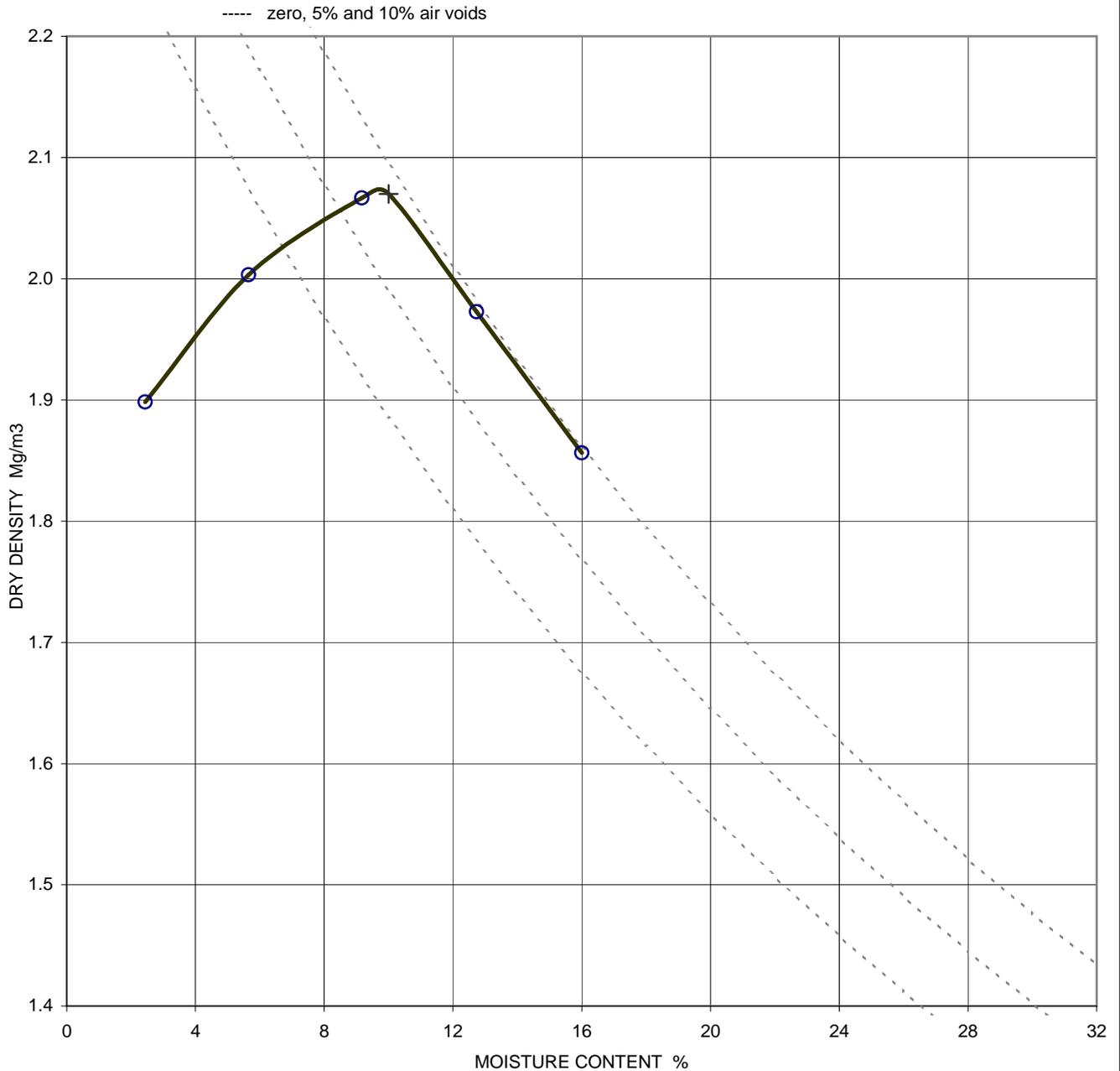


Printed:18/11/2011 17:33

**Figure**  
**COMPH 12**

**DRY DENSITY / MOISTURE CONTENT RELATIONSHIP**  
**BS1377 : PART 4 : 1990 : HEAVY COMPACTION, 4.5 kg rammer**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.15		
			Samp No	19	Type	B
			ID	ESGA1077-1120111011000000144		
			Spec Ref			



Soil description	Brown slightly sandy slightly gravelly CLAY.	Derived Parameters +
Test method	BS 1377:part 4:1990: clause 3.5, 4.5 kg rammer in a 1 litre mould	Maximum dry density, Mg/m <sup>3</sup>
Preparation	Original material was natural, separate specimens tested	<b>2.07</b>
Material > 37.5mm	0 %	Optimum moisture content, %
Material < 37.5mm > 20mm	1 %	<b>10</b>
Particle density	2.65 assumed	
Remarks		

**QA Ref**  
 SLD 4, 3.5/6  
 Rev 66  
 Aug 11



Printed:18/11/2011 17:33

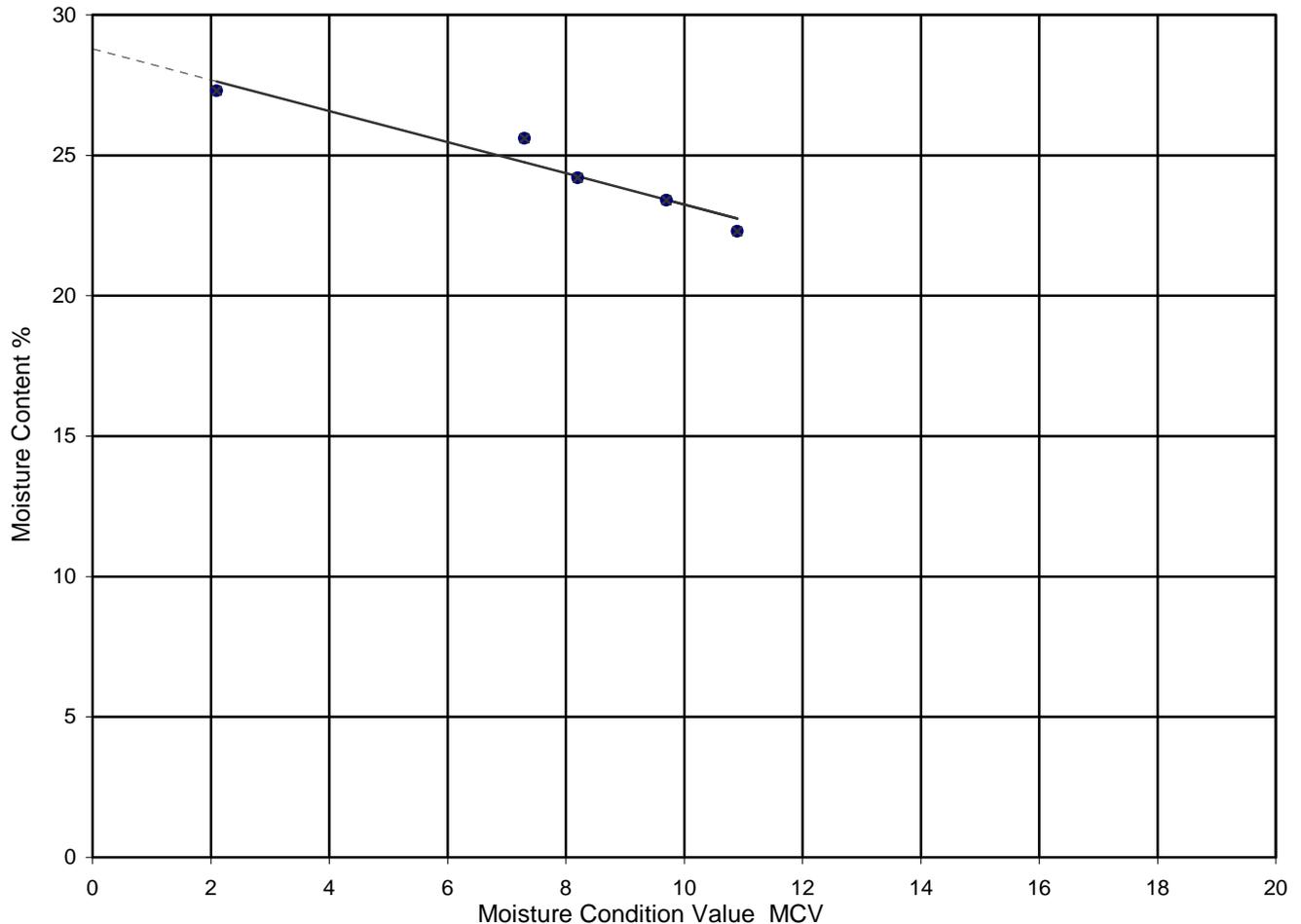
**Figure**  
**COMPH 13**

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	4	Type	B
			ID	ESGA1077-11201110100000000004		
			Spec Ref			

### MCV CALIBRATION

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



#### Characteristics of calibration line (determined using linear regression)

Intercept	28.8
Slope	-0.55
Sensitivity ( Change in MCV per 1% moisture content )	1.80
Correlation ( proximity of test points to regression line )	-0.97
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

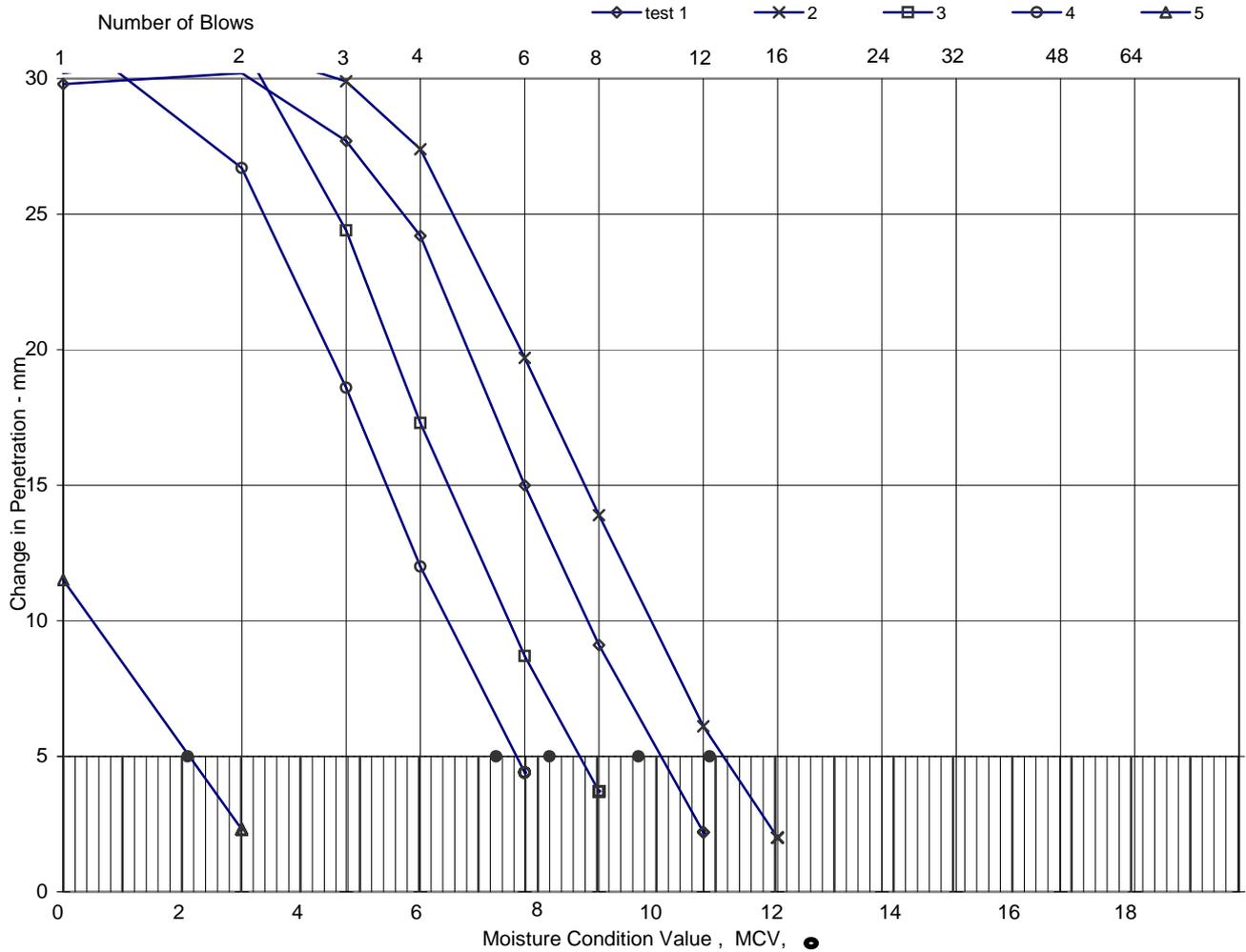


Printed: 18/11/2011 17:35

Figure  
**MCVREL 1**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50
			Samp No	4
			Type	B
			ID	ESGA1077-11201110100000000004
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		9.7	10.9	8.2	7.3	2.1
Moisture Content	%	23.4	22.3	24.2	25.6	27.3
Bulk density after test	Mg/m <sup>3</sup>	2.00	2.04	2.00	1.98	1.91
Dry density after test	Mg/m <sup>3</sup>	1.62	1.67	1.61	1.58	1.50

Soil description	Dark brown slightly gravelly sandy CLAY with occasional rootlets.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	23.4
Material retained on 20mm sieve	%	0

Method of determining MCV	Steepest straight line
---------------------------	------------------------

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed: 18/11/2011 17:35

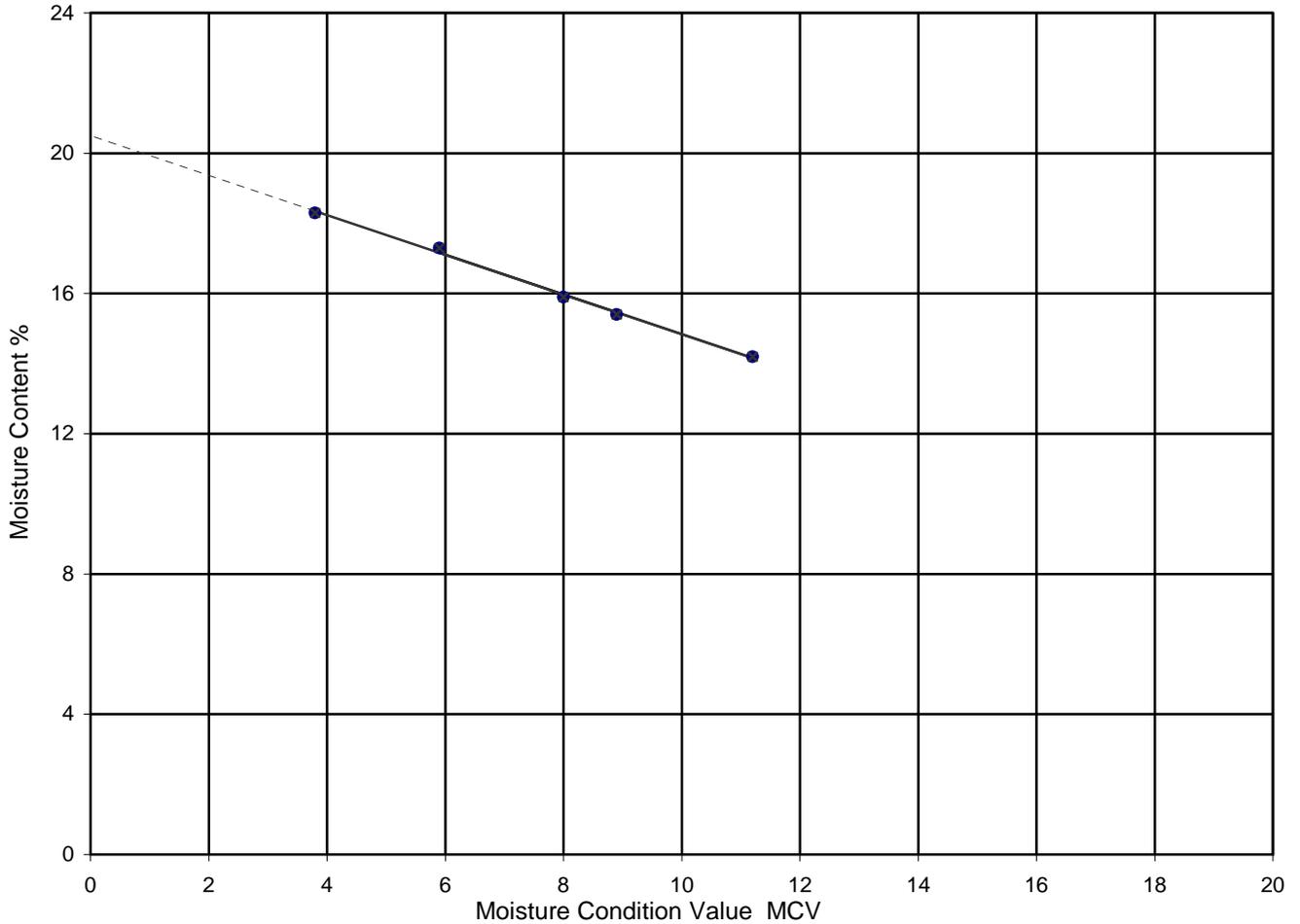
Figure  
**MCVREL 1**  
sheet 2 of 2

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	8	Type	B
			ID	ESGA1077-1120111010000000009		
			Spec Ref			

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



**Characteristics of calibration line (determined using linear regression)**

Intercept	20.5
Slope	-0.57
Sensitivity ( Change in MCV per 1% moisture content )	1.76
Correlation ( proximity of test points to regression line )	-1.00
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

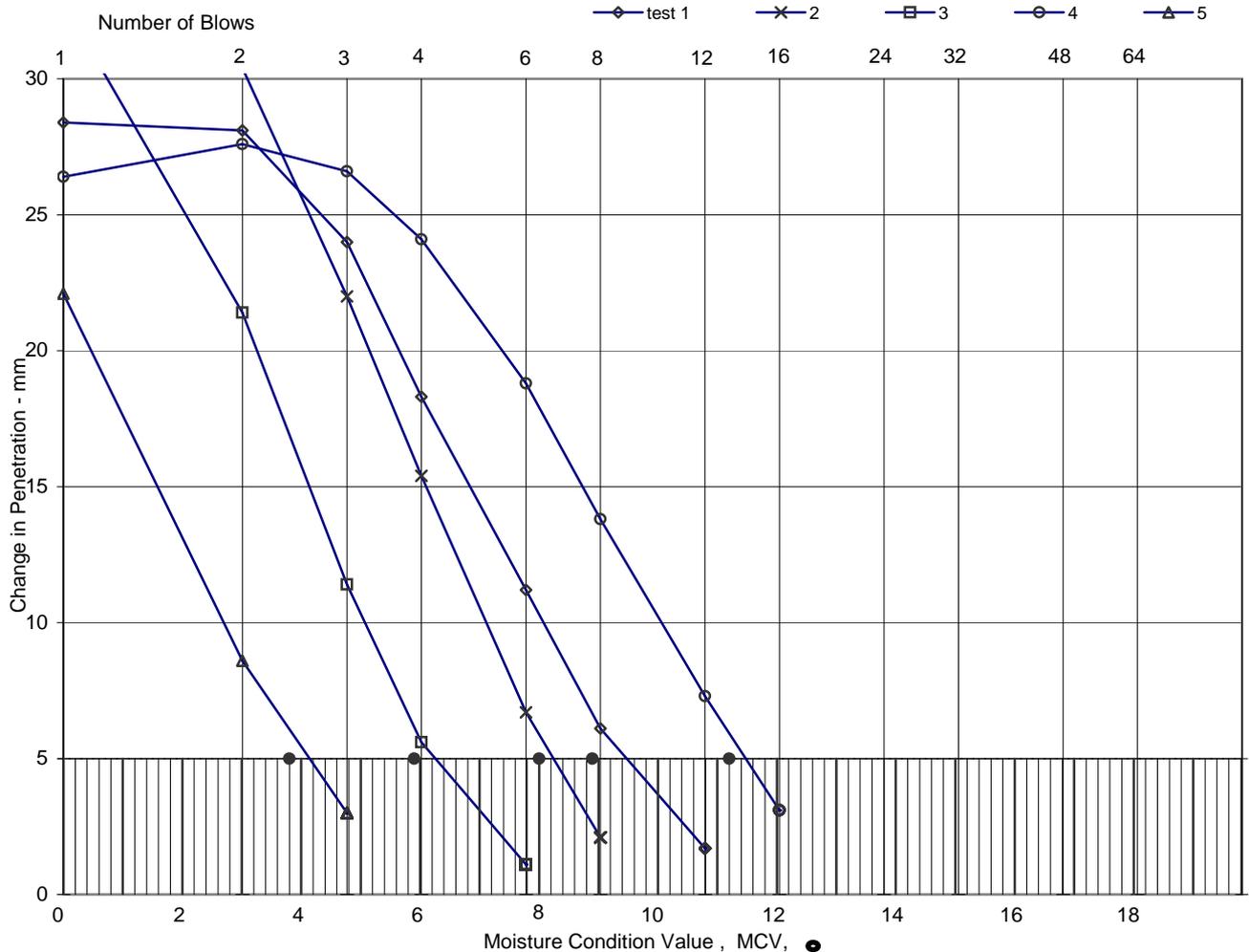


Printed:18/11/2011 17:35

Figure  
**MCVREL 2**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	8
			Type	B
			ID	ESGA1077-1120111010000000009
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		8.9	8.0	5.9	11.2	3.8
Moisture Content	%	15.4	15.9	17.3	14.2	18.3
Bulk density after test	Mg/m <sup>3</sup>	2.19	2.16	2.11	2.21	2.09
Dry density after test	Mg/m <sup>3</sup>	1.90	1.86	1.80	1.94	1.77

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	15.4
Material retained on 20mm sieve	%	4

Method of determining MCV
Steepest straight line

**QA Ref**  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

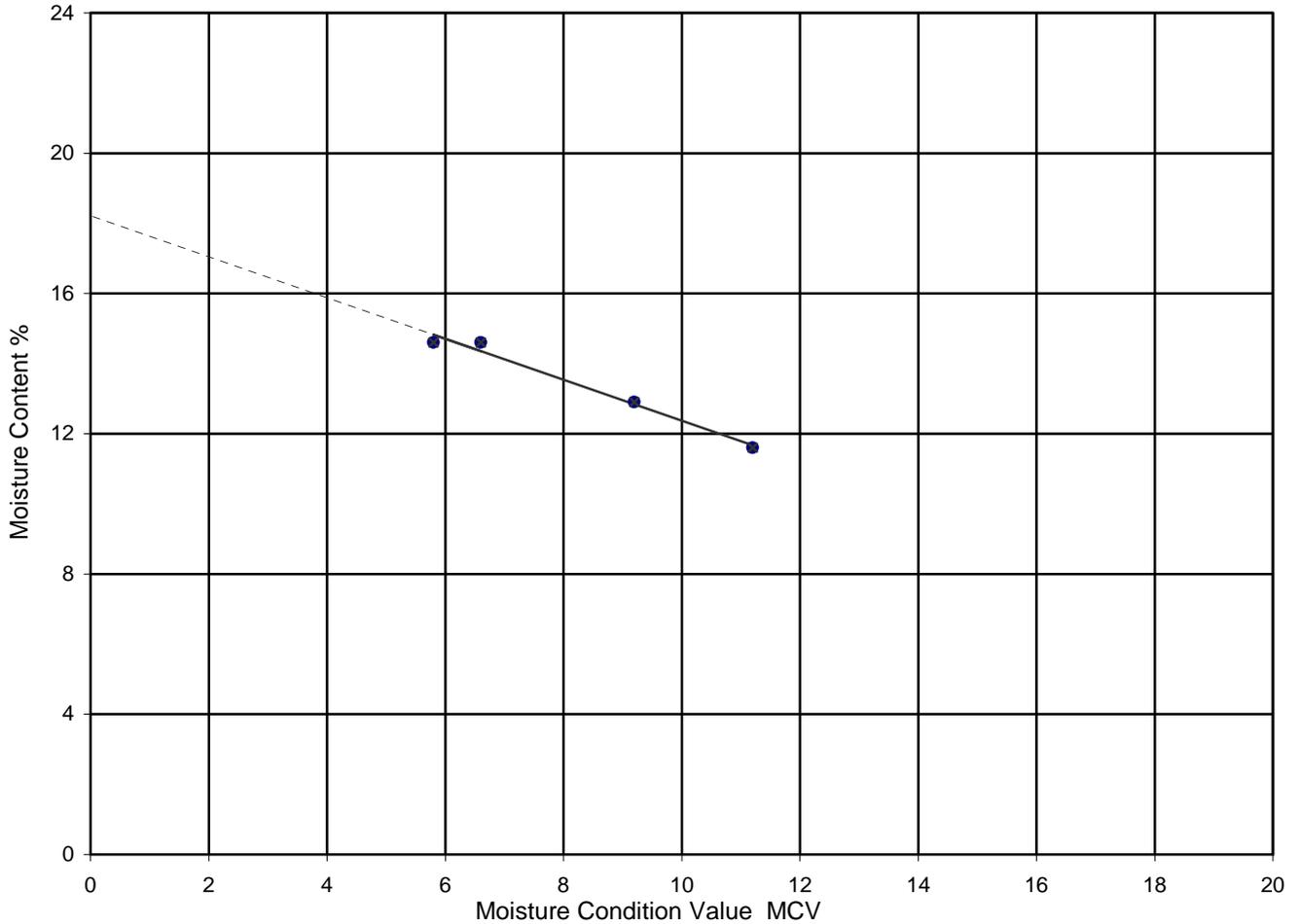
**Figure**  
**MCVREL 2**  
sheet 2 of 2

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	12	Type	B
			ID	ESGA1077-11201110100000000013		
			Spec Ref			

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



**Characteristics of calibration line (determined using linear regression)**

Intercept	18.2
Slope	-0.58
Sensitivity ( Change in MCV per 1% moisture content )	1.71
Correlation ( proximity of test points to regression line )	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

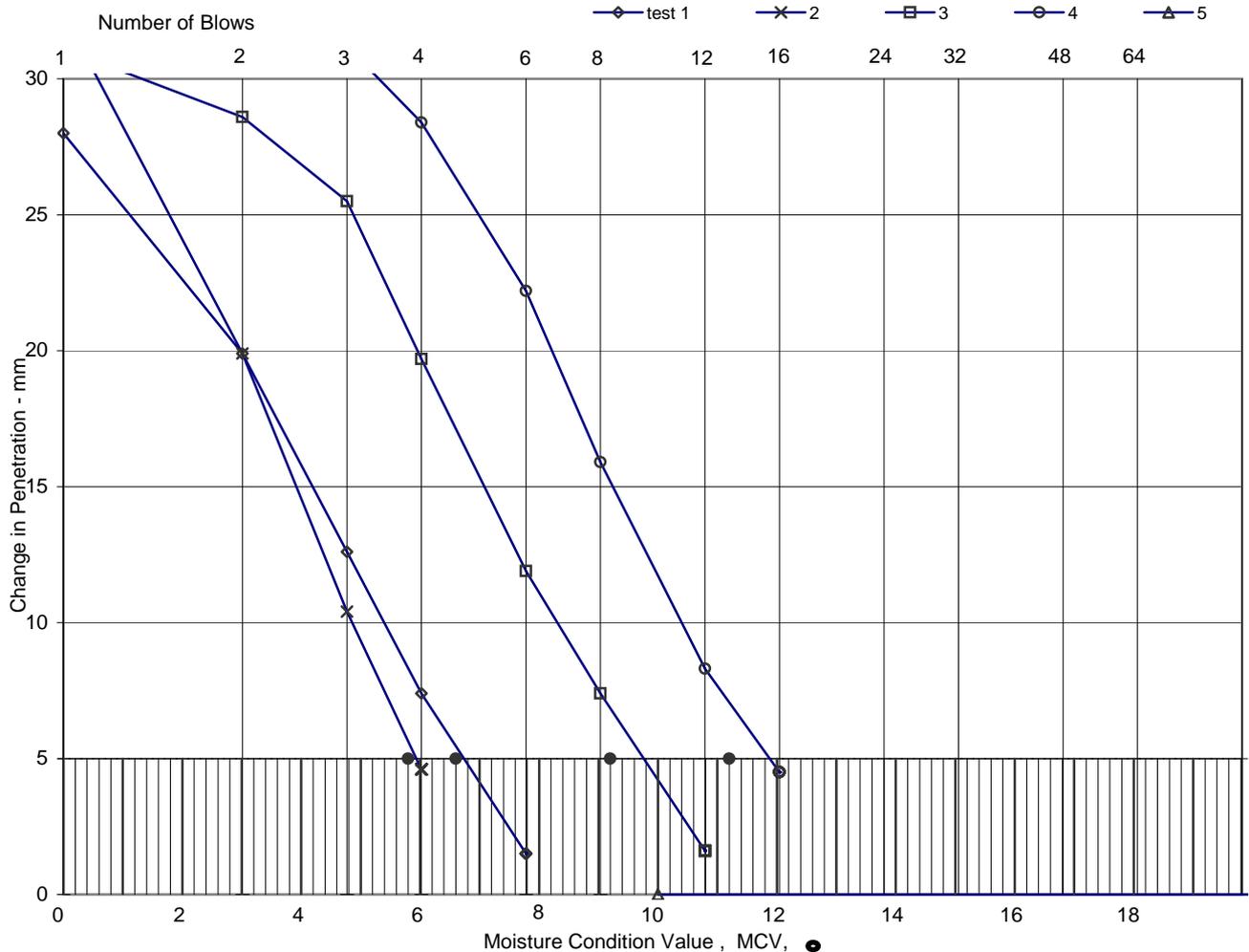


Printed:18/11/2011 17:35

Figure  
**MCVREL 3**  
sheet 1 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00
			Samp No	12
			Type	B
			ID	ESGA1077-11201110100000000013
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		6.6	5.8	9.2	11.2
Moisture Content	%	14.6	14.6	12.9	11.6
Bulk density after test	Mg/m <sup>3</sup>	2.20	2.19	2.25	2.29
Dry density after test	Mg/m <sup>3</sup>	1.92	1.91	1.99	2.05

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	14.6
Material retained on 20mm sieve	%	5

Method of determining MCV
Steepest straight line

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

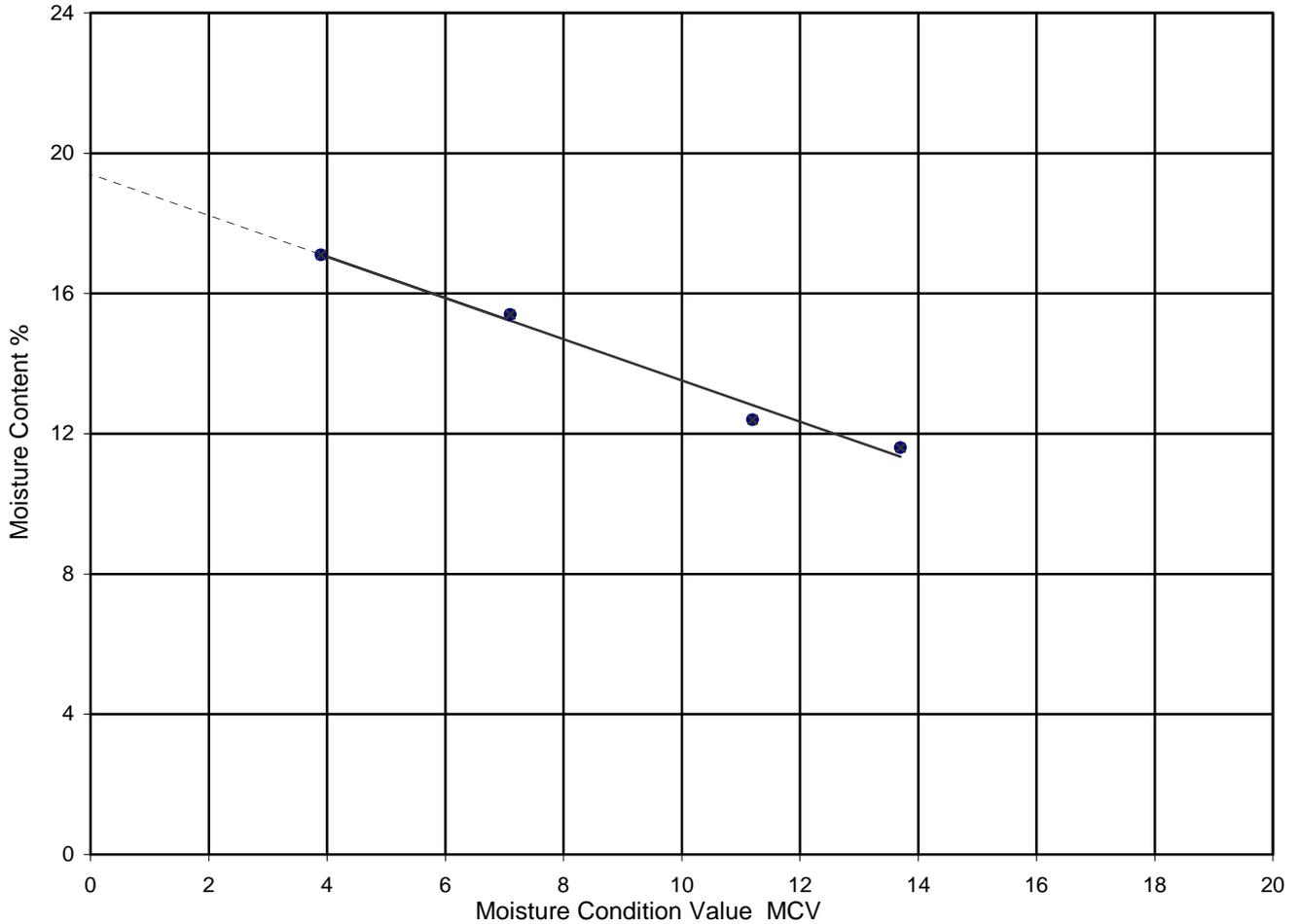
Figure  
**MCVREL 3**  
sheet 2 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.65		
			Samp No	16	Type	B
			ID	ESGA1077-11201110100000000017		
			Spec Ref			

### MCV CALIBRATION

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



#### Characteristics of calibration line (determined using linear regression)

Intercept	19.4
Slope	-0.59
Sensitivity ( Change in MCV per 1% moisture content )	1.70
Correlation ( proximity of test points to regression line )	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

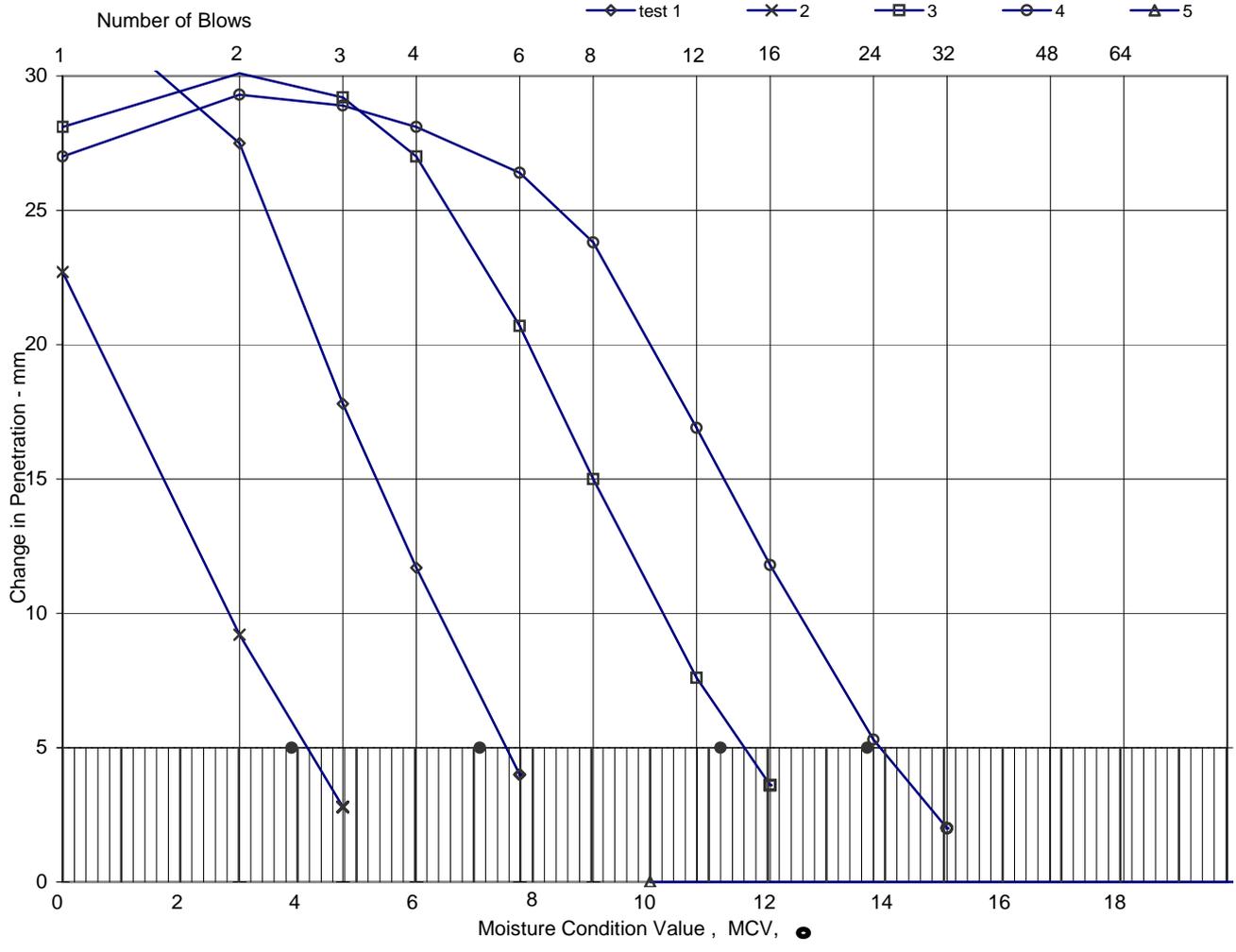


Printed: 18/11/2011 17:35

Figure  
**MCVREL 4**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH1
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	5.65
			Samp No	16
			Type	B
			ID	ESGA1077-11201110100000000017
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		7.1	3.9	11.2	13.7
Moisture Content	%	15.4	17.1	12.4	11.6
Bulk density after test	Mg/m <sup>3</sup>	2.21	2.13	2.28	2.30
Dry density after test	Mg/m <sup>3</sup>	1.92	1.82	2.03	2.06

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	15.4
Material retained on 20mm sieve	%	3.7

Method of determining MCV
Steepest straight line

**QA Ref**  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed: 18/11/2011 17:35

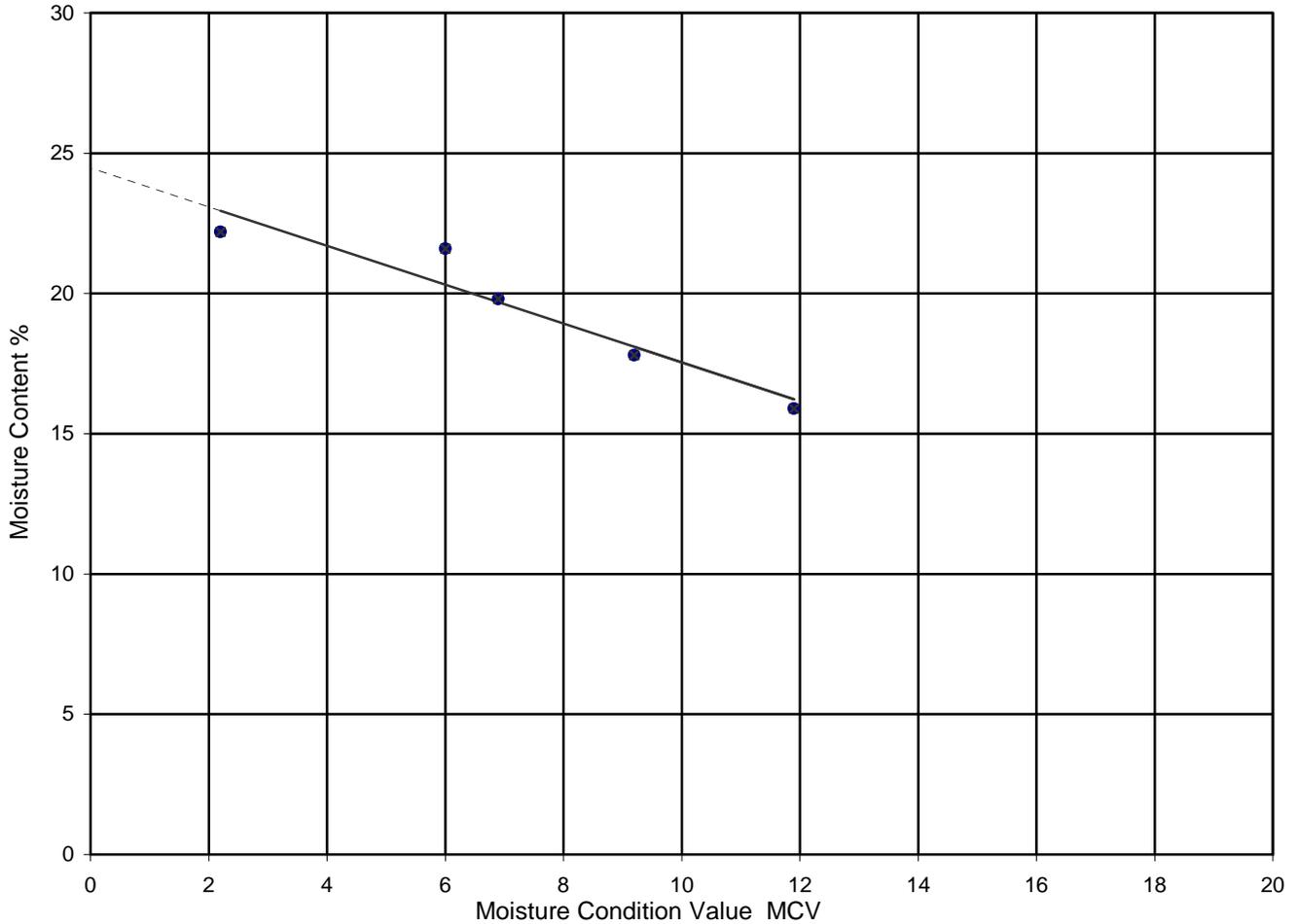
**Figure**  
**MCVREL 4**  
sheet 2 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-1120111010000000069		
			Spec Ref			

### MCV CALIBRATION

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



#### Characteristics of calibration line (determined using linear regression)

Intercept	24.5
Slope	-0.69
Sensitivity ( Change in MCV per 1% moisture content )	1.44
Correlation ( proximity of test points to regression line )	-0.96
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

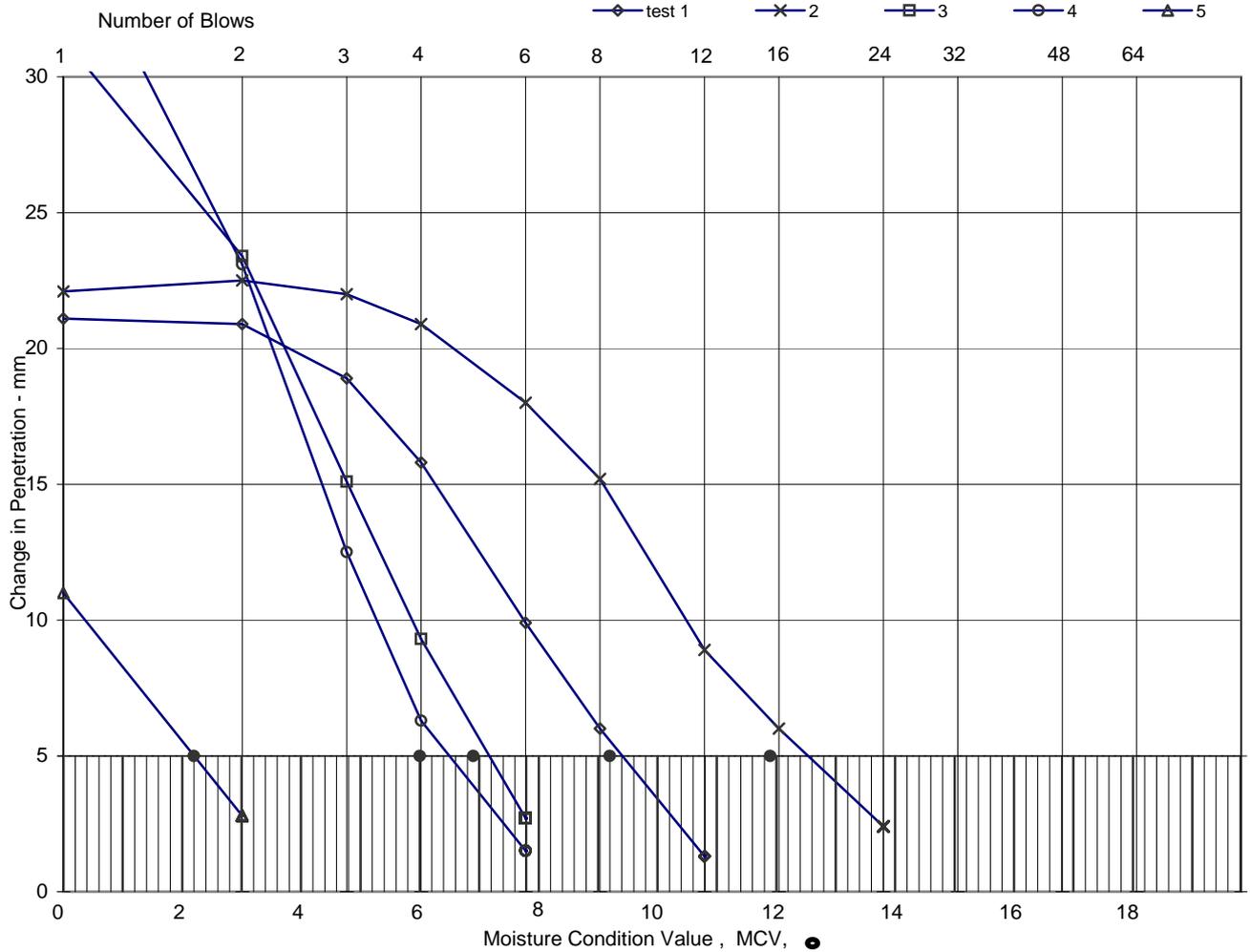


Printed: 18/11/2011 17:35

Figure  
**MCVREL 5**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50
			Samp No	3
			Type	B
			ID	ESGA1077-11201110100000000069
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		9.2	11.9	6.9	6.0	2.2
Moisture Content	%	17.8	15.9	19.8	21.6	22.2
Bulk density after test	Mg/m <sup>3</sup>	2.09	2.16	2.05	2.02	1.95
Dry density after test	Mg/m <sup>3</sup>	1.77	1.86	1.71	1.66	1.60

Soil description	Brown slightly gravelly sandy silty CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	17.8
Material retained on 20mm sieve	%	0

Method of determining MCV	Steepest straight line
---------------------------	------------------------

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

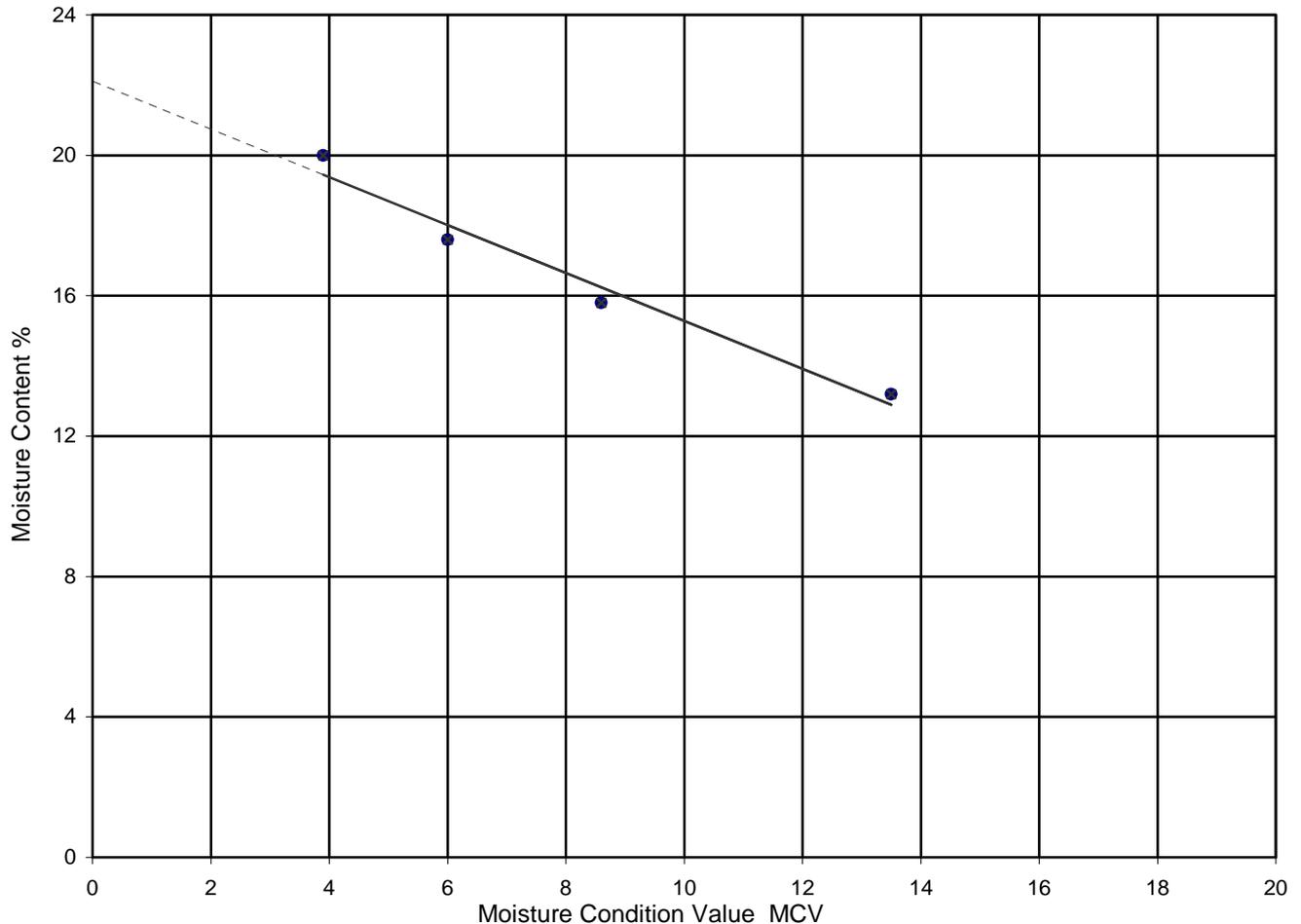
Figure  
**MCVREL 5**  
sheet 2 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00		
			Samp No	7	Type	B
			ID	ESGA1077-11201110100000000073		
			Spec Ref			

### MCV CALIBRATION

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



#### Characteristics of calibration line (determined using linear regression)

Intercept	22.1
Slope	-0.68
Sensitivity ( Change in MCV per 1% moisture content )	1.46
Correlation ( proximity of test points to regression line )	-0.98
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

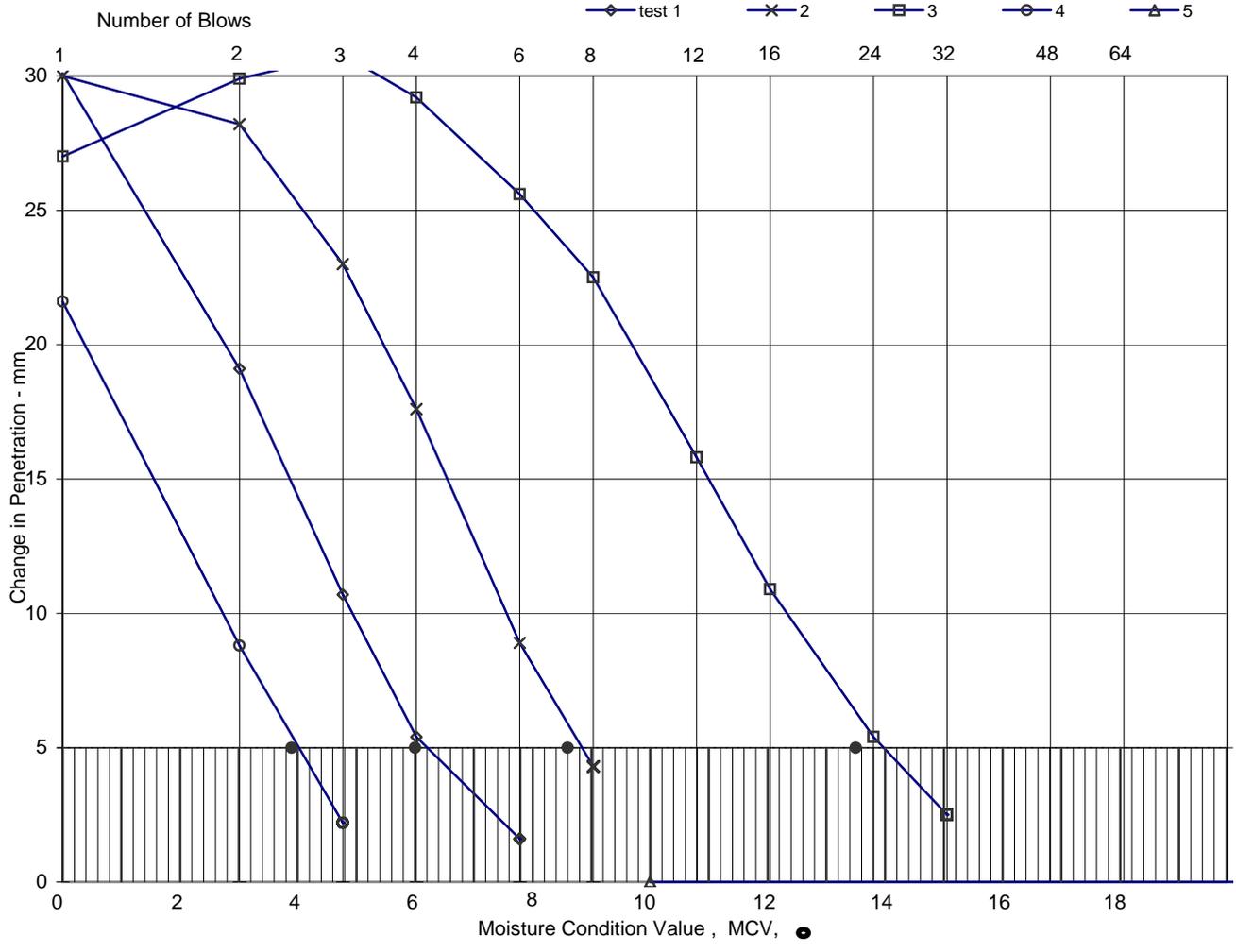


Printed: 18/11/2011 17:35

Figure  
**MCVREL 6**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	2.00
			Samp No	7
			Type	B
			ID	ESGA1077-11201110100000000073
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		6.0	8.6	13.5	3.9
Moisture Content	%	17.6	15.8	13.2	20.0
Bulk density after test	Mg/m <sup>3</sup>	2.09	2.15	2.23	2.04
Dry density after test	Mg/m <sup>3</sup>	1.78	1.86	1.97	1.70

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	17.6
Material retained on 20mm sieve	%	2

Method of determining MCV
Steepest straight line

**QA Ref**  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

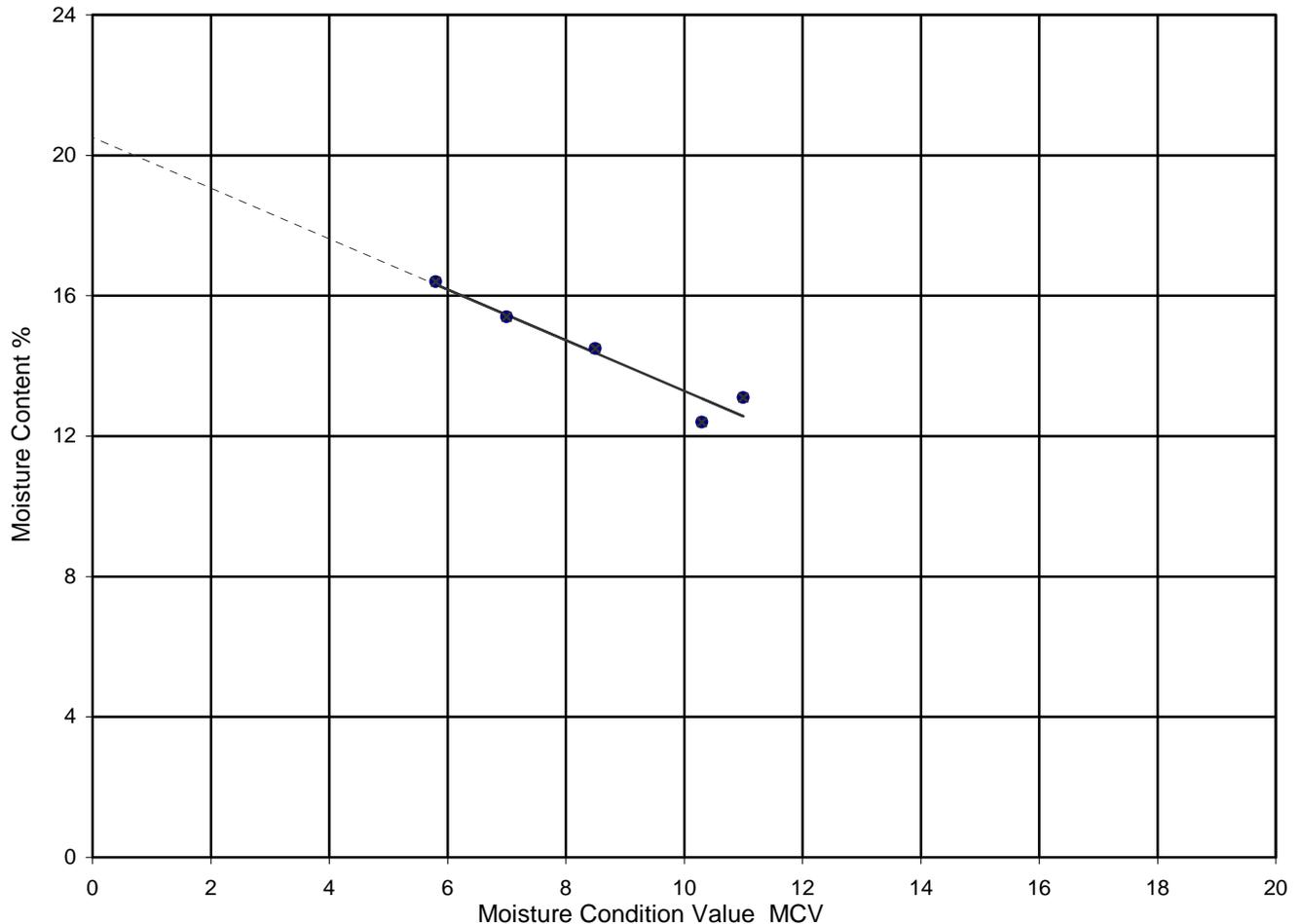
**Figure**  
**MCVREL 6**  
sheet 2 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00		
			Samp No	11	Type	B
			ID	ESGA1077-11201110100000000077		
			Spec Ref			

### MCV CALIBRATION

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



#### Characteristics of calibration line (determined using linear regression)

Intercept	20.5
Slope	-0.72
Sensitivity ( Change in MCV per 1% moisture content )	1.38
Correlation ( proximity of test points to regression line )	-0.96
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

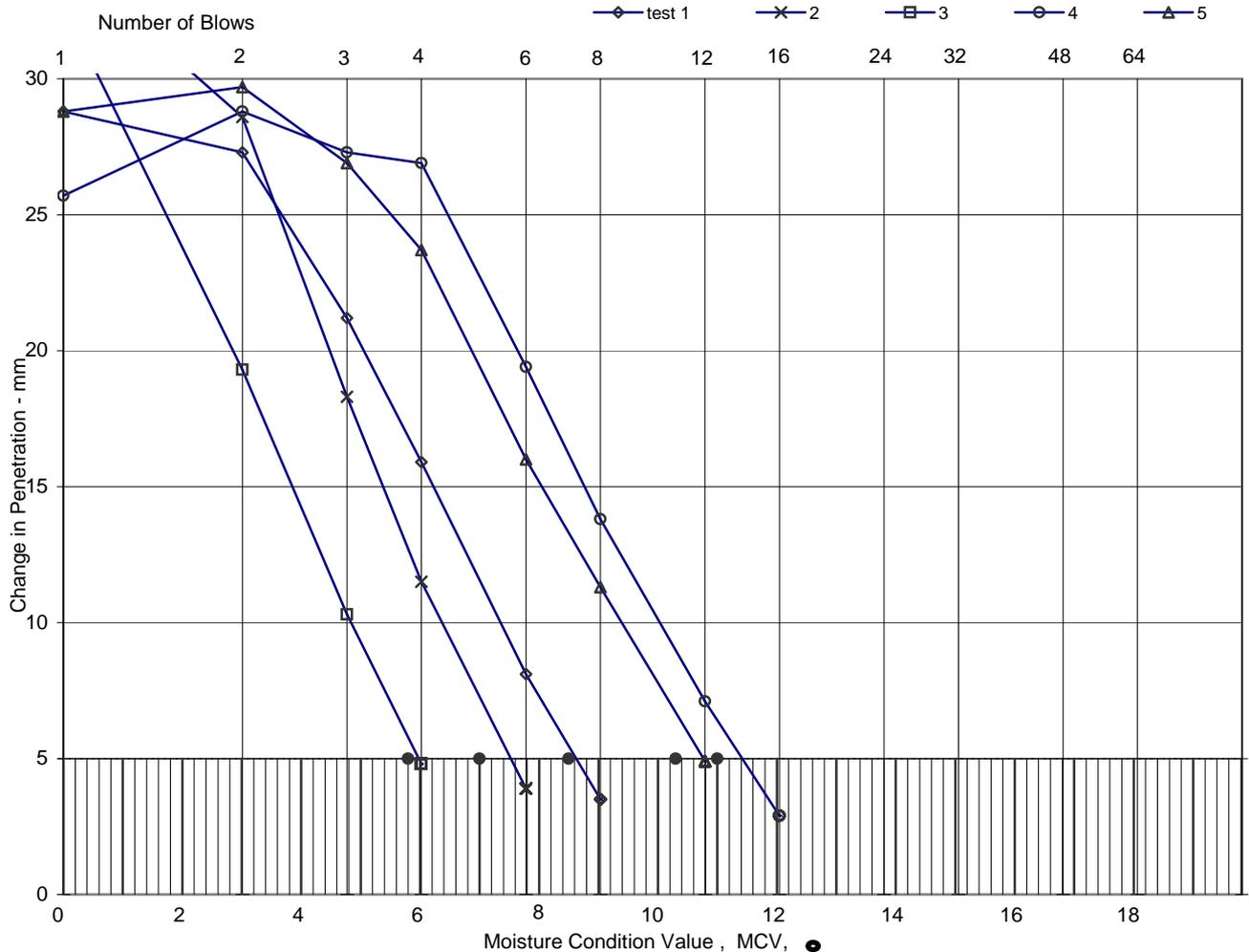


Printed: 18/11/2011 17:35

Figure  
**MCVREL 7**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.00
			Samp No	11
			Type	B
			ID	ESGA1077-11201110100000000077
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		8.5	7.0	5.8	11.0	10.3
Moisture Content	%	14.5	15.4	16.4	13.1	12.4
Bulk density after test	Mg/m <sup>3</sup>	2.21	2.17	2.15	2.24	2.23
Dry density after test	Mg/m <sup>3</sup>	1.93	1.88	1.85	1.98	1.98

Soil description	Gryeish brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	14.5
Material retained on 20mm sieve	%	1.7

Method of determining MCV
Steepest straight line

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

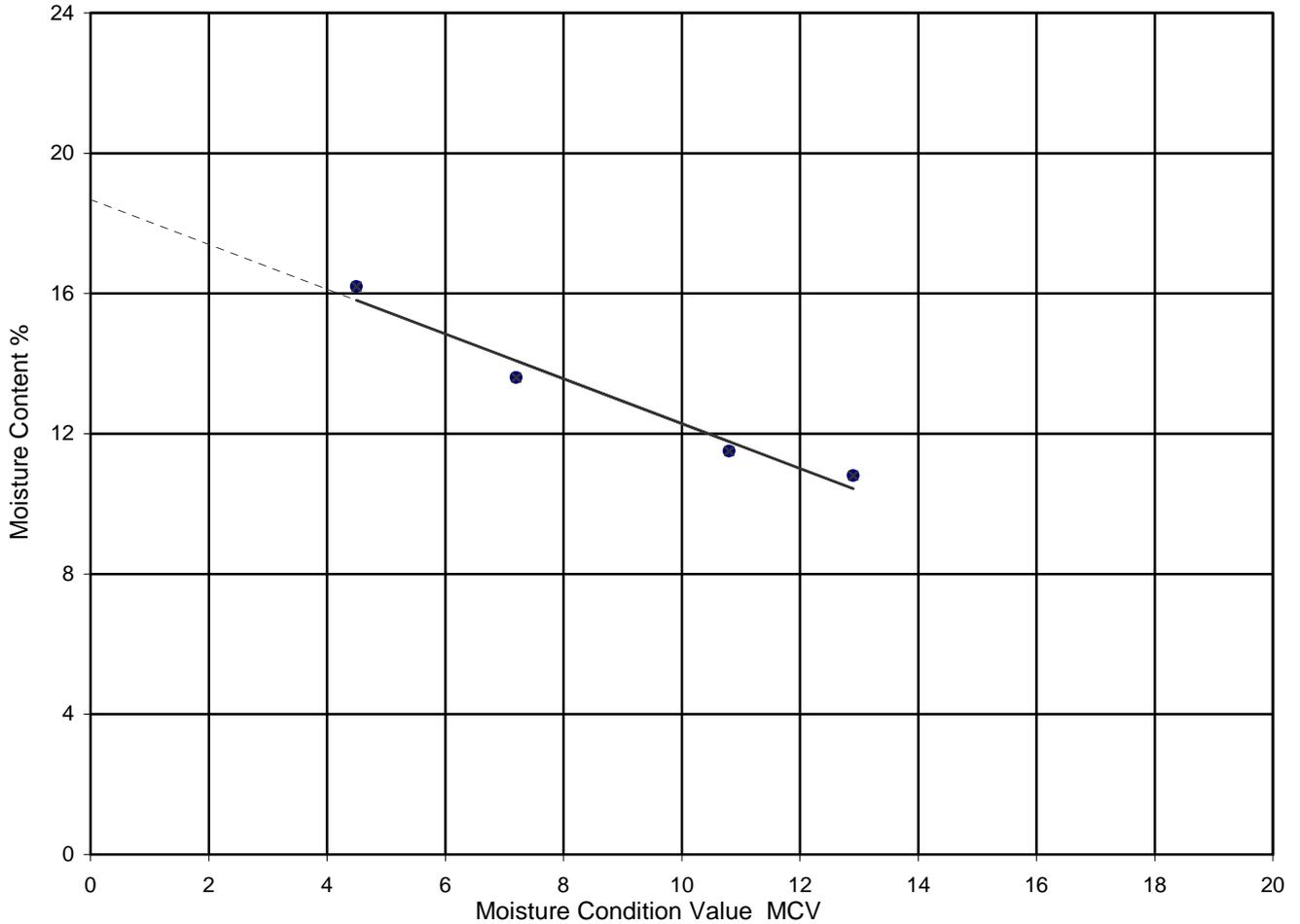
Figure  
**MCVREL 7**  
sheet 2 of 2

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH2		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.00		
			Samp No	15	Type	B
			ID	ESGA1077-11201110100000000081		
			Spec Ref			

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



**Characteristics of calibration line (determined using linear regression)**

Intercept	18.7
Slope	-0.64
Sensitivity ( Change in MCV per 1% moisture content )	1.56
Correlation ( proximity of test points to regression line )	-0.98
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

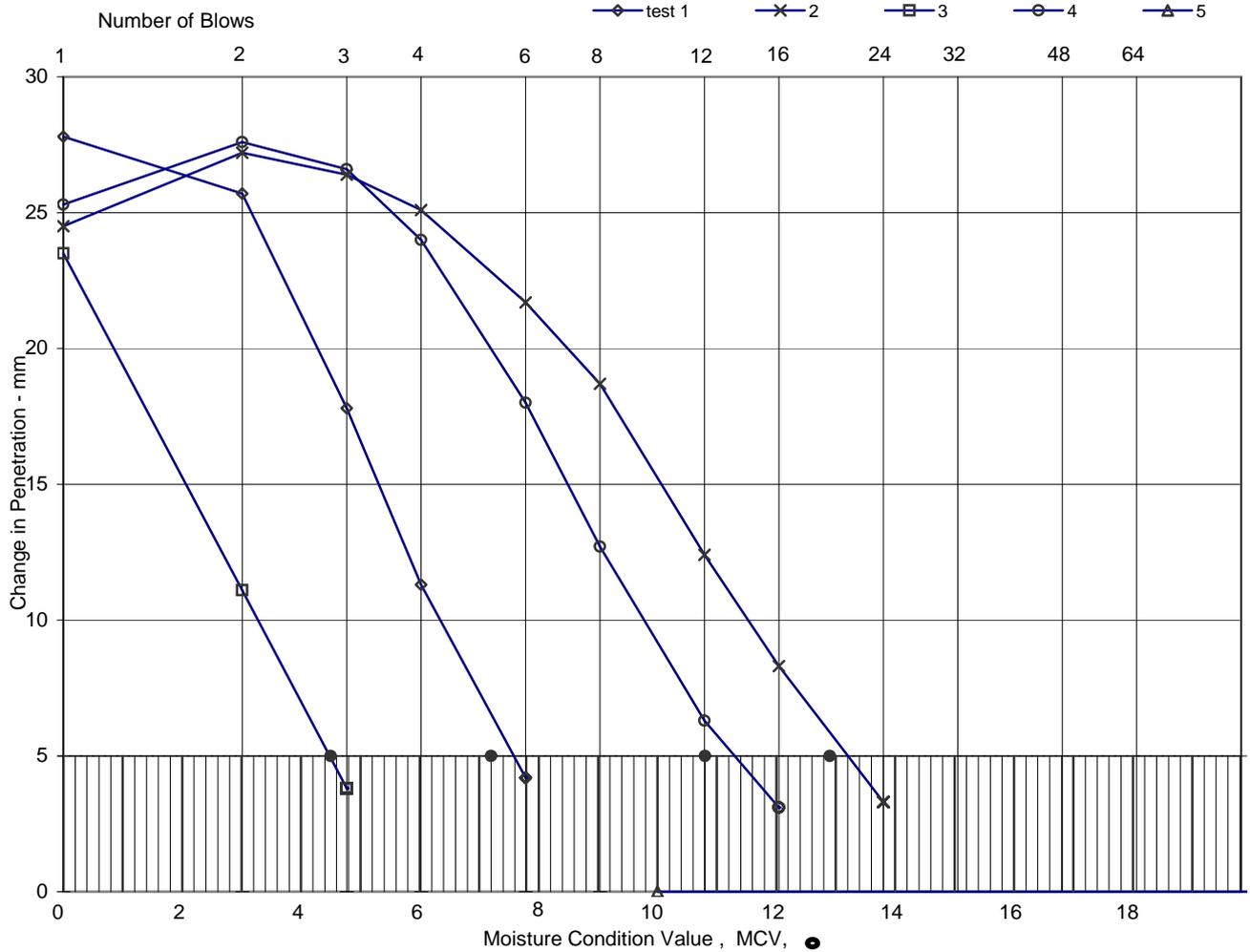


Printed:18/11/2011 17:35

Figure  
**MCVREL 8**  
sheet 1 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH2
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.00
			Samp No	15
			Type	B
			ID	ESGA1077-11201110100000000081
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		7.2	12.9	4.5	10.8
Moisture Content	%	13.6	10.8	16.2	11.5
Bulk density after test	Mg/m <sup>3</sup>	2.20	2.30	2.14	2.27
Dry density after test	Mg/m <sup>3</sup>	1.94	2.08	1.84	2.04

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	13.6
Material retained on 20mm sieve	%	1.2

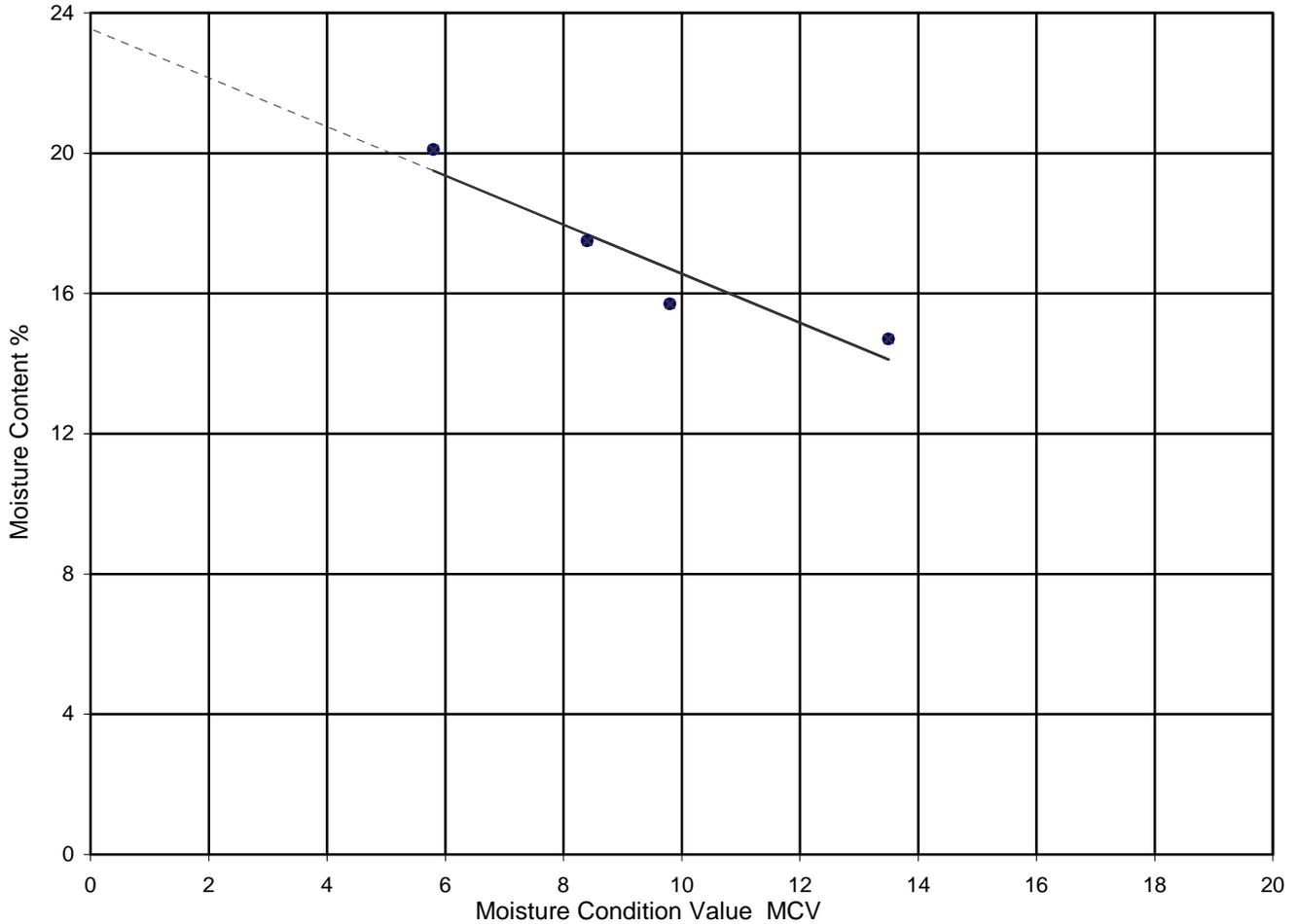
Method of determining MCV
Steepest straight line

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50		
			Samp No	3	Type	B
			ID	ESGA1077-11201110110000000128		
			Spec Ref			

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - - extended regression



Characteristics of calibration line (determined using linear regression)	
Intercept	23.6
Slope	-0.70
Sensitivity ( Change in MCV per 1% moisture content )	1.43
Correlation ( proximity of test points to regression line )	-0.95
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

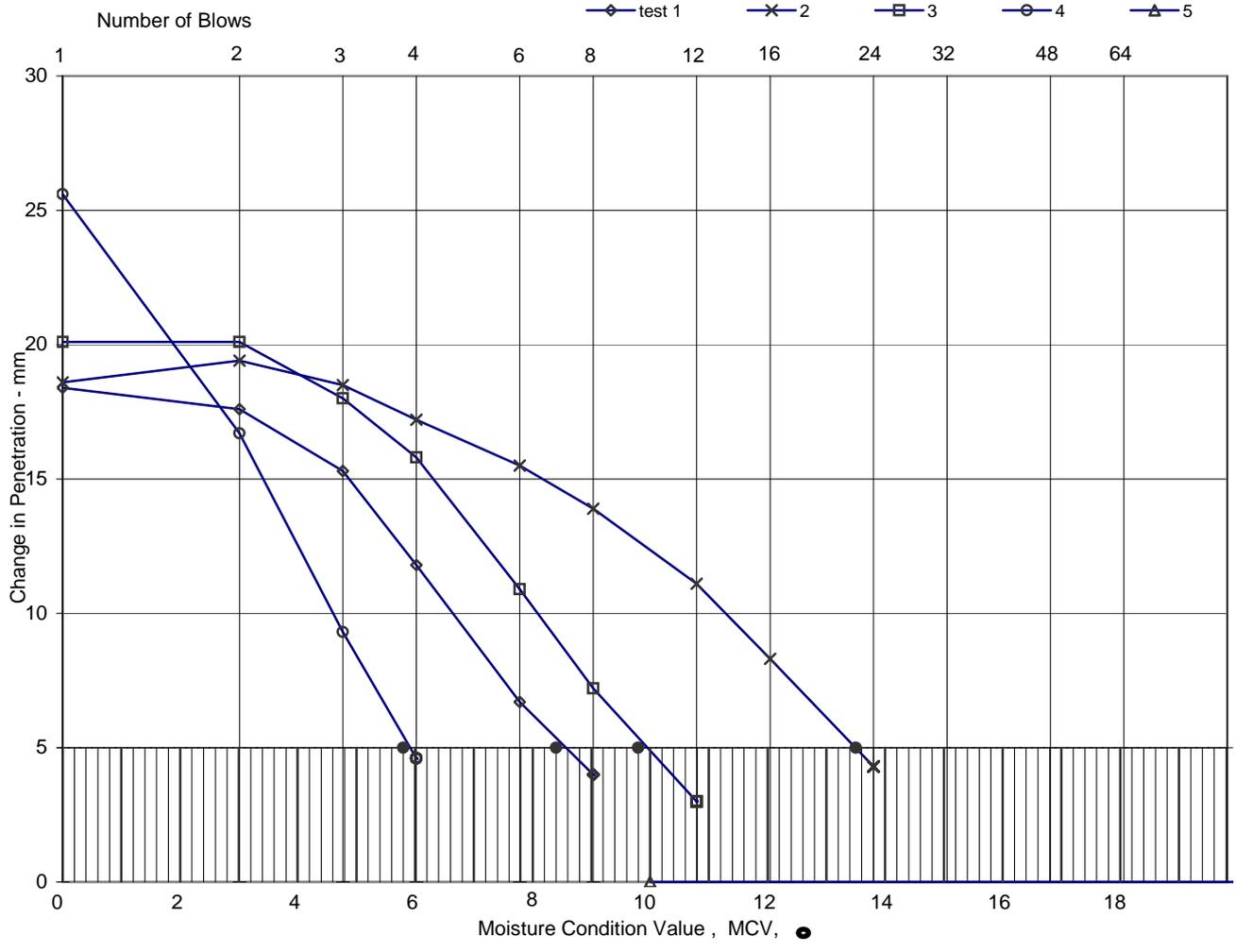


Printed:18/11/2011 17:35

Figure  
**MCVREL 9**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	0.50
			Samp No	3
			Type	B
			ID	ESGA1077-1120111011000000128
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		8.4	13.5	9.8	5.8
Moisture Content	%	17.5	14.7	15.7	20.1
Bulk density after test	Mg/m <sup>3</sup>	2.05	2.16	2.10	2.02
Dry density after test	Mg/m <sup>3</sup>	1.74	1.88	1.82	1.68

Soil description	Brown slightly gravelly sandy CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	17.5
Material retained on 20mm sieve	%	0

Method of determining MCV
Steepest straight line

**QA Ref**  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed: 18/11/2011 17:35

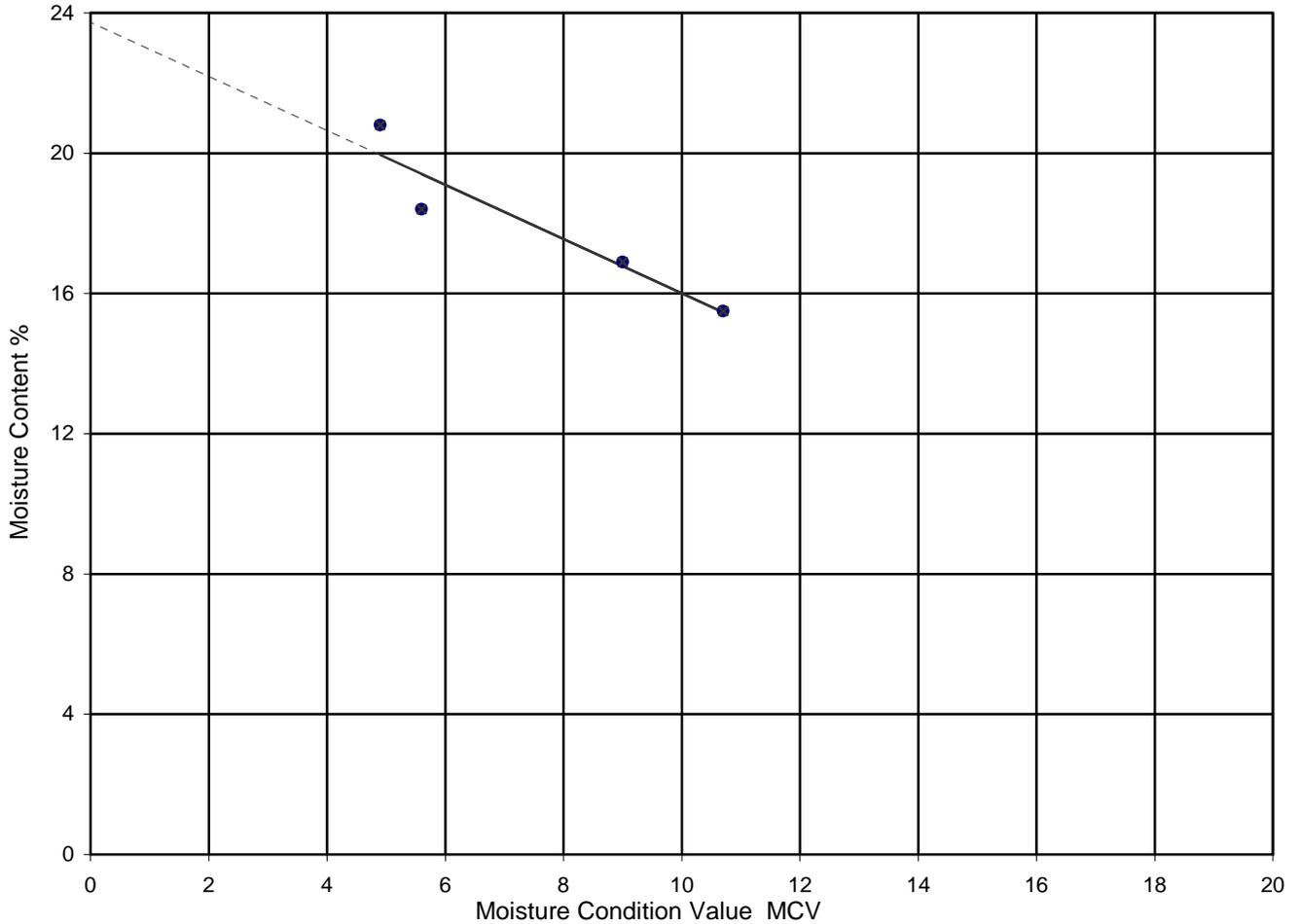
**Figure**  
**MCVREL 9**  
sheet 2 of 2

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.85		
			Samp No	7	Type	B
			ID	ESGA1077-11201110110000000132		
			Spec Ref			

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



**Characteristics of calibration line (determined using linear regression)**

Intercept	23.7
Slope	-0.77
Sensitivity ( Change in MCV per 1% moisture content )	1.29
Correlation ( proximity of test points to regression line )	-0.94
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

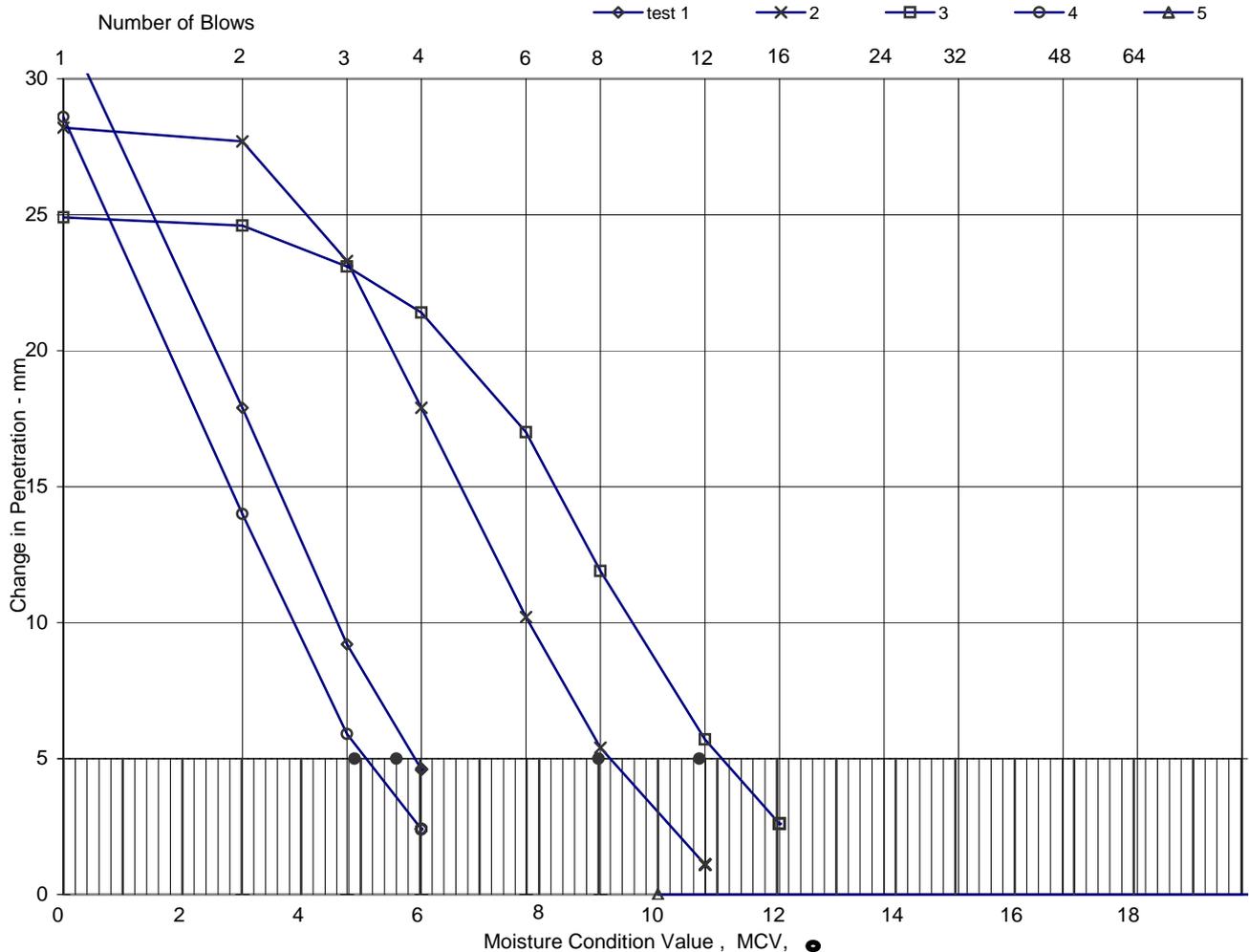


Printed:18/11/2011 17:35

Figure  
**MCVREL 10**  
sheet 1 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	1.85
			Samp No	7
			Type	B
			ID	ESGA1077-1120111011000000132
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	*
Moisture Condition Value		5.6	9.0	10.7	4.9	
Moisture Content	%	18.4	16.9	15.5	20.8	
Bulk density after test	Mg/m <sup>3</sup>	2.07	2.13	2.16	2.04	
Dry density after test	Mg/m <sup>3</sup>	1.75	1.82	1.87	1.69	

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	18.4
Material retained on 20mm sieve	%	1

Method of determining MCV	Steepest straight line
---------------------------	------------------------

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

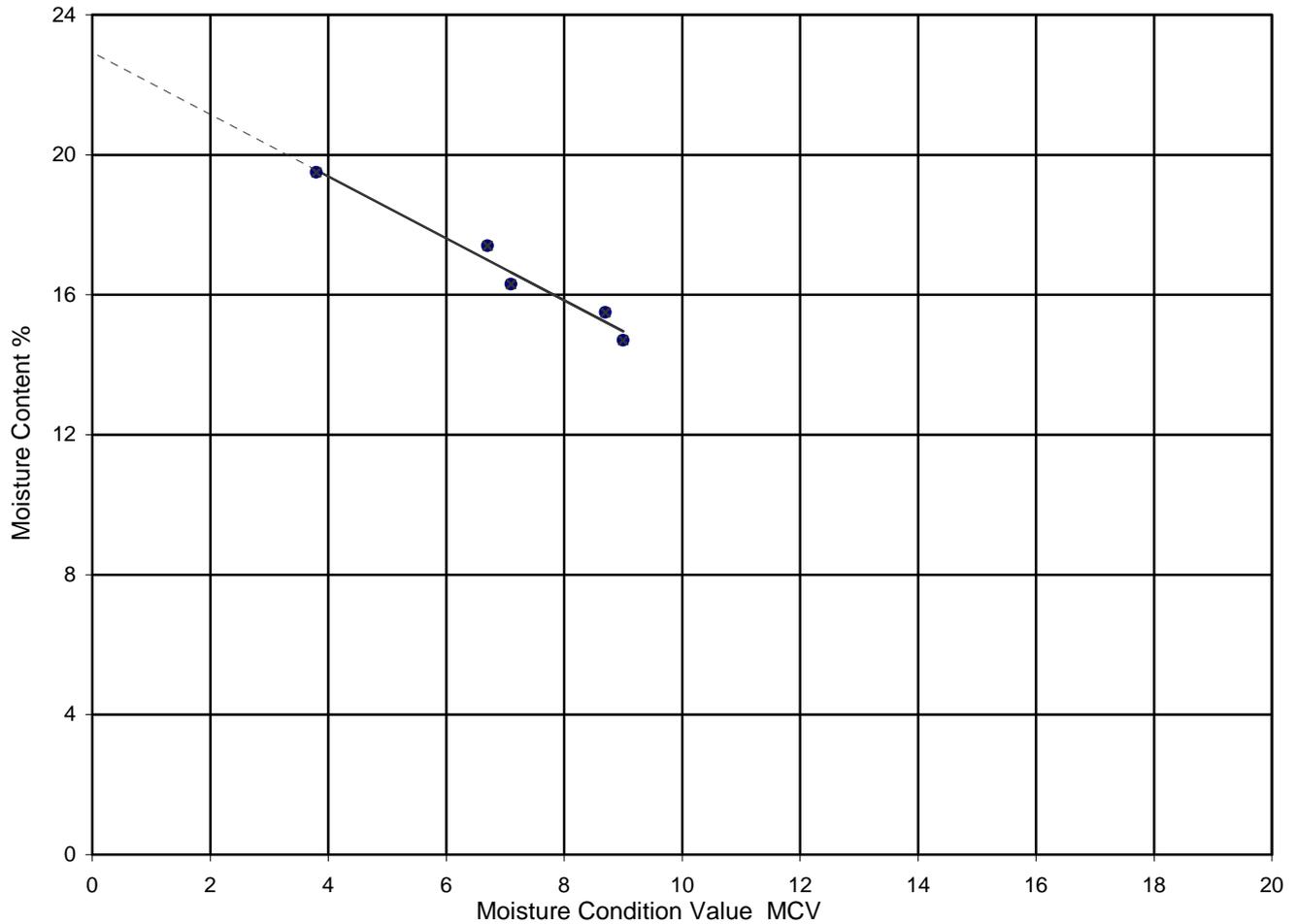
Figure  
**MCVREL 10**  
sheet 2 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.15		
			Samp No	11	Type	B
			ID	ESGA1077-11201110110000000136		
			Spec Ref			

### MCV CALIBRATION

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



#### Characteristics of calibration line (determined using linear regression)

Intercept	22.9
Slope	-0.89
Sensitivity ( Change in MCV per 1% moisture content )	1.13
Correlation ( proximity of test points to regression line )	-0.98
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

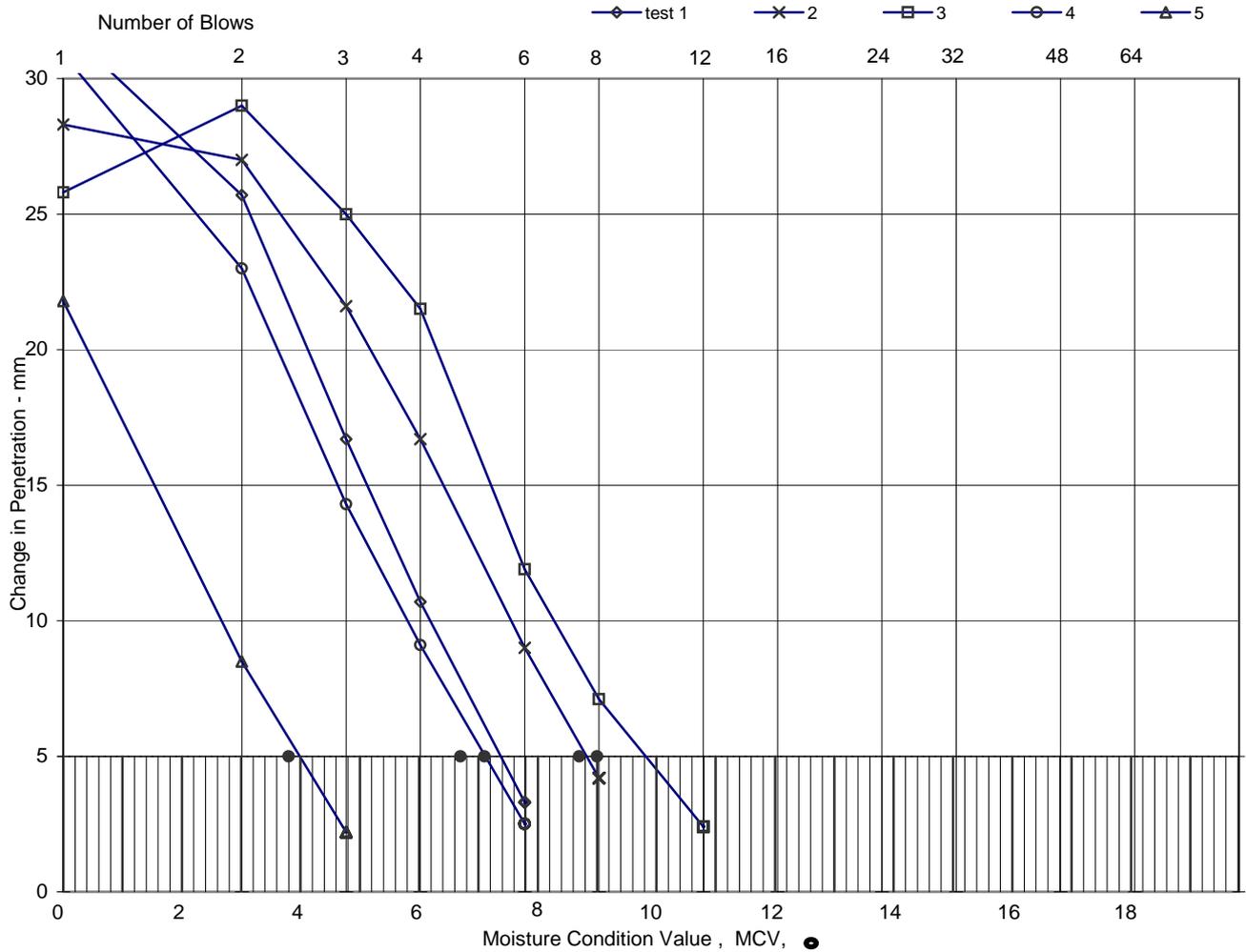


Printed: 18/11/2011 17:35

Figure  
**MCVREL 11**  
sheet 1 of 2

# MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	3.15
			Samp No	11
			Type	B
			ID	ESGA1077-1120111011000000136
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4	5
Moisture Condition Value		7.1	8.7	9.0	6.7	3.8
Moisture Content	%	16.3	15.5	14.7	17.4	19.5
Bulk density after test	Mg/m <sup>3</sup>	2.14	2.17	2.19	2.12	2.05
Dry density after test	Mg/m <sup>3</sup>	1.84	1.88	1.91	1.81	1.72

Soil description	Reddish brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	16.3
Material retained on 20mm sieve	%	0

Method of determining MCV
Steepest straight line

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

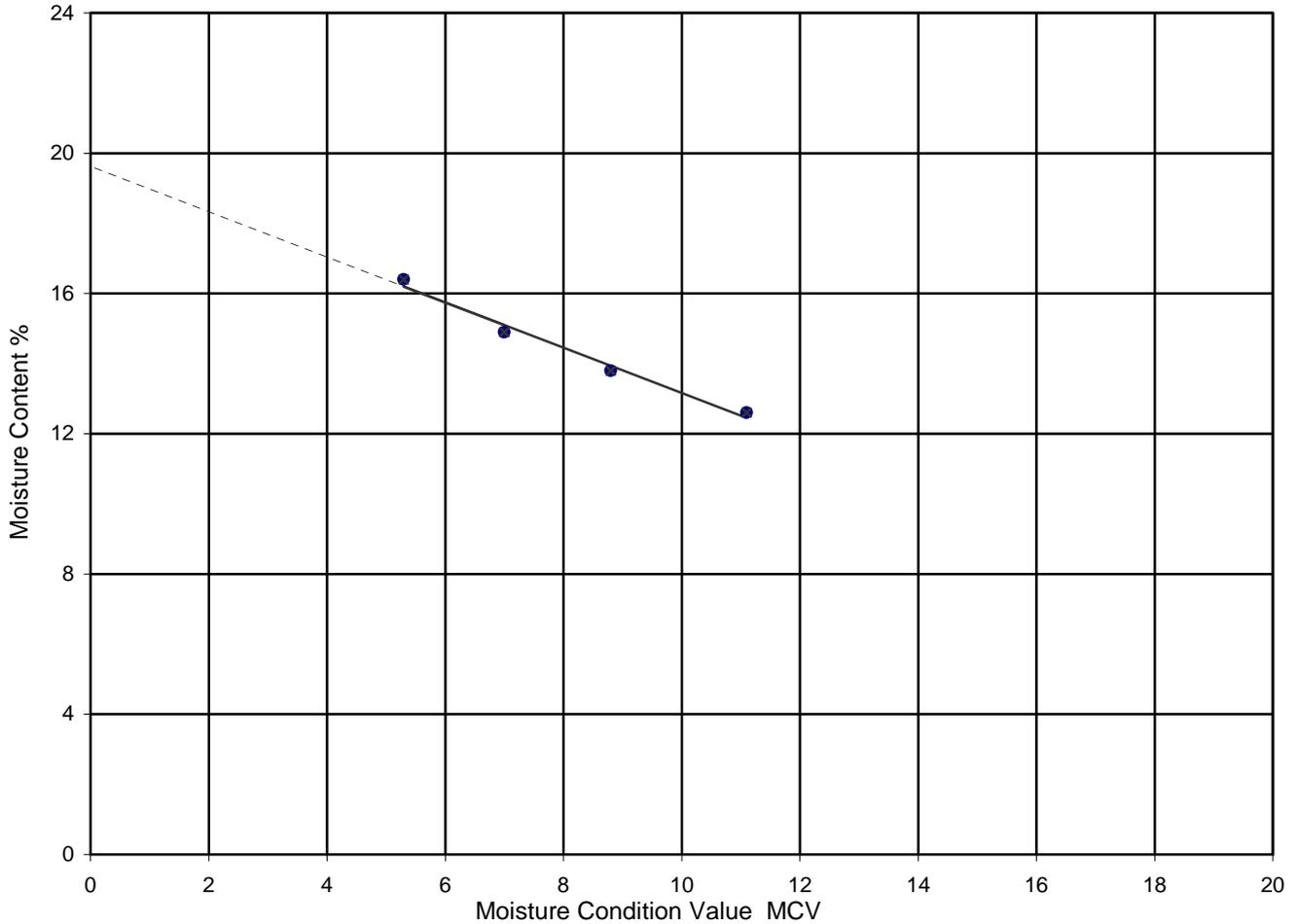
Figure  
**MCVREL 11**  
sheet 2 of 2

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH3			
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65			
			Samp No	15	Type	B	
			ID	ESGA1077-11201110110000000140			
			Spec Ref				

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



**Characteristics of calibration line (determined using linear regression)**

Intercept	19.6
Slope	-0.65
Sensitivity ( Change in MCV per 1% moisture content )	1.55
Correlation ( proximity of test points to regression line )	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

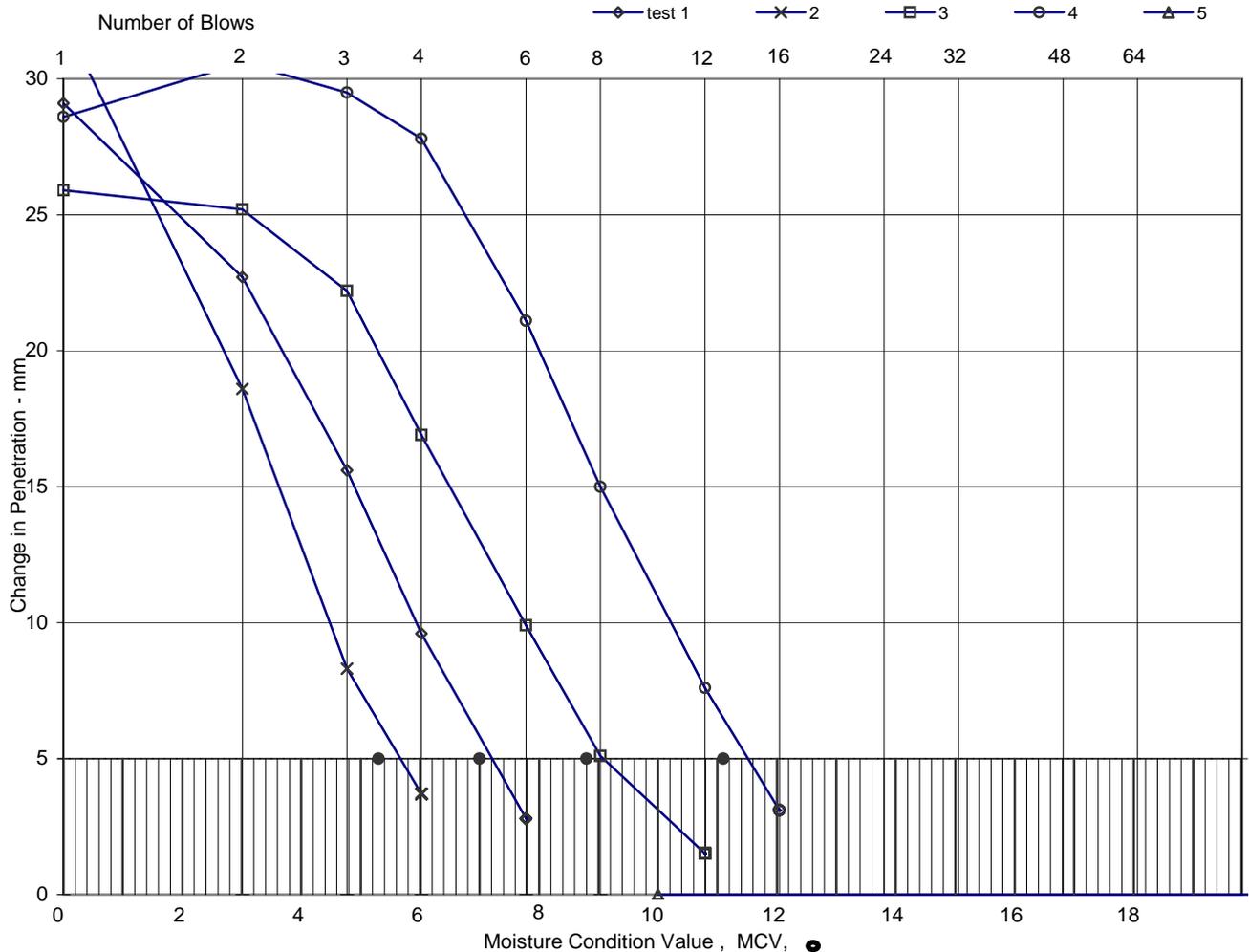


Printed:18/11/2011 17:35

Figure  
**MCVREL 12**  
sheet 1 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	4.65
			Samp No	15
			Type	B
			ID	ESGA1077-1120111011000000140
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		7.0	5.3	8.8	11.1
Moisture Content	%	14.9	16.4	13.8	12.6
Bulk density after test	Mg/m <sup>3</sup>	2.20	2.15	2.24	2.26
Dry density after test	Mg/m <sup>3</sup>	1.91	1.85	1.97	2.01

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	14.9
Material retained on 20mm sieve	%	1

Method of determining MCV
Steepest straight line

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

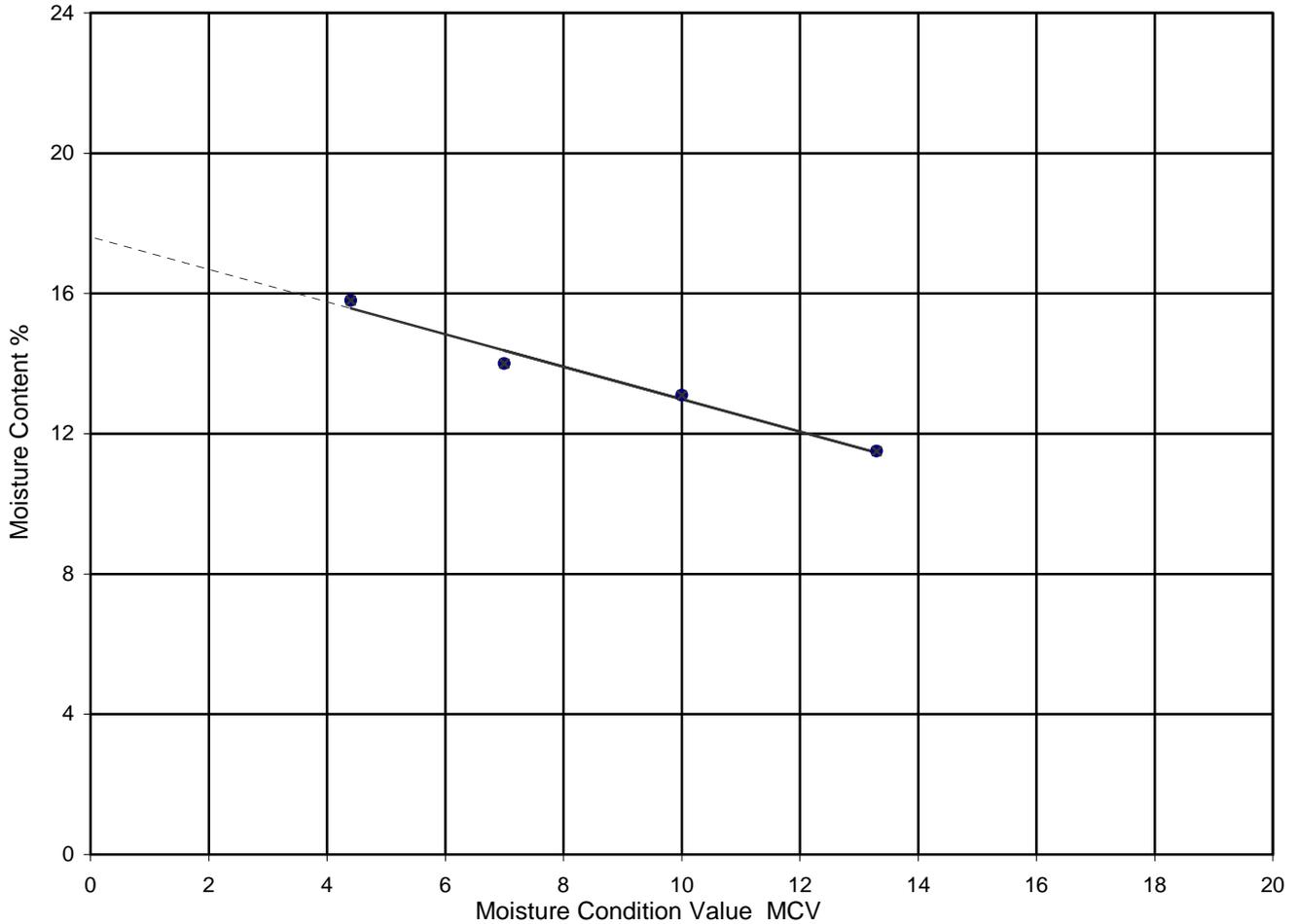
Figure  
**MCVREL 12**  
sheet 2 of 2

**MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP  
BS1377:PART 4 : 1990 : CLAUSE 5.5**

Project No	A1077-11	Sample Details:	Hole No	BH3		
Project Name	SANDESEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.15		
			Samp No	19	Type	B
			ID	ESGA1077-11201110110000000144		
			Spec Ref			

**MCV CALIBRATION**

- valid points
- × ineffective/invalid
- regression
- - - - extended regression



**Characteristics of calibration line (determined using linear regression)**

Intercept	17.6
Slope	-0.46
Sensitivity ( Change in MCV per 1% moisture content )	2.16
Correlation ( proximity of test points to regression line )	-0.99
Method of interpretation of MCV	Steepest straight line

The above characteristics are NOT covered by UKAS accreditation to BS1377

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11

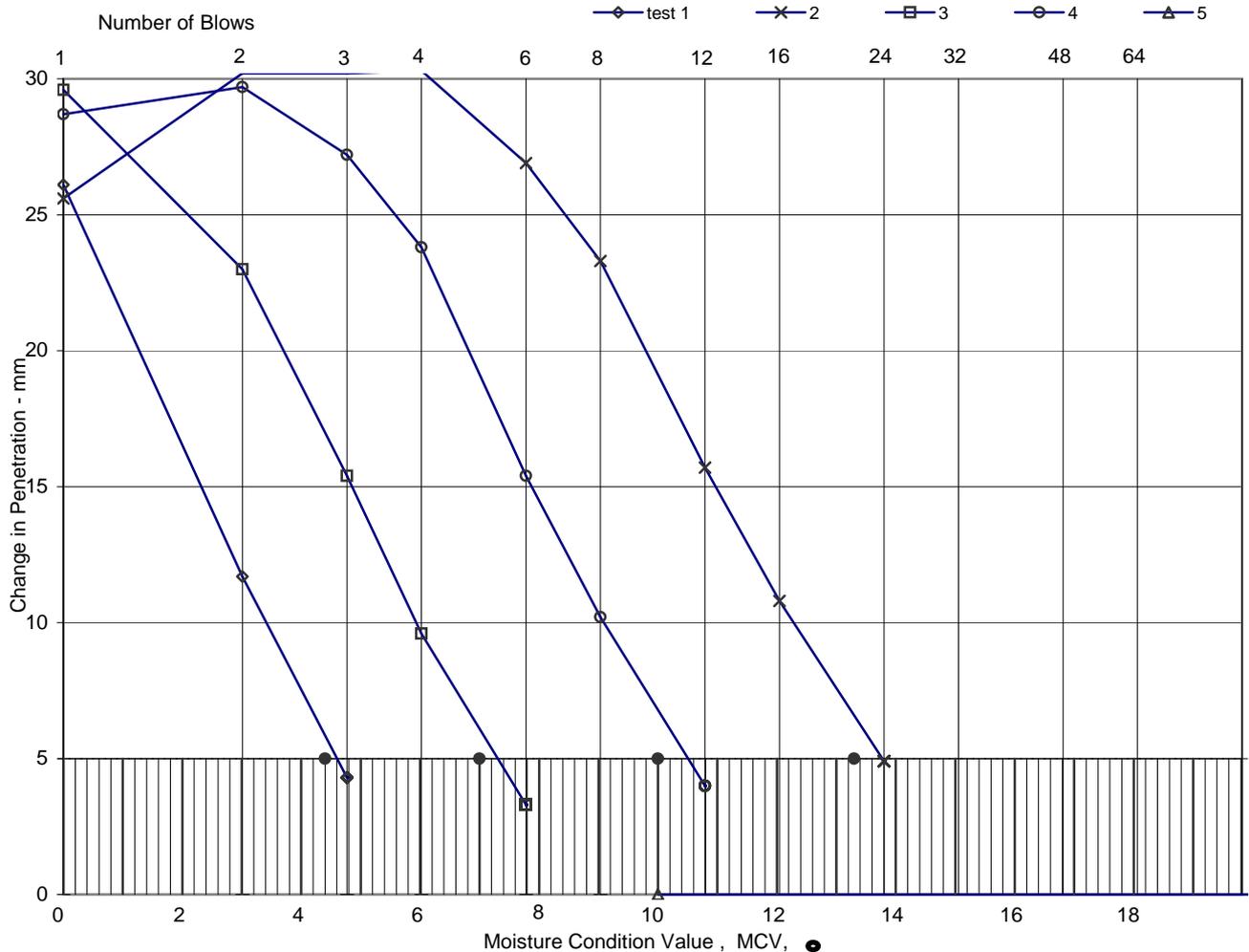


Printed:18/11/2011 17:35

Figure  
**MCVREL 13**  
sheet 1 of 2

## MOISTURE CONDITION VALUE ( MCV ) / MOISTURE CONTENT RELATIONSHIP BS1377:PART 4 : 1990 : CLAUSE 5.5

Project No	A1077-11	Sample Details:	Hole No	BH3
Project Name	SANDSEND BOREHOLES, NORTH YORKSHIRE		Depth (m BGL)	6.15
			Samp No	19
			Type	B
			ID	ESGA1077-11201110110000000144
			Spec Ref	



Test No	* ineffective / invalid point	1	2	3	4
Moisture Condition Value		4.4	13.3	7.0	10.0
Moisture Content	%	15.8	11.5	14.0	13.1
Bulk density after test	Mg/m <sup>3</sup>	2.12	2.31	2.19	2.26
Dry density after test	Mg/m <sup>3</sup>	1.83	2.07	1.92	2.00

Soil description	Brown slightly sandy slightly gravelly CLAY.
Procedure / Preparation	BS1377:Part 1 and Part 4, clause 5.5
Remarks	

Initial moisture content <20mm	%	15.8
Material retained on 20mm sieve	%	1.3

Method of determining MCV
Steepest straight line

QA Ref  
SLD 4, 5.5  
Rev 72  
Aug 11



Printed:18/11/2011 17:35

Figure  
**MCVREL 13**  
sheet 2 of 2

# CHEMICAL TESTS - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANSEN BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	Org %	LOI %	pH	Sulphate as SO <sub>4</sub>			SD1 options		CO <sub>2</sub> %	Chloride, Cl		<2 mm %	Remarks	
	No.	Depth (m)		type					Preparation/test *	2:1 water sol. g/L	ground water g/L	acid sol. %	TS %		Mg NO <sub>3</sub> mg/L NH <sub>4</sub>	water sol. %			acid sol. %
		from	to																
BH1	6	1.65	1.85	D	Brown slightly sandy slightly gravelly CLAY.			7.9	1+3	0.20							93		
BH1	22	8.45	8.65	D	Brown slightly gravelly CLAY.			7.9	1+3	0.20							88		
BH1	42	15.95	16.15	D	Brown slightly sandy gravelly CLAY.			7.5	1+3	0.25							57		
BH1	60	22.50	23.95	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			7.9	1+3	0.26							91		
BH2	8	3.00	3.45	U	Stiff greyish brown slightly sandy slightly gravelly CLAY.			8.0	1+3	0.14							89		
BH2	25	11.45	11.65	D	Brown slightly sandy slightly gravelly CLAY.			7.8	1+3	0.33							91		
BH2	36	15.90		W				7.7	2+3		0.86								
BH2	38	17.45	17.65	D	Brown slightly sandy slightly gravelly CLAY.			7.6	1+3	0.22							98		
BH2	39	17.90		W				7.7	2+3		0.68								
BH2	54	24.45	24.65	D	Brown slightly sandy slightly gravelly CLAY.			7.6	1+3	0.19							93		
BH3	1	0.10		D	Brown slightly gravelly SILT with occasional rootlets.			5.7	1+3	0.12							96		
BH3	9	2.95	3.95	D	Brown slightly sandy slightly gravelly CLAY.			7.9	1+3	0.16							97		
BH3	22	7.65	8.10	D	Brown slightly sandy slightly gravelly CLAY.			7.8	1+3	0.35							91		
BH3	35	12.50	12.95	D	Greyish brown slightly sandy CLAY.			7.9	1+3	0.06							100		
BH3	48	17.95	18.15	D	Greyish brown slightly sandy slightly gravelly CLAY.			7.7	1+3	0.21							94		
BH4	4	0.20	0.30	B	Light grey sandy gravelly SILT.			8.7	1+3	0.98							56		
BH4	8	0.80	1.10	B	Brown slightly gravelly sandy CLAY.			7.6	1+3	0.33							69		
BH4	21	4.50		D	Dark grey slightly gravelly CLAY. Gravel is mainly weak mudstone.			8.1	1+3	1.19							100		
BH5	8	1.20	1.65	D	Grey very gravelly slightly clayey SAND.			7.1	1+3	0.41							63		

BS 1377 : definitive method unless stated :	* Sulphate tests preparation / test methods :	BRE Special Digest SD1, dependent options :
Org Organic matter content ( s-sulphides, c-chlorides identified )	1. BS 1377:Part 3:1990:clause 5.3	TS Total Sulphur to BR279 / EN ISO15178
LOI Mass loss on ignition at 440°C	2. BS 1377:Part 3:1990:clause 5.4	Mg Soluble Magnesium to BR279, colorimetric
CO <sub>2</sub> Carbonate content ( rapid titration )	3. BS 1377:Part 3:1990:clause 5.5	NO <sub>3</sub> Soluble Nitrate to BR279, colorimetric
Cl Chloride content	4. TRL447 - 1 water soluble sulphate	NH <sub>4</sub> qualitative
	5. TRL447 - 2 acid soluble sulphate	
	6. BR279 - groundwater sulphate	
	< 2mm material passing 2mm sieve	

QA Ref		Printed:01/12/2011 09:37	Table <b>CHEM 1</b>
SLR 3 Rev 95 Aug 11			

# CHEMICAL TESTS - SUMMARY OF RESULTS

Project No	Project Name
A1077-11	SANSEN BOREHOLES, NORTH YORKSHIRE

Hole No.	Sample				Soil Description	Org %	LOI %	pH	Sulphate as SO <sub>4</sub>			SD1 options		CO <sub>2</sub> %	Chloride, Cl		<2 mm %	Remarks	
	No.	Depth (m)		type					Preparation/test *	2:1 water sol. g/L	ground water g/L	acid sol. %	TS %		Mg NO <sub>3</sub> NH <sub>4</sub> mg/L	water sol. %			acid sol. %
		from	to																
WS1	2	0.30	1.00	B	Brown SAND.			8.0	1+3	0.22							100		
WS2	2	0.30	1.00	B	Brownish grey gravelly SAND.			8.1	1+3	0.20							92		
WS2	7	2.80		D	Grey sandy clayey SILT.			7.3	1+3	0.26							100		
WS3	2	0.30	1.00	B	Brown gravelly SAND.			7.2	1+3	0.14							87		
WS4	2	0.40	0.90	B	Grey and brown very gravelly SAND.			7.6	1+3	0.15							68		
WS5	4	1.70		D	Brown slightly sandy slightly gravelly CLAY.			7.6	1+3	0.19							93		

BS 1377 : definitive method unless stated : Org Organic matter content ( s-sulphides, c-chlorides identified ) LOI Mass loss on ignition at 440°C CO <sub>2</sub> Carbonate content ( rapid titration ) Cl Chloride content	* Sulphate tests preparation / test methods : 1. BS 1377:Part 3:1990:clause 5.3 2. BS 1377:Part 3:1990:clause 5.4 3. BS 1377:Part 3:1990:clause 5.5 < 2mm material passing 2mm sieve	BRE Special Digest SD1, dependent options : TS Total Sulphur to BR279 / EN ISO15178 Mg Soluble Magnesium to BR279, colorimetric NO3 Soluble Nitrate to BR279, colorimetric NH <sub>4</sub> qualitative
--	--	--

QA Ref		Printed:01/12/2011 09:37	Table <b>CHEM 2</b>
SLR 3 Rev 95 Aug 11			

**ENCLOSURE D**  
**GEOENVIRONMENTAL LABORATORY TEST RESULTS**

ESG Scientifics Reports Nos

EFS/119196  
EXR/127351

# TEST REPORT

## SOIL SAMPLE ANALYSIS



Report No. EFS/119196 (Ver. 1)

Soil Mechanics  
Askern Road  
Carcroft  
Doncaster  
South Yorkshire  
DN6 8DG

**Site: Sandsend Boreholes, North Yorkshire**

The 4 samples described in this report were registered for analysis by ESG on 01-Nov-2011. This report supersedes any versions previously issued by the laboratory.

The analysis was completed by: 14-Nov-2011

Tests where the accreditation is set to N or No, and any individual data items marked with a \* are not UKAS accredited  
Any opinions or interpretations expressed herein are outside the scope of any UKAS accreditation held by ESG.

The following tables are contained in this report:

- Table 1 Main Analysis Results (Pages 2 to 4)
- Table of PAH (MS-SIM) (80) Results (Pages 5 to 7)
- Table of PCB Congener Results (Page 8)
- Table of TPH Texas banding (std) (Page 9)
- GC-FID Chromatograms (Pages 10 to 12)
- Table of Asbestos Screening Results (Page 13)
- Analytical and Deviating Sample Overview (Pages 14 to 15)
- Table of Method Descriptions (Page 16)
- Table of Report Notes (Page 17)

On behalf of  
ESG :  
Andrew Timms

Operations Manager

Date of Issue: 14-Nov-2011

Tests marked 'N' have been subcontracted to another laboratory.

ESG accepts no responsibility for any sampling not carried out by our personnel.

Where individual results are flagged see report notes for status.







# Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

<b>Customer and Site Details:</b>	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
<b>Sample Details:</b>	BH1 ES 4A 1.00	<b>Job Number:</b>	S11_9196
<b>LIMS ID Number:</b>	CL1142188	<b>Date Booked in:</b>	01-Nov-11
<b>QC Batch Number:</b>	112027	<b>Date Extracted:</b>	11-Nov-11
<b>Quantitation File:</b>	Initial Calibration	<b>Date Analysed:</b>	14-Nov-11
<b>Directory:</b>	1411PAH.MS5\	<b>Matrix:</b>	Soil
<b>Dilution:</b>	1.0	<b>Ext Method:</b>	Ultrasonic

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration mg/kg	% Fit
Naphthalene	91-20-3	-	< 0.08	-
Acenaphthylene	208-96-8	-	< 0.08	-
Acenaphthene	83-32-9	-	< 0.08	-
Fluorene	86-73-7	-	< 0.08	-
Phenanthrene	85-01-8	-	< 0.08	-
Anthracene	120-12-7	-	< 0.08	-
Fluoranthene	206-44-0	-	< 0.08	-
Pyrene	129-00-0	-	< 0.08	-
Benzo[a]anthracene	56-55-3	-	< 0.08	-
Chrysene	218-01-9	-	< 0.08	-
Benzo[b]fluoranthene	205-99-2	-	< 0.08	-
Benzo[k]fluoranthene	207-08-9	-	< 0.08	-
Benzo[a]pyrene	50-32-8	-	< 0.08	-
Indeno[1,2,3-cd]pyrene	193-39-5	-	< 0.08	-
Dibenzo[a,h]anthracene	53-70-3	-	< 0.08	-
Benzo[g,h,i]perylene	191-24-2	-	< 0.08	-
Total (USEPA16) PAHs	-	-	< 1.28	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	122
Acenaphthene-d10	122
Phenanthrene-d10	139
Chrysene-d12	148
Perylene-d12	150

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	96
Terphenyl-d14	104

Concentrations are reported on a wet weight basis.

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.

# Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

<b>Customer and Site Details:</b>	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
<b>Sample Details:</b>	BH3 ES 2A 0.30	<b>Job Number:</b>	S11_9196
<b>LIMS ID Number:</b>	CL1142189	<b>Date Booked in:</b>	01-Nov-11
<b>QC Batch Number:</b>	112027	<b>Date Extracted:</b>	11-Nov-11
<b>Quantitation File:</b>	Initial Calibration	<b>Date Analysed:</b>	14-Nov-11
<b>Directory:</b>	1411PAH.MS5\	<b>Matrix:</b>	Soil
<b>Dilution:</b>	1.0	<b>Ext Method:</b>	Ultrasonic

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration mg/kg	% Fit
Naphthalene	91-20-3	-	< 0.08	-
Acenaphthylene	208-96-8	-	< 0.08	-
Acenaphthene	83-32-9	-	< 0.08	-
Fluorene	86-73-7	-	< 0.08	-
Phenanthrene	85-01-8	6.03	0.13	100
Anthracene	120-12-7	-	< 0.08	-
Fluoranthene	206-44-0	7.43	0.33	93
Pyrene	129-00-0	7.73	0.35	M
Benzo[a]anthracene	56-55-3	9.45	0.22	91
Chrysene	218-01-9	9.50	0.18	97
Benzo[b]fluoranthene	205-99-2	11.00	0.30	69
Benzo[k]fluoranthene	207-08-9	11.03	0.10	69
Benzo[a]pyrene	50-32-8	11.44	0.25	98
Indeno[1,2,3-cd]pyrene	193-39-5	12.85	0.16	97
Dibenzo[a,h]anthracene	53-70-3	-	< 0.08	-
Benzo[g,h,i]perylene	191-24-2	13.20	0.12	90
Total (USEPA16) PAHs	-	-	< 2.62	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	119
Acenaphthene-d10	119
Phenanthrene-d10	131
Chrysene-d12	137
Perylene-d12	140

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	93
Terphenyl-d14	100

Concentrations are reported on a wet weight basis.

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.

# Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

<b>Customer and Site Details:</b>	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
<b>Sample Details:</b>	BH3 ES 7A 2.00	<b>Job Number:</b>	S11_9196
<b>LIMS ID Number:</b>	CL1142190	<b>Date Booked in:</b>	01-Nov-11
<b>QC Batch Number:</b>	112027	<b>Date Extracted:</b>	11-Nov-11
<b>Quantitation File:</b>	Initial Calibration	<b>Date Analysed:</b>	14-Nov-11
<b>Directory:</b>	1411PAH.MS5\	<b>Matrix:</b>	Soil
<b>Dilution:</b>	1.0	<b>Ext Method:</b>	Ultrasonic

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration mg/kg	% Fit
Naphthalene	91-20-3	-	< 0.08	-
Acenaphthylene	208-96-8	-	< 0.08	-
Acenaphthene	83-32-9	-	< 0.08	-
Fluorene	86-73-7	-	< 0.08	-
Phenanthrene	85-01-8	-	< 0.08	-
Anthracene	120-12-7	-	< 0.08	-
Fluoranthene	206-44-0	-	< 0.08	-
Pyrene	129-00-0	-	< 0.08	-
Benzo[a]anthracene	56-55-3	-	< 0.08	-
Chrysene	218-01-9	-	< 0.08	-
Benzo[b]fluoranthene	205-99-2	-	< 0.08	-
Benzo[k]fluoranthene	207-08-9	-	< 0.08	-
Benzo[a]pyrene	50-32-8	-	< 0.08	-
Indeno[1,2,3-cd]pyrene	193-39-5	-	< 0.08	-
Dibenzo[a,h]anthracene	53-70-3	-	< 0.08	-
Benzo[g,h,i]perylene	191-24-2	-	< 0.08	-
Total (USEPA16) PAHs	-	-	< 1.28	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	119
Acenaphthene-d10	120
Phenanthrene-d10	133
Chrysene-d12	139
Perylene-d12	140

Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	95
Terphenyl-d14	103

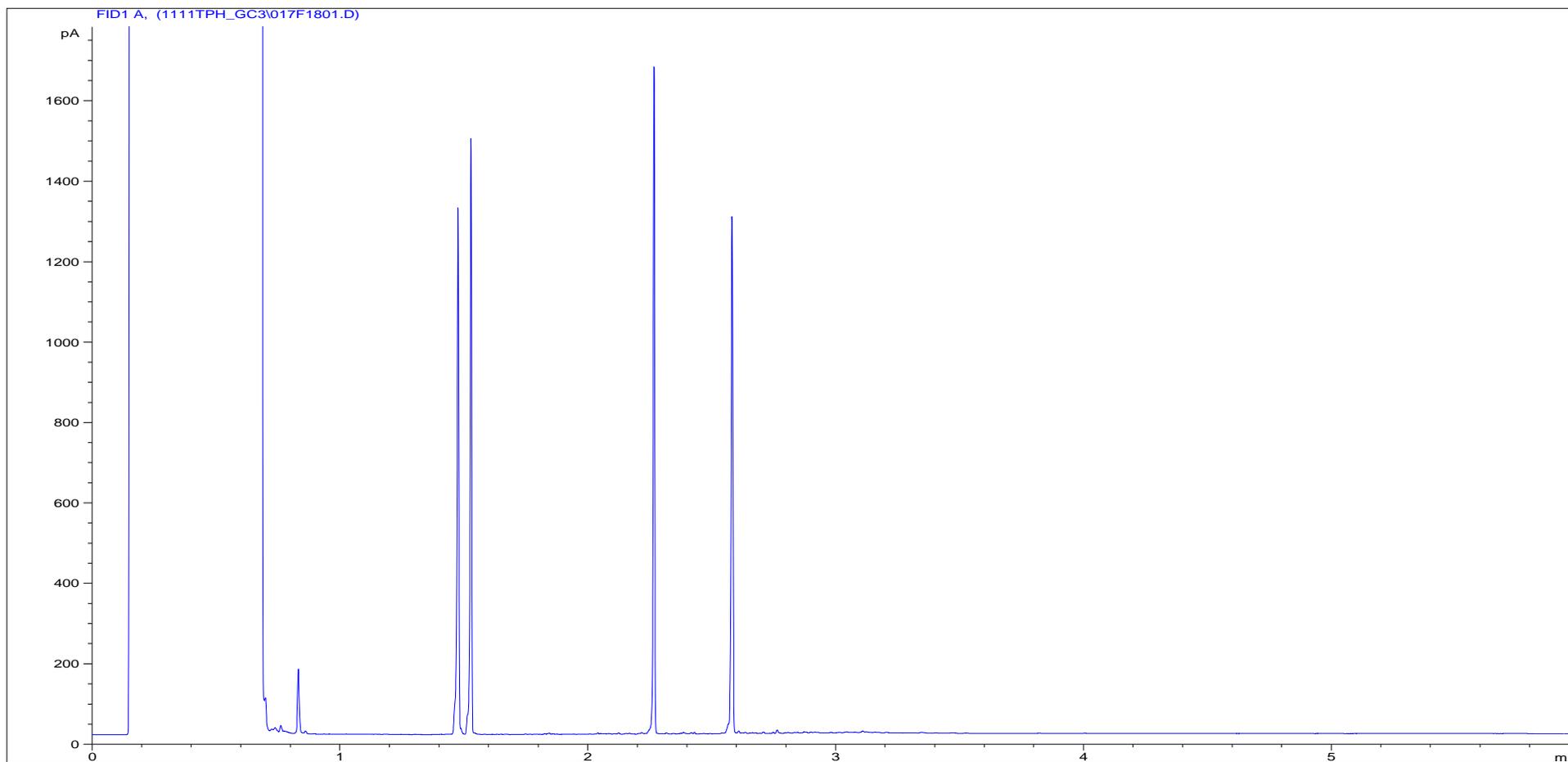
Concentrations are reported on a wet weight basis.

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.





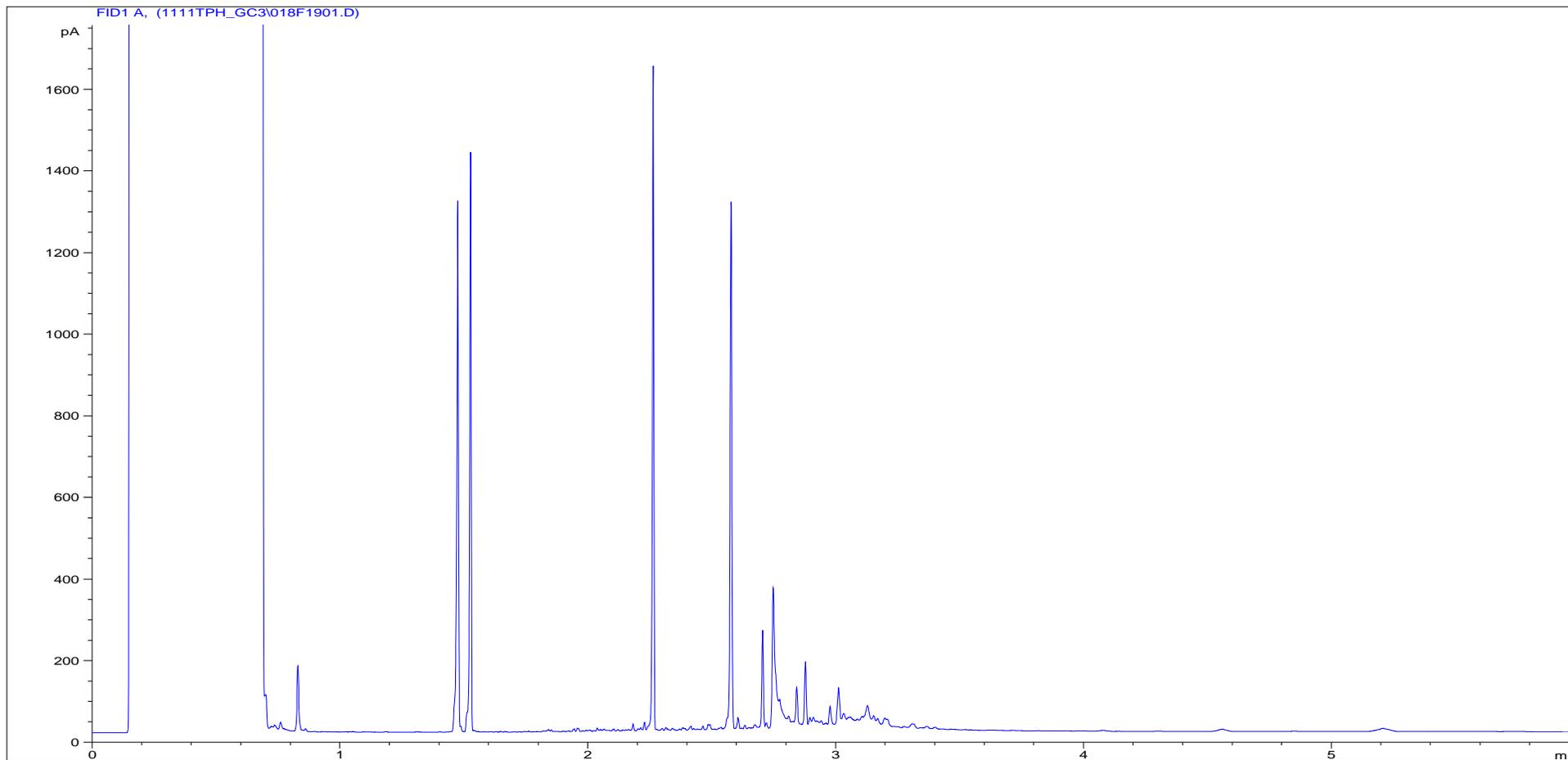
# Petroleum Hydrocarbons (C8 to C40) by GC/FID



<b>Sample ID:</b>	CL1142188	<b>Job Number:</b>	s11_9196
<b>Multiplier:</b>	8	<b>Client:</b>	Soil Mechanics
<b>Dilution:</b>	1	<b>Site:</b>	Sandsend Boreholes, North Yorkshire
<b>Acquisition Method:</b>	5UL_RUNF.M	<b>Client Sample Ref:</b>	BH1 ES 4A 1.00
<b>Acquisition Date/Time:</b>	11-Nov-11		
<b>Datafile:</b>	D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\017F1801.D		

Where individual results are flagged see report notes for status.

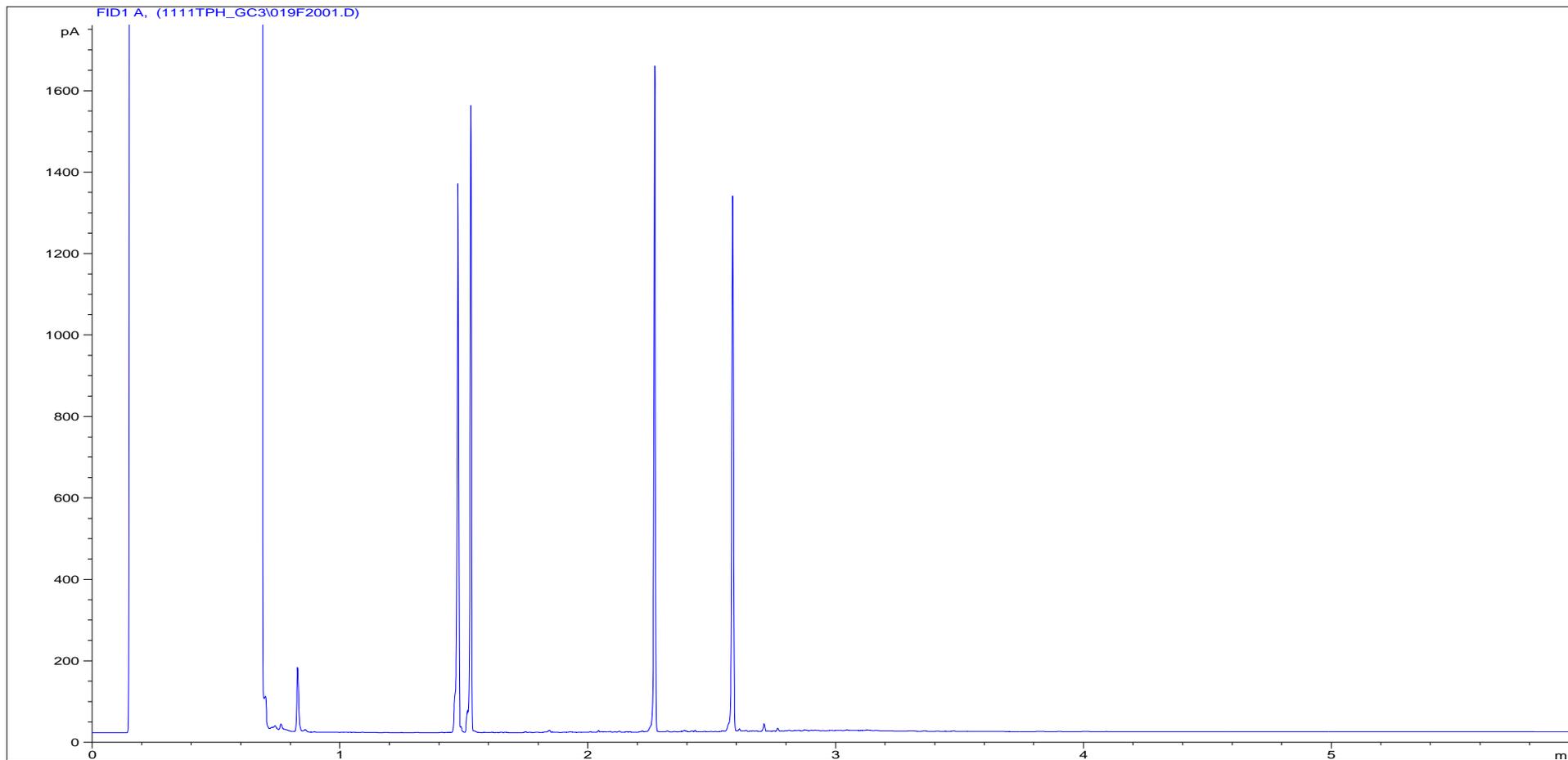
# Petroleum Hydrocarbons (C8 to C40) by GC/FID



<b>Sample ID:</b>	CL1142189	<b>Job Number:</b>	s11_9196
<b>Multiplier:</b>	8	<b>Client:</b>	Soil Mechanics
<b>Dilution:</b>	1	<b>Site:</b>	Sandsend Boreholes, North Yorkshire
<b>Acquisition Method:</b>	5UL_RUNF.M	<b>Client Sample Ref:</b>	BH3 ES 2A 0.30
<b>Acquisition Date/Time:</b>	11-Nov-11		
<b>Datafile:</b>	D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\018F1901.D		

Where individual results are flagged see report notes for status.

Petroleum Hydrocarbons (C8 to C40) by GC/FID



<b>Sample ID:</b>	CL1142190	<b>Job Number:</b>	s11_9196
<b>Multiplier:</b>	8	<b>Client:</b>	Soil Mechanics
<b>Dilution:</b>	1	<b>Site:</b>	Sandsend Boreholes, North Yorkshire
<b>Acquisition Method:</b>	5UL_RUNF.M	<b>Client Sample Ref:</b>	BH3 ES 7A 2.00
<b>Acquisition Date/Time:</b>	11-Nov-11		
<b>Datafile:</b>	D:\TES\DATA\Y2011\NOVEMBER2011\1111TPH_GC3\019F2001.D		

Where individual results are flagged see report notes for status.





Customer Soil Mechanics  
Site Sandsend Boreholes, North Yorkshire  
Report No S119196

Consignment No S24253  
Date Logged 01-Nov-2011

Report Due 14-Nov-2011

ID Number	Description	MethodID	PHSOIL	SFAP1	Sub002a	TMSS	TPHFI005	WMLM59	Sampled
Accredited to ISO17025									
CL/1142187	BH1 0.10	D	✓	✓	✓	✓	✓	✓	✓
CL/1142188	BH1 1.00	D	✓	✓	✓	✓	✓	✓	✓
CL/1142189	BH3 0.30	D	✓	✓	✓	✓	✓	✓	✓
CL/1142190	BH3 2.00	D	✓	✓	✓	✓	✓	✓	✓

**Note: For analysis where the Report Due date is greater than 7 days (PAH, Pesticides, PCB, Phenols, Herbicides) or 2 days (BOD) after the sampling date, although we will do our utmost to prioritise your samples, they may become deviant whilst being processed in the Laboratory.**

**In this instance, please contact the Laboratory immediately should you wish to discuss how you would like us to proceed. If you do not respond within 24 hours, we will proceed as originally requested.**

Deviating Sample Key	
A	The sample was received in an inappropriate container for this analysis
B	The sample was received without the correct preservation for this analysis
C	Headspace present in the sample container
D	The sampling date was not supplied so holding time may be compromised - applicable to all analysis
E	Sample processing did not commence within the appropriate holding time
Requested Analysis Key	
■	Analysis Required
■	Analysis dependant upon trigger result - <b>Note: due date may be affected if triggered</b>
■	No analysis scheduled
^	Analysis Subcontracted

Where individual results are flagged see report notes for status.

# Method Descriptions

Matrix	MethodID	Analysis Basis	Method Description
Soil	BTEXHSA	As Received	Determination of Benzene, Toluene, Ethyl benzene and Xylenes (BTEX) by Headspace GCFID
Soil	GROHSA	As Received	Determination of Total Gasoline Range Organics Hydrocarbons (GRO) by Headspace GCFID
Soil	ICPBOR	Air Dried	Determination of Boron in soil samples by hot water extraction followed by ICPOES detection
Soil	ICPMSS	Air Dried	Determination of Metals in soil samples by aqua regia digestion followed by ICPMS
Soil	ICPSOIL	Air Dried	Determination of Metals in soil samples by aqua regia digestion followed by ICPOES detection
Soil	ICPWSS	Air Dried	Determination of Water Soluble Sulphate in soil samples by water extraction followed by ICPOES detection
Soil	PAHMSUS	As Received	Determination of Polycyclic Aromatic Hydrocarbons (PAH) by hexane/acetone extraction followed by GCMS detection
Soil	PCBUSECDAR	As Received	Determination of Polychlorinated Biphenyl (PCB) congeners/arocloris by hexane/acetone extraction followed by GCECD detection
Soil	PHSOIL	As Received	Determination of pH of 2.5:1 deionised water to soil extracts using pH probe.
Soil	SFAPI	As Received	Segmented flow analysis with colorimetric detection
Soil	SubCon*	*	Contact Laboratory for details of the methodology used by the sub-contractor.
Soil	TMSS	As Received	Determination of the Total Moisture content at 105°C by loss on oven drying gravimetric analysis
Soil	TPHFIDUS	As Received	Determination of hexane/acetone extractable Hydrocarbons in soil with GCFID detection.
Soil	WSLM59	Air Dried	Determination of Organic Carbon in soil using sulphurous Acid digestion followed by high temperature combustion and IR detection

Where individual results are flagged see report notes for status.

# Report Notes

## Generic Notes

### Soil/Solid Analysis

Unless stated otherwise,

- Results expressed as mg/kg have been calculated on an air dried basis
- Sulphate analysis not conducted in accordance with BS1377
- Water Soluble Sulphate is on a 2:1 water:soil extract

### Waters Analysis

Unless stated otherwise results are expressed as mg/l

**Nil:** Where "Nil" has been entered against Total Alkalinity or Total Acidity this indicates that a measurement was not required due to the inherent pH of the sample.

### Oil analysis specific

Unless stated otherwise,

- Results are expressed as mg/kg
- SG is expressed as g/cm<sup>3</sup>@ 15°C

### Gas (Tedlar bag) Analysis

Unless stated otherwise, results are expressed as ug/l

### Asbestos Analysis

**CH** Denotes Chrysotile

**CR** Denotes Crocidolite

**AM** Denotes Amosite

**NAIS** No Asbestos Identified in Sample

## Symbol Reference

**^** Sub-contracted analysis. Note: The accreditation status is that assigned by the subcontract laboratory.

**\$\$** Unable to analyse due to the nature of the sample

**¶** Samples submitted for this analyte were not preserved on site in accordance with laboratory protocols.

This may have resulted in deterioration of the sample(s) during transit to the laboratory.

Consequently the reported data may not represent the concentration of the target analyte present in the sample at the time of sampling

**¥** Results for guidance only due to possible interference

**&** Blank corrected result

**I.S** Insufficient sample to complete requested analysis

**I.S(g)** Insufficient sample to re-analyse, results for guidance only

**Intf** Unable to analyse due to interferences

**N.D** Not determined

**N.Det** Not detected

**Req** Analysis requested, see attached sheets for results

**▮** Raised detection limit due to nature of the sample

\* All accreditation has been removed by the laboratory for this result

‡ MCERTS accreditation has been removed for this result

**Note:** The Laboratory may only claim that data is accredited when all of the requirements of our Quality System have been met. Where these requirements have not been met the laboratory may elect to include the data in its final report and remove the accreditation from individual data items if it believes that the validity of the data has not been affected. If further details are required of the circumstances which have led to the removal of accreditation then please do not hesitate to contact the laboratory.

**END OF REPORT**

Where individual results are flagged see report notes for status.

# TEST REPORT

## CEN LEACHATE 2:1



Report No. EXR/127351 (Ver. 1)

Soil Mechanics  
Askern Road  
Carcroft  
Doncaster  
South Yorkshire  
DN6 8DG

**Site: Sandsend Boreholes, North Yorkshire**

The 2 samples described in this report were registered for analysis by ESG on 01-Nov-2011. This report supersedes any versions previously issued by the laboratory.

The analysis was completed by: 16-Nov-2011

Tests where the accreditation is set to N or No, and any individual data items marked with a \* are not UKAS accredited  
Any opinions or interpretations expressed herein are outside the scope of any UKAS accreditation held by ESG.

The following tables are contained in this report:

Table 1 Main Analysis Results (Pages 2 to 4)  
Table of PAH (MS-SIM) (10) Results (Page 5)  
Table of PCB Congener Results (Page 6)  
Table of TPH Texas banding (0.01) (Page 7)  
GC-FID Chromatograms (Page 8)  
Analytical and Deviating Sample Overview (Pages 9 to 10)  
Table of Method Descriptions (Page 11)  
Table of Report Notes (Page 12)

On behalf of  
ESG :  
Andrew Timms

Operations Manager

Date of Issue: 16-Nov-2011

Tests marked 'N' have been subcontracted to another laboratory.

ESG accepts no responsibility for any sampling not carried out by our personnel.

Where individual results are flagged see report notes for status.







# Polycyclic Aromatic Hydrocarbons GC/MS (SIM)

<b>Customer and Site Details:</b>	Soil Mechanics: Sandsend Boreholes, North Yorkshire		
<b>Sample Details:</b>	BH1 1.00	<b>Job Number:</b>	W12_7351
<b>LIMS ID Number:</b>	EX1147751	<b>Date Booked in:</b>	01-Nov-11
<b>QC Batch Number:</b>	110983	<b>Date Extracted:</b>	15-Nov-11
<b>Quantitation File:</b>	Initial Calibration	<b>Date Analysed:</b>	15-Nov-11
<b>Directory:</b>	115PAH.MS17\	<b>Matrix:</b>	Leachate
<b>Dilution:</b>	1.0	<b>Ext Method:</b>	Bottle

UKAS accredited?: Yes

Target Compounds	CAS #	R.T. (min)	Concentration ug/l	% Fit
Naphthalene	91-20-3	3.36	0.012	94
Acenaphthylene	208-96-8	4.41	0.035	95
Acenaphthene	83-32-9	4.53	0.204	98
Fluorene	86-73-7	4.92	0.138	98
Phenanthrene	85-01-8	-	< 0.010	-
Anthracene	120-12-7	-	< 0.010	-
Fluoranthene	206-44-0	7.13	0.063	89
Pyrene	129-00-0	7.42	0.119	87
Benzo[a]anthracene	56-55-3	9.10	0.024	82
Chrysene	218-01-9	9.15	0.019	83
Benzo[b]fluoranthene	205-99-2	10.63	0.010	91
Benzo[k]fluoranthene	207-08-9	-	< 0.010	-
Benzo[a]pyrene	50-32-8	-	< 0.010	-
Indeno[1,2,3-cd]pyrene	193-39-5	-	< 0.010	-
Dibenzo[a,h]anthracene	53-70-3	-	< 0.010	-
Benzo[g,h,i]perylene	191-24-2	-	< 0.010	-
Total (USEPA16) PAHs	-	-	< 0.694	-

"M" denotes that % fit has been manually interpreted

Internal Standards	% Area
1,4-Dichlorobenzene-d4	NA
Naphthalene-d8	96
Acenaphthene-d10	95
Phenanthrene-d10	95
Chrysene-d12	99
Perylene-d12	97

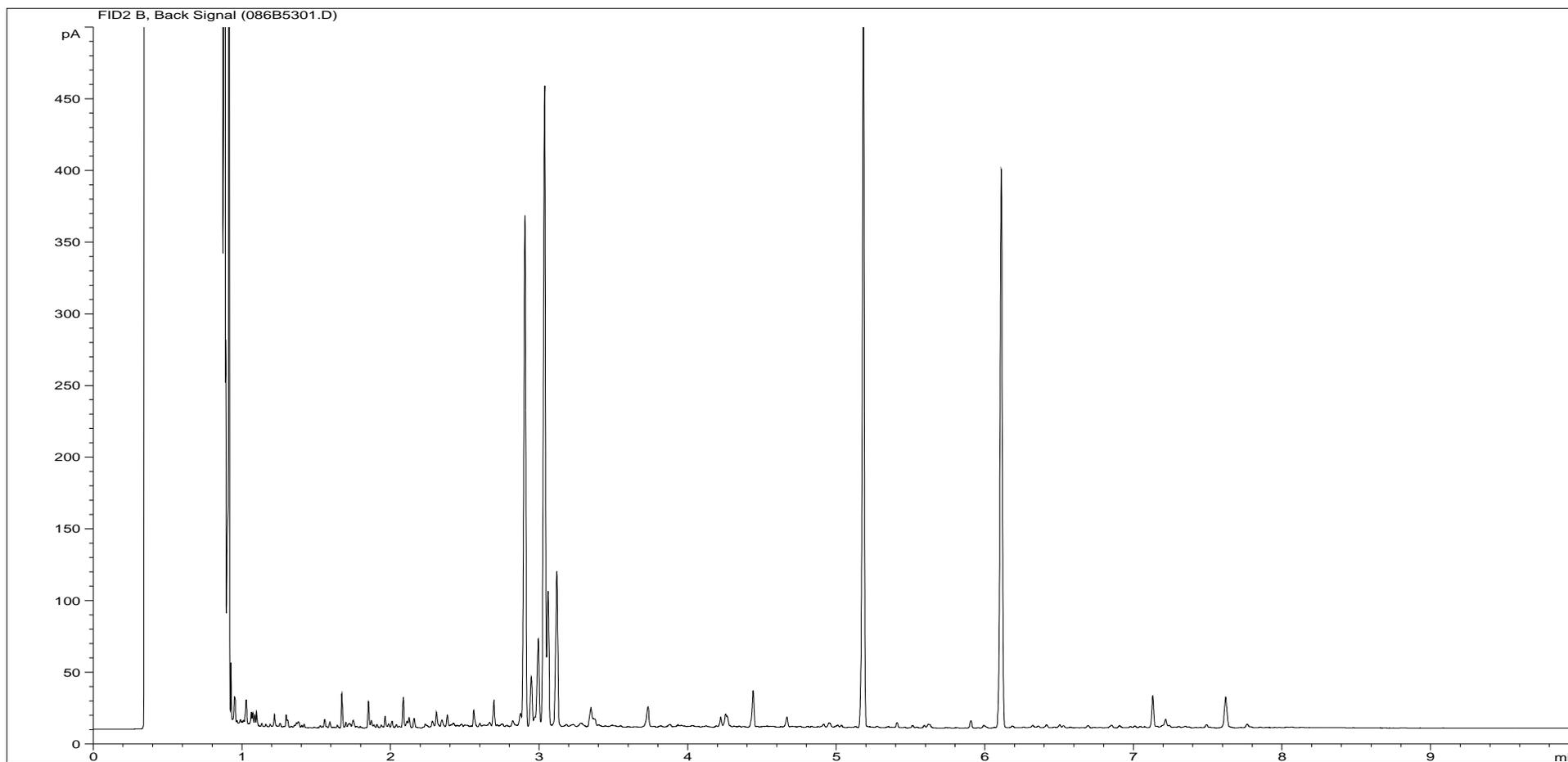
Surrogates	% Rec
Nitrobenzene-d5	NA
2-Fluorobiphenyl	94
Terphenyl-d14	116

The Total PAH result is the sum of non-rounded individual PAH results and therefore may differ to the sum of the rounded individual PAH results printed above. By convention, where any one or more result is a "less than", the total is expressed as a "less than" and includes the "less than" concentration within the total.





# Petroleum Hydrocarbons (C8 to C40) by GC/FID



<b>Sample ID:</b>	EX1147751	<b>Job Number:</b>	W12_7351
<b>Multiplier:</b>	0.005	<b>Client:</b>	Soil Mechanics
<b>Dilution:</b>	1	<b>Site:</b>	Sandsend Boreholes, North Yorkshire
<b>Acquisition Method:</b>	TPH_RUNF.M	<b>Client Sample Ref:</b>	BH1 1.00
<b>Acquisition Date/Time:</b>	09-Nov-11, 01:50:04		
<b>Datafile:</b>	D:\TES\DATA\Y2011\110811TPH_GC16\110811 2011-11-08 09-00-42\086B5301.D		

Where individual results are flagged see report notes for status.

**Customer** Soil Mechanics  
**Site** Sandsend Boreholes, North Yorkshire  
**Report No** W127351

Consignment No S24253  
 Date Logged 01-Nov-2011

Report Due 14-Nov-2011

ID Number	Description	MethodID	BTEXHSA	CUSTSERV	GROHSA	ICPMSW	ICPMSVAR														LeachPrep	PAHMSW	PCBONEC	SFAP1		
			BTEX-HSA + MTBE analysis	Report B (CEN1)	GRO-HSA	Nickel as Ni MS (Dissolved)	Chromium as Cr MS (Dissolved)	Cadmium as Cd MS (Dissolved)	Copper as Cu MS (Dissolved)	Lead as Pb MS (Dissolved)	Zinc as Zn MS (Dissolved)	Manganese as Mn MS (Dissolved)	Arsenic as As MS (Dissolved)	Mercury as Hg MS (Dissolved)	Selenium as Se MS (Dissolved)	Vanadium as V MS (Dissolved)	Cobalt as Co MS (Dissolved)	Total Sulphur as SO4 (Diss) VAR	Magnesium as Mg (Dissolved) VAR	Barium as Ba (Dissolved) VAR	Iron as Fe (Dissolved) VAR	Aluminium as Al (Dissolved) VAR	Boron as B (Dissolved) VAR	Leachate Prep	PAH GC-MS (16)	PCB - 7 Congeners Analysis
Accredited to ISO17025			✓		✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓	✓
EX/1147750	BH1 0.10																									
EX/1147751	BH1 1.00																									

**Note: For analysis where the Report Due date is greater than 7 days (PAH, Pesticides, PCB, Phenols, Herbicides) or 2 days (BOD) after the sampling date, although we will do our utmost to prioritise your samples, they may become deviant whilst being processed in the Laboratory.**

**In this instance, please contact the Laboratory immediately should you wish to discuss how you would like us to proceed. If you do not respond within 24 hours, we will proceed as originally requested.**

Deviating Sample Key	
A	The sample was received in an inappropriate container for this analysis
B	The sample was received without the correct preservation for this analysis
C	Headspace present in the sample container
D	The sampling date was not supplied so holding time may be compromised - applicable to all analysis
E	Sample processing did not commence within the appropriate holding time
Requested Analysis Key	
	Analysis Required
	Analysis dependant upon trigger result - <b>Note: due date may be affected if triggered</b>
	No analysis scheduled
	Analysis Subcontracted

**Customer** Soil Mechanics  
**Site** Sandsend Boreholes, North Yorkshire  
**Report No** W127351

Consignment No S24253  
 Date Logged 01-Nov-2011

Report Due 14-Nov-2011

ID Number	Description	MethodID	TPHFID	WSLM3	WSLM3
		Sampled	TPH Carbon Banding	TPH GC (0.01)	Total Organic Carbon
Accredited to ISO17025		✓	✓	✓	✓
EX/1147750	BH1 0.10				
EX/1147751	BH1 1.00				

**Note: For analysis where the Report Due date is greater than 7 days (PAH, Pesticides, PCB, Phenols, Herbicides) or 2 days (BOD) after the sampling date, although we will do our utmost to prioritise your samples, they may become deviant whilst being processed in the Laboratory.**

**In this instance, please contact the Laboratory immediately should you wish to discuss how you would like us to proceed. If you do not respond within 24 hours, we will proceed as originally requested.**

Deviating Sample Key	
A	The sample was received in an inappropriate container for this analysis
B	The sample was received without the correct preservation for this analysis
C	Headspace present in the sample container
D	The sampling date was not supplied so holding time may be compromised - applicable to all analysis
E	Sample processing did not commence within the appropriate holding time
Requested Analysis Key	
<span style="background-color: #90EE90;"> </span>	Analysis Required
<span style="background-color: #FFFF00;"> </span>	Analysis dependant upon trigger result - <b>Note: due date may be affected if triggered</b>
<span style="background-color: #D3D3D3;"> </span>	No analysis scheduled
^	Analysis Subcontracted

# Method Descriptions

Matrix	MethodID	Analysis Basis	Method Description
Water	BTEXHSA	As Received	Benzene, Toluene, Ethylbenzene, & Xylenes by headspace extraction GCFID quantitation
Water	GROHSA	As Received	Determination of Total Gasoline Range Organics Hydrocarbons (GRO) by Headspace FID
Water	ICPMSW	As Received	Direct quantitative determination of Metals in water samples using ICPMS
Water	ICPWATVAR	As Received	Direct determination of Metals and Sulphate in water samples using ICPOES
Water	PAHMSW	As Received	Determination of PolyAromatic Hydrocarbons in water by pentane extraction GCMS quantitation
Water	PCBCONEC	As Received	Determination of Polychlorinated Biphenyl (PCB) congeners by pentane extraction followed by GCECD detection
Water	SFAPI	As Received	Segmented flow analysis with colorimetric detection
Water	TPHFID	As Received	Determination of pentane extractable hydrocarbons in water by GCFID
Water	WSLM13	As Received	Instrumental analysis using acid/persulphate digestion and dispersive IR detection
Water	WSLM3	As Received	Determination of the pH of water samples by pH probe

Where individual results are flagged see report notes for status.

# Report Notes

## Generic Notes

### **Soil/Solid Analysis**

Unless stated otherwise,

- Results expressed as mg/kg have been calculated on an air dried basis
- Sulphate analysis not conducted in accordance with BS1377
- Water Soluble Sulphate is on a 2:1 water:soil extract

### **Waters Analysis**

Unless stated otherwise results are expressed as mg/l

**Nil:** Where "Nil" has been entered against Total Alkalinity or Total Acidity this indicates that a measurement was not required due to the inherent pH of the sample.

### **Oil analysis specific**

Unless stated otherwise,

- Results are expressed as mg/kg
- SG is expressed as g/cm<sup>3</sup>@ 15°C

### **Gas (Tedlar bag) Analysis**

Unless stated otherwise, results are expressed as ug/l

### **Asbestos Analysis**

**CH** Denotes Chrysotile

**CR** Denotes Crocidolite

**AM** Denotes Amosite

**NAIS** No Asbestos Identified in Sample

## Symbol Reference

^ Sub-contracted analysis. Note: The accreditation status is that assigned by the subcontract laboratory.

\$\$ Unable to analyse due to the nature of the sample

¶ Samples submitted for this analyte were not preserved on site in accordance with laboratory protocols.

This may have resulted in deterioration of the sample(s) during transit to the laboratory.

Consequently the reported data may not represent the concentration of the target analyte present in the sample at the time of sampling

¥ Results for guidance only due to possible interference

& Blank corrected result

I.S Insufficient sample to complete requested analysis

I.S(g) Insufficient sample to re-analyse, results for guidance only

Intf Unable to analyse due to interferences

N.D Not determined

N.Det Not detected

Req Analysis requested, see attached sheets for results

▮ Raised detection limit due to nature of the sample

\* All accreditation has been removed by the laboratory for this result

‡ MCERTS accreditation has been removed for this result

**Note:** The Laboratory may only claim that data is accredited when all of the requirements of our Quality System have been met. Where these requirements have not been met the laboratory may elect to include the data in its final report and remove the accreditation from individual data items if it believes that the validity of the data has not been affected. If further details are required of the circumstances which have led to the removal of accreditation then please do not hesitate to contact the laboratory.

**END OF REPORT**

Where individual results are flagged see report notes for status.

**ENCLOSURE E  
PHOTOGRAPHS**

Rotary Cores

Plates 1 to 8

**ENCLOSURE F  
DRAWINGS**

Site Location Plan  
Site Plan

F1  
F2 and 3

# Site Location Plan



Soil Mechanics



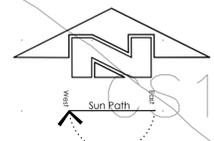
**THE SITE**

Reproduced from the 1996 Ordnance Survey 1:50 000 scale Landranger map No 94 by permission of Ordnance Survey on behalf of The Controller of Her Majesty's Stationery Office, © Crown copyright, Environmental Scientifics Group Limited. All rights reserved. Licence Number 100006060

Notes:  
Scale 1:50 000

Project SANDSEND SLOPE STABILISATION, WHITBY, NORTH YORKSHIRE  
 Project No. A1077-11  
 Carried out for Balfour Beatty Living Places

Figure  
**F1**



**GENERAL NOTES**

- 1.Reproduced from Ordnance Survey Data with the permission of The Controller of Her Majesty's Stationery Office, a Crown Copyright. Supplied by \*\*\*\*\*.
- 2.Reproduced from CLIENT NAME's Drawing No. \*\*\*\*\* Rev No. \*\*\*.
- 3.Hole Locations to OSGB36 Co-ordinate Reference System.
- 4.All dimensions are in metres unless indicated otherwise.
- 5.All levels are in metres and related to OS Orthometric level datum.

Survey Control Co-ordinates			
Reference	East	North	Elevation
SANDS REF	48711.203	51292.729	30.81

OS Investigation Co-ordinates (OSGB36 + OS Orthometric Ht)				
Reference	East	North	Elevation	Description
BH01	486765.856	512240.035	44.48	Borehole
BH02	486799.848	512274.241	45.54	Borehole
BH03	486650.134	512295.035	45.95	Borehole
BH04	486819.851	512318.442	14.59	Borehole
BH05	486705.875	512374.439	12.66	Borehole
BH06	486797.823	512388.088	13.02	Borehole
W001	487176.623	512177.532	4.43	Window Sample
W002	487089.808	512215.234	3.88	Window Sample
W003	486809.973	512351.996	3.98	Window Sample
W004	486649.562	512432.748	2.89	Window Sample
W005	486380.618	512549.270	3.72	Window Sample

**LEGEND TO SYMBOLS**

- ⊕ Borehole Location
- ⊕ Window Sample Location
- ⊕ Trial Pit Location
- ⊕ Dynamic Probe Location
- ⊕ Inspection Pit Location
- ⊕ Water Sample Location

**Geology Codes:**

- TS Topsoil
- MG Made Ground
- AL Alluvium
- LC London Clay
- LT Lambeth Group
- GT Glacial Till

Scale: 1:NNN

Rev	Drawn	Date	Approv.	Date	Modification Details

**AMENDMENTS**

Title: Ground Investigation Survey

Project: Sandsend Whitby Nth Yorkshire

Client: CLIENT NAME OR LOGO

**Soil Mechanics**

Date	Drawn By	Approv. By
12.10.11	HSL	DEF

Sheet Size	Scale	Project No
A1-1	1:1000	123456

Drawing No	Rev
123456/001	

**PRELIMINARY / DRAFT**

